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PLANNING COMMISSION MEETING AGENDA REGULAR MEETING

Carlton Faison, Chairman, Tom Krent, Vice Chairman Ollie Apahidean, Karen Crusse, Barbara Fowler, Michael W. Hutson, David Lambert, Sadek Rahman and John J. Tagle

November 12, 2019

7:00 P.M.

Council Chambers

- 1. ROLL CALL
- 2. APPROVAL OF AGENDA
- 3. <u>APPROVAL OF MINUTES</u> October 22, 2019
- 4. PUBLIC COMMENT For Items Not on the Agenda

SPECIAL USE

 <u>PUBLIC HEARING - SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File</u> <u>Number SU JPLN2019-0008)</u> – Proposed Courtyard Hotel, East side of New King, west of Crooks, 5550 New King (PIN 88-20-08-276-006), Section 8, Currently Zoned OM (Office Mixed Use) Zoning District

PRELIMINARY SITE PLAN REVIEW

- PRELIMINARY SITE PLAN REVIEW (SP JPLN2019-0032) Proposed GFA Long Lake Multifamily Development, West side of Dequindre, north of Long Lake (PIN 88-20-12-476-070), Section 12, Currently Zoned NN (Neighborhood Node "J") and EP (Environmental Protection) Districts
- 7. <u>PUBLIC COMMENT</u> Items on Current Agenda
- 8. PLANNING COMMISSION COMMENT

ADJOURN

NOTICE: People with disabilities needing accommodations for effective participation in this meeting should contact the City Clerk by e-mail at <u>clerk@troymi.gov</u> or by calling (248) 524-3317 at least two working days in advance of the meeting. An attempt will be made to make reasonable accommodations.

Chair Faison called the Regular meeting of the Troy City Planning Commission to order at 7:00 p.m. on October 22, 2019 in the Council Chamber of the Troy City Hall.

1. ROLL CALL

Present: Ollie Apahidean Karen Crusse Carlton M. Faison Barbara Fowler Michael W. Hutson Tom Krent David Lambert Sadek Rahman

<u>Absent:</u> John J. Tagle

<u>Also Present:</u> R. Brent Savidant, Community Development Director Ben Carlisle, Carlisle Wortman Associates Lori Grigg Bluhm, City Attorney Jackie Ferencz, Planning Department Administrative Assistant Kathy L. Czarnecki, Recording Secretary

2. <u>APPROVAL OF AGENDA</u>

Resolution # PC-2019-10-071

Moved by: Krent Support by: Apahidean

RESOLVED, To approve the Agenda as prepared.

Yes: All present (8) Absent: Tagle

MOTION CARRIED

3. APPROVAL OF MINUTES

Resolution # PC-2019-10-072

Moved by: Lambert Support by: Fowler

RESOLVED, To approve the minutes of the October 8, 2019 Regular meeting as submitted.

Yes: All present (8) Absent: Tagle

MOTION CARRIED

4. <u>PUBLIC COMMENT</u> – Items not on the Agenda

There was no one present who wished to speak.

PRELIMINARY SITE PLAN REVIEW

 <u>PRELIMINARY SITE PLAN REVIEW (SP JPLN2019-0031)</u> – Proposed GFA Hopedale Site Condominium, 7 units/lots, North of Hopedale, West of John R, South of Square Lake (88-20-11-277-024), Section 11, Currently Zoned R-1C (One Family Residential) Zoning District

Mr. Carlisle reviewed the Preliminary Site Plan application. Mr. Carlisle said the proposed development is permitted by right and meets all requirements of the Zoning Ordinance. He addressed the stub street extension of Viking to Hopedale, the mitigation of trees and elevations. Mr. Carlisle recommended Preliminary Site Plan approval.

Applicant Gary Abitheira addressed the various elevations inclusive of a custom ranch option, building materials and price ranges.

Mr. Savidant announced the Planning Department received correspondence from Dr. Kay-Uwe and Ulrike Wagner of 5564 Viking in opposition addressing concerns with non-preservation of existing green space.

Chair Faison opened the floor for public comment.

- Andreas Gebauer, 5548 Viking; voiced concerns with extension of Viking, vehicular lights from side-entry garages.
- Mircea Costeiu, 5563 Viking; voiced concerns with potential increased traffic, home values.
- Paul Krebs, 1896 Hopedale; voiced concerns with parking along Hopedale, extension of Viking in relation to his property, property maintenance.
- Ulrike Wagner, 5564 Viking; asked to dedicate existing green space to the neighborhood.

Chair Faison closed the floor for public comment.

Mr. Abitheira brought to the Board's attention a notation on the site plan with reference to the detention basin. He said if an alternate off-site location for the detention basin is available, the plan would allow for an additional lot/unit.

There was discussion on:

- Elevations; side entry garages.
- Stormwater detention basin; Site Plan notation to convert to additional lot/unit.
- Traffic concerns.
- Stub street; designed for future development.
- Existing green space; potential to dedicate as neighborhood park.

Resolution # PC-2019-10-073

Moved by: Krent Support by: Lambert

RESOLVED, That Preliminary Site Condominium Approval, pursuant to Article 8 and Section 10.02 of the Zoning Ordinance, as requested for GFA Hopedale Site Condominium, 7 units/lots, North of Hopedale, West of John R, South of Square Lake (88-20-11-277-024), Section 11, Currently Zoned R-1C (One Family Residential) District, be granted, and that the Planning Commission supports utilization of the stormwater detention convertible area as an additional lot/unit totaling eight (8) lots/units, subject to engineering approval.

Yes: All present (8) Absent: Tagle

MOTION CARRIED

 <u>PUBLIC HEARING - CONDITIONAL REZONING (CR JPLN2019-001)</u> – Proposed MNK Troy 1, LLC Conditional Rezoning, East side of Rochester, South of Shallowdale (88-20-14-152-001 and 88-20-14-301-031), Section 14, From R-1C (One Family Residential), RT (One Family Attached Residential) and EP (Environmental Protection) Zoning Districts to RT (One Family Attached Residential) and EP (Environmental Protection) Zoning Districts

Mr. Carlisle reviewed the Conditional Rezoning application. He addressed the history of the parcels as relates to zoning and land use policies, noting for the past 20 years the zoning classification for Rochester Road is medium density residential. Mr. Carlisle compared what could be developed by right versus what could be developed with the proposed conditional rezoning.

Mr. Carlisle addressed site plan issues; 1) the proposed EP zoned portion cannot be counted toward the overall lot density, reducing the maximum number of units to 27; 2) the proposed setbacks differ from setbacks shown on conceptual plan; and 3) the minimum landscape requirement is 20%, not 15% as proposed.

Mr. Carlisle recommended to postpone the application so the applicant can address site plan issues and take into consideration comments from both the Planning Commission and public.

John Thompson of Professional Engineering Associates (PEA) was present.

PUBLIC HEARING OPENED

• Jon Hughes, 4495 Harold; voiced opposition. He addressed concerns with a reduction of the EP-zoned portion and its effect on his property.

• Jim McCauley, 4435 Harold; representative of Shallowbrook Homeowners' Association. Mr. McCauley addressed concerns with the reduction of the negotiated EP zoned area, EP density as relates to the number of proposed lots, reduction of open space, retention/detention plans and deficiencies in the application. He asked the Board's consideration in postponing the item to allow the applicant to address the impact on homeowners as relates to the negotiated buffer in place since 2002.

PUBLIC HEARING CLOSED

There was discussion on:

- Intent of EP zoned area to serve as buffer to residential from more intense uses.
- Proposed reduction of EP zoned area and open space.
- Consideration of homeowners who purchased homes with understanding of EP zoned protected area.
- Conditional Rezoning process; recommending body to City Council.

Mr. Thompson said the applicant is amenable to addressing concerns expressed this evening at the public hearing and take into consideration comments of the Planning Commission. He said the intent is to provide significant buffer and open space that correlates to the original agreement. Mr. Thompson said development of the property as currently zoned almost makes the property undevelopable.

Resolution # PC-2019-10-074

Moved by: Hutson Support by: Rahman

RESOLVED, That the Planning Commission hereby postpones this item to provide the applicant an opportunity to clarify their application, based on issues identified in the report and comments by the Planning Commission. Furthermore, postponement will provide the applicant an opportunity to consider public comments from residents and input from the Planning Commission.

Yes: All present (8) Absent: Tagle

MOTION CARRIED

OTHER BUSINESS

7. <u>POTENTIAL TEXT AMENDMENTS</u> – (a) Transitions; (b) Increasing Square Footage Cap in a Cluster Development

Mr. Savidant addressed item 7. (b) *Increasing Square Footage Cap in a Cluster Development*. He said the proposed text would increase the maximum square footage of a home from 1,500 square feet to 1,700 square feet as an incentive to develop homes desirable to empty-nesters.

After a brief discussion, the Board agreed to move forward with the proposed text amendment and schedule a Public Hearing.

Mr. Carlisle addressed item 7. (a) *Transitions*. Mr. Carlisle said the proposed text amendment relates only to Neighborhood Nodes. He addressed the proposed amendment as relates to development height, setback and greenbelt provisions for any non-single-family development in Neighborhood Nodes.

Discussion followed on:

• Revising text to clarify the minimum 20-foot landscaped greenbelt is within the twenty (20) foot building setback for parcels abutting single family residential.

The Board agreed to move forward with the proposed text amendment and schedule a Public Hearing.

8. PUBLIC COMMENT

There was no one present who wished to speak.

9. PLANNING COMMISSION COMMENT

There were general Planning Commission comments.

Mr. Apahidean said tonight might be his final Planning Commission meeting dependent on the outcome of the upcoming election.

The Regular meeting of the Planning Commission adjourned at 9:05 p.m.

Respectfully submitted,

Carlton Faison, Chair

Kathy L. Czarnecki, Recording Secretary

C:\Users\bob\Documents\Kathy\COT Planning Commission Minutes\2019\2019 10 22 Regular Meeting_Draft.doc

DATE: November 6, 2019

- TO: Planning Commission
- FROM: R. Brent Savidant, Planning Director
- SUBJECT: <u>PUBLIC HEARING SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File</u> <u>Number SU JPLN2019-0008)</u> – Proposed Courtyard Hotel, East side of New King, west of Crooks, 5550 New King (PIN 88-20-08-276-006), Currently Zoned OM (Office Mixed Use) Zoning District

The petitioner Troy Elite Hospitality, LLC submitted the above referenced Special Use and Preliminary Site Plan application for the proposed 5-story, 138 room Courtyard. The 2.83-acre site is currently vacant and zoned OM.

The subject site is part of an overall 6.1 acre parcel that is controlled by a Conditional Rezoning application that was approved by City Council on October 8, 2012 (see attached). The portion of the site fronting on Crooks is zoned CB and has been developed with a stand-alone Panera Bread restaurant, a two-story mixed use building with drive through, and a vacant site on the corner of Crooks and Corporate (to be developed as a restaurant building). The Agreement states the following: "The OM parcel (designated as Phase II) will be limited to and developed as either an office building or a hotel as presented in the approval package".

The attached report prepared by Carlisle/Wortman Associates, Inc. (CWA), the City's Planning Consultant, summarizes the project. CWA prepared the report with input from various City departments including Planning, Engineering, Public Works and Fire. City Management supports the findings of fact contained in the report and the recommendations included therein.

Attachments:

- 1. Maps
- 2. Report prepared by Carlisle/Wortman Associates, Inc.
- 3. Traffic Impact Study, Prepared by Rowe Professional Services, dated July 19, 2019.
- 4. Traffic Impact Study, Prepared by Rowe Professional Services, dated August 14, 2019.
- 5. Memorandum prepared by OHM, dated August 28, 2019.
- 6. Memorandum prepared by OHM, dated October 1, 2019.
- 7. Conditional Rezoning Agreement, approved by City Council on October 8, 2012.

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PROPOSED RESOLUTION

<u>PUBLIC HEARING - SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File Number</u> <u>SU JPLN2019-0008)</u> – Proposed Courtyard Hotel, East side of New King, west of Crooks, 5550 New King (PIN 88-20-08-276-006), Section 8, Currently Zoned OM (Office Mixed Use) Zoning District

Resolution # PC-2019-11-

Moved by: Seconded by:

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval, pursuant to Articles 8 and 9 of the Zoning Ordinance, as requested for the proposed Courtyard Hotel, located on the east side of New King, west of Crooks, 5550 New King, Section 8, Currently Zoned OM (Office Mixed Use) District, be granted, subject to applicant the following:

) or
(denied, for the following reasons:) or
(postponed, for the following reasons:)

Yes: No:

MOTION CARRIED/FAILED

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1,189

GIS Online

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595

1,189Feet



Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.



1,189

GIS Online



Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.



117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

Date: November 5, 2019

Special Use and Preliminary Site Plan Review For City of Troy, Michigan

Applicant:	Steller Development
Project Name:	Courtyard
Plan Date:	September 24, 19
Location:	Crooks Road
Zoning:	OM, Office Mixed Use
Action Requested:	Preliminary Site Plan and Special Use Approval

PROJECT AND SITE DESCRIPTION

In 2012, the City conditionally rezoned the entire 6.1-acre site from PUD to CB, Commercial Business, and OM., Office Mixed Use. The parcel is located at the corner of Crooks Road, Corporate Drive, and New King Drive. The conditional rezoning permitted the site to be developed in multiple phases.

The first phase of the development, rezoned to CB, is the front part of the site that is adjacent to Crooks. Phase 1 development includes a Panera and two-story mixed use building with drive-through. Phase 2 is located at the rear of the site, adjacent to New King Drive was conditionally rezoned to OM. Phase 2 was conceptually approved for an office or hotel use, with the requirement that site development go through site plan and special use review. Phase 3, the pad at the hard corner of Corporate Drive and Crooks, was rezoned to CB, and was approved as a restaurant. Phase 3 will require a future site plan review by the Planning Commission.

Courtyard October 29, 2019

The Planning Commission is only considering a site plan and Special Use review for Phase 2. For Phase 2, the applicant is requesting special use and preliminary site plan approval for a five (5) story, 64-feet, one hundred and thirty-eight (138) room, Courtyard Hotel. The hotel will include direct access off New King and Corporate Drive.



Figure 1. – Subject site aerial photo

Size of subject property: 1.54 acres (Phase 2 portion of site)

<u>Current use of subject property:</u> Vacant

Proposed use of subject site: 5-story hotel

<u>Current Zoning:</u> The property is currently zoned OM, Office Mixed Use

Surrounding Property Details:

Direction	Zoning	Use
North	RC, Research Center	Office
South	OM, Office Mixed Use	Office
East	OM, Office Mixed Use	Vacant, former Met Hotel
West	RC, Research Center	Office

According to the Troy Master Plan – Future Land Use map, this area "Located at the intersection of two main arterial roads with a direct access ramp to I-75, North Troy is strategically located to serve as a major employment hub for Oakland County. North Troy currently is home to over 5,000 primarily daytime employees; in addition, there are over 2,000 households within one mile of the area. However, the area is dominated by single-use office buildings with limited interconnections, and few amenities for these workers and nearby residents.

Preparing North Troy for the next generation of growth will require a broader and more creative real estate strategy that will tap into regional trends and market opportunities in order to create a more vibrant, attractive, and flexible work environment. A compatible and vibrant mix of uses will create a life and vibrancy, provide interconnections and a relationship with the adjacent neighborhoods, reduce automobile trips, and enhance walkability by providing destinations."

Adding a compatible and vibrant use to the existing office fabric is consistent with the Master Plan.

NATURAL RESOURCES

The site was previously developed. Currently the site has been improved with buildings and prepared to accommodate the new development.

Items to be Addressed: None

BUILDING LOCATION AND SITE ARRANGEMENT

The proposed building is situated adjacent to New King Road with the front entrance oriented towards the the shared Phase 1 and 2 parking lot. The site includes pedestrian and vehicular connections to Phase 1 and future Phase 3 of the development. The hotel will include direct access off New King and Corporate Drive.

Items to be Addressed: None

AREA, WIDTH, HEIGHT, SETBACKS

OM, Office Mixed Use bulk requirements are set forth in section 4.17.

Table 1. – Site requirements and proposed dimensions

	Required	Building Site	Complies
Front (New King Drive)	10 feet	10.0 feet	Complies
Open Space	20%	28.5%	Complies
Building Coverage	40%	13.6%	Complies
Building Height	5 stories (75 feet)	5 story, 64.5 feet	Complies

The site plan complies will all bulk requirements.

Items to be Addressed: None

PARKING

Parking Calculations:

The parking calculations provided are included in *Table 2.,* below.

Table 2. – Required	and	provided	parking
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	Required	Provided
Hotel: One (1) space for each room, plus one (1) space per one (1) employee on the largest typical shift	133 Rooms + 5 total employees = 138 spaces	138 spaces (with shared parking)
Barrier Free	5	5
Bicycle Parking	2	3
Loading	1	0
Total	138 automobile + 2 bicycle	138 automobile + 3 bicycle

The 138 spaces include shared parking with the rest of the development. The parking has been reviewed by the Traffic Engineer Consultant, OHM, who notes that shared parking is sufficient. The loading space is a requirement that can be waived by Planning Commission.

Items to be Addressed: *None, if Planning Commission grants loading space deviation.*

TRAFFIC

The applicant completed a traffic study for the entire development which includes:

- 133 room hotel
- 8,500 sq/ft of office
- 2,176 sq/ft of medial office
- 2,000 sq/ft coffee shop with drive-through
- 4,400 sq/ft Panera bread with drive-through
- 4,500 sq/ft sit down restaurant

The traffic study concludes that 478 cars during the AM peak hour and 323 cars during the PM peak hour:

	A	M Peak I	Hour	PN	/I Peak Ho	our	Week Day
	In	Out	Total	In	Out	Total	
Total Trips	254	224	478	162	161	323	5,028

The trip generation will be distributed as follows:

AM Peak Hour

47% from and 24% to the north 16% from and 46% to the south 32% from and 17% to the east 5% from and 13% to the west PM Peak Hour 21% from and 46% to the north 41% from and 24% to the south 23% from and 26% to the east 15% from and 4% to the west

Based on existing level of services, the traffic study concludes that the applicant should construct a left turn lane at the intersection of Corporate Drive and Crooks Drive to mitigate additional and significant traffic delays caused by their development.

OHM has reviewed the traffic study and concludes:

OHM accepts the findings of the traffic impact study. The study shows that there are lengthy delays at the Crooks Road/I-75 off-ramp/Corporate Drive intersection and the development will significantly add to the delays at this intersection. The suggested eastbound left turn lane with 200' of storage on Corporate Drive will improve intersection operations to an acceptable level of service.

Items to be Addressed: Work with Engineering to provide required traffic improvements as noted in traffic study.

SITE ACCESS AND CIRCULATION

Vehicular access:

The hotel will include direct access off New King and Corporate Drive.

Pedestrian access:

There are direct pedestrian connections from both New King Road and a pedestrian spine that connects to the development that is adjacent to Crooks Road.

Items to be Addressed: None

LIGHTING

The applicant has provided a lighting (photometric) plan and lighting fixture details. The applicant is proposing a three (3) flagpole lights, 15 bollard lights, and two (2) two-head pole and one (1) one-headed pole lights. The photometrics comply with ordinance requirements.

Items to be Addressed: None

LANDSCAPING

The application includes a landscape plan and calculations.

	<u>Required:</u>	Provided:	Compliance:
Street Trees: The Ordinance requires that the greenbelt shall be landscaped with a minimum of one (1) deciduous tree for every thirty (30) lineal feet, or fraction thereof, of frontage abutting a public road right-of-way.	New King Road 550 LF = 19 trees 10 trees	19 trees	Compliant
Site landscaping: A minimum of twenty percent (15%) of the site area shall be comprised of hardscape and landscape material.	20%	28.5%	Compliant
Parking Lot Landscaping: 1 tree for every 8 parking spaces. Trees may be located adjacent to parking lot with planning commission approval.	138 spaces = 19 trees	19 trees	Compliant

Parking Lot Screening:	Screen 7 spaces	Alternative	Deciduous trees	Compliant
facing New King Road		landscape	planted every 10	
		plantings	feet and shrub	
			screening	

The applicant proposes a retaining wall and handrail between the sidewalk on New King and the building courtyard. The height of the wall varies in height. A profile of the retaining wall is shown on the plans.

Items to be Addressed: None

FLOOR PLANS AND ELEVATIONS

The applicant has submitted floor plans and elevations. Materials include a mix of "wall plank" panels, and fiber cement board. As set forth in Section 8.06.B. Development shall incorporate the following recognized best architectural building design practices:

- 1) Foster a lasting impact on the community through the provision of high-quality design, construction, and detailing.
- 2) Provide high quality, durable materials, such as but not limited to stone, brick, glass, and metal. E.I.F.S. or material equivalent shall only be used as an accent material.
- 3) Develop buildings with creativity that includes balanced compositions and forms.
- 4) Design roofs that are appropriate to the architectural style of the building and create an appropriate visual exterior mass of the building given the context of the site.
- 5) For commercial buildings, incorporate clearly defined, highly visible customer entrances using features such as canopies, porticos, arcades, arches, wing walls, ground plane elements, and/or landscape planters.
- 6) Include community amenities that add value to the development such as patio/ seating areas, water features, art work or sculpture, clock towers, pedestrian plazas with park benches or other features located in areas accessible to the public.

The applicant should bring to the meeting the building materials samples, and a color rendering or a 3-D model to evaluate the overall architecture program. The Planning Commission shall discuss the materials and elevations based upon the additional information provided at the meeting.

Items to be Addressed: Provide building material samples and a color rendering or a 3-D model to evaluate the overall architecture program.

STANDARDS FOR APPROVAL

Hotels are permitted subject to Special Use approval. For any Special Use, according to Section 9.02.D, the Planning Commission shall "...review the request, supplementary materials either in support or opposition thereto, as well as the Planning Department's report, at a Public Hearing established for that purpose, and shall either grant or deny the request, table action on the request, or grant the request subject to specific conditions."

Section 9.03 states that before approving any requests for Special Use approval, the Planning Commission shall consider:

- 1. Compatibility with Adjacent Uses. The Special Use shall be designed and constructed in a manner harmonious with the character of adjacent property and the surrounding area. In determining whether a Special Use will be harmonious and not create a significant detrimental impact, as compared to the impacts of permitted uses.
- 2. Compatibility with the Master Plan. The proposed Special Use shall be compatible and in accordance with the goals and objectives of the City of Troy Master Plan and any associated sub-area and corridor plans.
- 3. Traffic Impact. The proposed Special Use shall be located and designed in a manner which will minimize the impact of traffic, taking into consideration: pedestrian access and safety; vehicle trip generation (i.e. volumes); types of traffic, access location, and design, circulation and parking design; street and bridge capacity and, traffic operations at nearby intersections and access points. Efforts shall be made to ensure that multiple transportation modes are safely and effectively accommodated in an effort to provide alternate modes of access and alleviate vehicular traffic congestion.
- 4. Impact on Public Services. The proposed Special Use shall be adequately served by essential public facilities and services, such as: streets, pedestrian or bicycle facilities, police and fire protection, drainage systems, refuse disposal, water and sewage facilities, and schools. Such services shall be provided and accommodated without an unreasonable public burden.
- 5. Compliance with Zoning Ordinance Standards. The proposed Special Use shall be designed, constructed, operated and maintained to meet the stated intent of the zoning districts and shall comply with all applicable ordinance standards.
- 6. Impact on the Overall Environment. The proposed Special Use shall not unreasonably impact the quality of natural features and the environment in comparison to the impacts associated with typical permitted uses.
- 7. Special Use Approval Specific Requirements. The general standards and requirements of this Section are basic to all uses authorized by Special Use Approval. The specific and detailed requirements relating to particular uses and area requirements must be also satisfied for those uses.

The site was conceptually approved in 2012 for an office or a hotel use, as per the Conditional Rezoning Agreement. The Planning Commission desires to review each lodging use on a caseby-case basis to ensure the architectural and material quality is consistent with the stated intention of the Master Plan and Zoning Ordinance; that such use does not oversaturate the Courtyard October 29, 2019

market especially considering the difficulty in retrofitting hotels for future uses; and ensure that a hotel development does potentially negatively impact adjacent properties.

Overall we find that the Special Use Standards have been met:

- The site is master planned as North Troy, which calls for inclusion of a "compatible and vibrant mix of uses will create a life and vibrancy, provide interconnections and a relationship with the adjacent neighborhoods, reduce automobile trips, and enhance walkability by providing destinations." Adding a compatible and vibrant use to the existing office fabric is consistent with the Master Plan.
- The adjacent uses are office, which is compatible with hotel uses.
- The applicant has submitted a traffic study which notes needed improvements.

RECOMMENDATION

Overall, we support the hotel use and find that it meets the Special Use standards. In addition to comments received at the public hearing, the Planning Commission discussion should include:

- 1. Loading space deviation
- 2. Material use and elevations

We recommend approval of the site plan and Special Use with the following items to be addressed by the applicant:

- 1. Work with engineering department to provide required traffic improvements as noted in traffic study.
- 2. Any changes to building material use and elevations.

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP

CARLISLE/WORTMAN ASSOC., INC Lauren Carlson Planner

COURTYARD TROY PLAZA CITY OF TROY, OAKLAND COUNTY, MICHIGAN SITE PLANS

APPLICANT

TROY ELITE HOSPITALITY, LLC 2600 AUBURN ROAD, SUITE 240 AUBURN HILLS, MI 48326

CONTACT: JIMMY ASMAR PHONE: (248) 419-5555

STANDARD LEGEND				
DESCRIPTION	PROPOSED	EXISTING		
BUILDING				
STORM SEWER	— — 12 " — —	12"		
SANITARY SEWER	S	S		
WATER		W		
GAS LINE	G	G		
ELECTRIC LINE	——————————————————————————————————————	———— E ———		
TELEPHONE LINE	T	T		
MANHOLE	•	0		
CATCH BASIN	A 1	0 0		
ENDSECTION		\square		
FIRE HYDRANT	¥	φ.		
GATE VALVE & WELL	•	\otimes		
UTILITY POLE	ſ	<i></i>		
UTILITY RISER				
SIGN				
LIGHT POLE	•	*		
CURB & GUTTER				
FENCE	xxx	xx		
SILT FENCE				
TREE - DECIDUOUS	AS NOTED ON PLANS			
TREE LINE				
SPOT ELEVATION	+100.00	×100.00		
CONTOUR LINE	100	100		
SECTION CORNER		•		
FOUND PROPERTY IRON		0		
SET PROPERTY IRON		•		
GAS METER		©		
ELECTRICAL METER		E		
TELEPHONE RISER		TPED IX		
MAILBOX		MB		
SOIL BORING LOCATION		¢		



THE LOCATION OF ALL UNDERGROUND UTILITIES AS SHOWN ON THESE DRAWINGS ARE BASED ON RECORDS PROVIDED BY THE UTILITY OWNERS AND VISIBLE EVIDENCE OBTAINED IN THE FIELD. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED TO THE COMPLETENESS OR ACCURACY THEREOF.

ARCHITECT

TSA HOSPITALITY 1420 WASHINGTON BLVD., #430 DETROIT, MI 48226

> CONTACT: BENJAMIN BERR PHONE: (313) 974-6456



DRAWING INDEX

C1.0 COVER SHEET

- C2.0 SURVEY PLAN
- C3.0 OVERALL SITE LAYOUT PLAN
- C3.1 SITE LAYOUT AND PAVING PLAN
- C3.2 SITE PHOTOMETRIC PLAN
- C3.3 CONCEPTUAL RESTAURANT AREA PLAN
- **GRADING PLAN** C4.0
- C4.1 **GRADING SECTION**
- C5.0 UTILITY PLAN
- SITE DETAILS C6.0
- L1.0 LANDSCAPE PLAN AND DETAILS

PLANS PREPARED BY TSA ARCHITECTS

COURTYARD FIRST LEVEL FLOOR PLAN A-101

- A-102 SECOND LEVEL FLOOR PLAN
- A-103 THIRD LEVEL FLOOR PLAN
- A-104 FOURTH LEVEL FLOOR PLAN
- A-105 FIFTH LEVEL FLOOR PLAN
- A-201 EXTERIOR ELEVATIONS
- A-202 EXTERIOR ELEVATIONS



BEGINNING. PARCEL 2

FEET TO THE POINT OF BEGINNING.

CHICAGO TITLE INSURANCE COMPNAY COMMITMENT NO. 171000605 REV #1 EFFECTIVE DATE 01-03-19 (AS MARKED ON TITLE COMMITMENT)

10107, PAGE 225. AS SHOWN PAGE 510. AS SHOWN

46278, PAGE 97, AS SHOWN

THEREOF WHICH ARE RECITED IN RECIPROCAL 498. BLANKET EASEMENT

CIVIL ENGINEER

STELLAR DEVELOPMENT, LLC 2600 AUBURN ROAD, SUITE 160 AUBURN HILLS, MI 48326

CONTACT: ANDREW ANDRE, P.E. PHONE: (248) 419-5550

ANDREW ANDRE, P.E. MICHIGAN PE 47380 SEAL PERTAINS TO PLANS AS PREPARED BY STELLAR DEVELOPMENT, LLC FURNISHED DESCRIPTION **FLOODPLAIN INFORMATION** CITY OF TROY OAKLAND COUNTY, MICHIGAN MAP NUMBER: 26125 C0531F EFFECTIVE DATE: 9-29-2006 FLOOD ZONE: X 'HE LAND REFERRED TO IN THIS COMMITMENT IS DESCRIBED A AREA OF MINIMAL FLOOD HAZARD (PER FIRM) LAND SITUATED IN THE CITY OF TROY, COUNTY OF OAKLAND, ZONING INFORMATION A PARCEL OF LAND LOCATED IN THE NORTHEAST 1/4 OF SECTION 8, TOWN 2 NORTH, RANGE 11 EAST, CITY OF TROY, ZONED- OM (OFFICE MIXED USE) SETBACKS-OAKLAND COUNTY, MICHIGAN DESCRIBED AS: COMMENCING AT THE EAST 1/4 CORNER OF SECTION 8, THENCE NORTH 02 FRONT: - 10' DEGREES 30 MINUTES 26 SECONDS WEST 715.45 FEET ALONG REAR: - 30' THE EAST LINE OF SECTION 8; THENCE SOUTH 87 DEGREES 29 SIDE: - 20' MIN. TOTAL OF 60' MINUTES 34 SECONDS WEST 350.00 FEET TO THE POINT OF BEGINNING; THENCE SOUTH 02 DEGREES 30 MINUTES 26 SECONDS EAST 170.00 FEET; THENCE SOUTH 87 DEGREES 29 MINUTES 34 SECONDS WEST 8.50 FEET; THENCE SOUTH 02 DEGREES 30 MINUTES 26 SECONDS EAST 192.00 FEET; THENCE SOUTH 87 DEGREES 29 MINUTES 34 SECONDS WEST 129.49 BENCHMARKS FEET; THENCE SOUTH 02 DEGREES 30 MINUTES 26 SECONDS EAST 62.15 FEET; THENCE SOUTH 19 DEGREES 21 MINUTES 58 B.M. #1 - ARROW ON HYD. ON N. SIDE OF CORPORATE DR. SECONDS EAST 39.85 FEET; THENCE 412.77 FEET ALONG THE ON SOUTH SIDE OF OF PROPERTY ARC OF A CURVE TO THE RIGHT ALSO BEING THE EASTERLY ELEV. 804.32 NAVD 88 RIGHT OF WAY LINE OF NEW KING STREET (70 FEET WIDE), HAVING A RADIUS OF 352.68 FEET A CHORD BEARING AND B.M. #2 -SET BM ON FND IRON ON NW COR OF PROPERTY ON DISTANCE OF NORTH 36 DEGREES 02 MINUTES 17 SECONDS EAST SIDE OF KING ROAD WEST 389.61 FEET; THENCE NORTH 02 DEGREES 30 MINUTES 26 SECONDS WEST 137.51 FEET; THENCE NORTH 87 DEGREES 29 MINUTES 34 SECONDS EAST 341.65 FEET TO THE POINT OF ELEV. 816.74 NAVD 88 A PARCEL OF LAND LOCATED IN THE NORTHEAST 1/4 OF UTILITY NOTE SECTION 8, TOWN 2 NORTH, RANGE 11 EAST, CITY OF TROY, OAKLAND COUNTY, MICHIGAN, DESCRIBED AS: COMMENCING AT THE UTILITY LOCATIONS AS HEREON SHOWN ARE BASED ON THE EAST 1/4 CORNER OF SECTION 8; THENCE NORTH 02 FIELD OBSERVATIONS AND A CAREFUL REVIEW OF MUNICIPAL DEGREES 30 MINUTES 26 SECONDS WEST 225.27 FEET ALONG AND UTILITY RECORDS. HOWEVER, IT IS NOT POSSIBLE TO THE EAST LINE OF SECTION 8; THENCE SOUTH 87 DEGREES 29 DETERMINE THE PRECISE SIZE, LOCATION, DEPTH, PRESSURE MINUTES 34 SECONDS WEST 90.00 FEET TO THE POINT OF OR ANY OTHER CHARACTERISTICS OF UNDERGROUND UTILITIES BEGINNING; THENCE SOUTH 87 DEGREES 29 MINUTES 34 TANKS OR SEPTIC FIELDS WITHOUT EXCAVATION. THEREFORE. SECONDS WEST 248.90 FEET ALONG THE NORTHERLY RIGHT OF WE CANNOT GUARANTEE THE ACCURACY OF COMPLETENESS O WAY LINE OF CORPORATE DRIVE (120 FEET WIDE); THENCE THE BURIED UTILITY INFORMATION HEREON SHOWN. THE 130.18 FEET ALONG THE ARC OF A CURVE TO THE RIGHT CONTRACTOR SHALL CALL MISS DIG (1-800-482-7171) A HAVING A RADIUS OF 352.68 FEET CHORD BEARING AND MINIMUM OF THREE WORKING DAYS PRIOR TO ANY EXCAVATION. DISTANCE OF NORTH 80 DEGREES 08 MINUTES 30 SECONDS T SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO WEST 129.44 FEET; THENCE NORTH 19 DEGREES 21 MINUTES VERIFY THESE UTILITY LOCATIONS PRIOR TO CONSTRUCTION AND 58 DEGREES WEST 39.85 FEET; THENCE NORTH 02 DEGREES MAKE EVERY EFFORT TO PROTECT AND/OR RELOCATE THEM AS MINUTES 26 SECONDS WEST 62.15 FEET; THENCE NORTH 87 REQUIRED. THE CONTRACTOR SHALL NOTIFY THE DEGREES 29 MINUTES 34 SECONDS EAST 398.00 FEET; THENCE ENGINEER/SURVEYOR AS SOON AS POSSIBLE IN THE EVENT A SOUTH 02 DEGREES 30 MINUTES 26 SECONDS EAST 128.18 DISCREPANCY IS FOUND. UTILITY CONTACTS SCHEDULE 'B' EXCEPTIONS TELEPHONE GAS CONSUMERS ENERGY AT&T 421 EAST GRAND RIVER 1030 FEATHERSTONE HOWELL, MI. 48843 PONTIAC, MI 48342 (517) 548-6911 (248) 858-4511 N) EASEMENT FOR PUBLIC UTILITIES GRANTED TO THE CITY OF TROY AS SET FORTH IN INSTRUMENT RECORDED IN LIBER ZONING WATER/SEWER CITY OF TROY CITY OF TROY 0) EASEMENT FOR WATER MAIN GRANTED TO THE CITY OFTROY ZONING & PLANNING AS SET FORTH IN INSTRUMENT RECORDED IN LIBER 11493, 500 WEST BIG BEAVER RD 4693 ROCHSTER RD TROY, MICHIGAN 48084 TROY, MICHIGAN 48085 (248) 524-3392 (248) 524-3300 P) TERMS, CONDITIONS, AND PROVISIONS WHICH ARE RECITED IN CONDITIONAL REZONING AGREEMENT AS RECORDED IN LIBER ELECTRIC DTE Q EASEMENTS AND THE TERMS, CONDITIONS, AND PROVISIONS NW PLANNING AND DESIGN 37849 INTERCHANGE DR. EASEMENTAGREEMENT AS RECORDED IN LIBER 51970, PAGE FARMINTGON HILLS, MI 48335

(248) 427-2987

2600 AUBURN ROAD, SUITE 160 AUBURN HILLS, MI 48326 PH: 810-444-7815 EPARED UNDER DIRECTIC ANDREW ANDRE, P.E. MI PE# 47380 STELLAR DEVELOPMENT, LLC TROY ELITE HOSPITALITY, LI 2600 AUBURN ROAD, SUITE 240 AUBURN HILLS, MI 48326 PH 248-419-5550 FX 248-553-4218 COURTYARE BY MARRIO YARD COURT ROY WHE BERY ATTEAPT HIS EEN MUE TO FROLUE ERFOR FREE COLMAN'S, IT S THE REPORBLITY OF THE BLUER MUCH TO HERY ALL ESSING CONTINGS, DIAGNOS, ETHALIER AND'AT THE CLART MY ERFORTING FRECEDING, DIAGNOS, ETHALIER CORPANICE, OF THESE DOCUMENTS THE APOTHECT TAKES HLL REPORTING TO AND STATUTION AND ADDITION TAKES HLL REPORTING TO AND STATUTION THE APOTHECT TAKES HLL REPORTING TO CHARGES AND OF AND STATUS BIT SHEEHED OF THESE REPORTING TO BE THESE DOCUMENTS THE APOTHED FWY OF CLART SERVICE THE CESSIN AND OF AND STATUS BIT SHEEKET TEAM THE REPORTING TO SHELL REPORTING AND NO ADDITIONL FRESTOR OF PARTES MAY USE THIS CESSIN AND OF DRAWNES FOR ANY OTHER PROFENC OF SHELL RECORDING THE ADDITIONL FRESTOR OF PARTES MAY USE THE CESSIN AND OF DRAWNES FOR ANY OTHER PROFENC WITHOUT THE CONSENT OF STELLAR DRAWNES FOR ANY OTHER PROFENC WITHOUT THE CONSENT OF STELLAR ISSUED FOR DATE PRELIM SPA 02/18/19 PRELIM SPA 05/09/19 PRELIM SPA 09/24/19 DATE : DRAWN: ACA CHECKED: ACA SCALE : NO SCALE JOB NO : SHEET TITLE : COVER SHEET SHEET C1.0



CITY OF TROY

FLOOD ZONE: X

COMMITMENT)

TELEPHONE AT&T (517) 548–6911

<u>ZONING</u> CITY OF TROY

ELECTRIC DTE

12-18-18.



40' 80' 120' SCALE: 1'' = 40'

SURVEY PROVIDED BY D&M SITE INC.



Surveying. Inspection. Testing. Engineering 401 BALSAM STREET PO BOX 159, CARROLLTON, MICHIGAN 48724 PHONE (989) 752-6500 • FAX (989) 752-6600





THE LOCATION OF ALL UNDERGROUND UTILITIES AS SHOWN ON THESE DRAWINGS ARE BASED ON RECORDS PROVIDED BY THE UTILITY OWNERS AND VISIBLE EVIDENCE OBTAINED IN THE FIELD. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED TO THE COMPLETENESS OR ACCURACY THEREOF.

PAVEMENT INFORMATION

BITUMINOUS PAVEMENT
CONCRETE PAVEMENT
STAMPED CONC. PAVEN

UTILITY NARRATIVES

<u>STORM MANAGEMENT</u> THE SURFACE WATER RUNOFF WILL BE COLLECTED VIA CATCH BASINS AND ROUTED TO THE EXISTING STORM SYSTEM THAT WAS INSTALLED AS PART OF THE FIRST PHASE OF THE DEVELOPMENT. AN UNDERGROUND ADS STORMTECH SYSTEM WAS SIZED FOR THE OVERALL DEVELOPMENT AND WAS INSTALLED UNDER THE PARKING LOT DURING THE INITIAL PHASE.

WATER SYSTEM THERE IS AN EXISTING WATER THAT WAS INSTALLED AS PART OF THE INITIAL PHASE OF THE DEVELOPMENT. THE WATER MAIN IS LOCATED ALONG THE EASTERN SIDE OF THE HOTEL AREA AND WILL PROVIDE FOR CONNECTION AND EXTENSION TO THE PROPERTY. THE WATER MAIN WILL SERVICE THE HOTEL FOR BOTH DOMESTIC SERVICE AND FIRE SUPPRESSION SERVICES. THERE IS SOME MODIFICATION TO THE EXISTING SYSTEM THAT WILL BE REQUIRED.

<u>SANITARY SEWER</u> EXISTING SANITARY SEWER LEAD HAS BEEN PROVIDED ALONG THE EXISTING ACCESS DRIVE. CONNECTION TO THE EXISTING SYSTEM WILL BE PROVIDED.





<u>GENERAL NOTES</u>

- 1. REFER TO ARCHITECTURAL PLANS TO VERIFY BUILDING DIMENSIONS.
- 2. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SECURE ALL PERMITS AND POST ALL BONDS PRIOR TO CONSTRUCTION, OR ENSURE THAT ALL REQUIRED PERMITS AND BONDS HAVE BEEN OBTAINED PRIOR TO
- 3. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS PRIOR TO CONSTRUCTION BY CALLING MISS-DIG AT 1-800-482-7171 AT LEAST 3 WORKING DAYS PRIOR TO EXCAVATION.
- 4. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR THE PROTECTION OF ALL EXISTING UTILITIES DURING CONSTRUCTION. ALL UTILITIES DAMAGED DURING CONSTRUCTION SHALL BE REPAIRED WITH LIKE MATERIAL. THE EXACT LOCATION OF EXISTING UTILITIES SHALL BE LOCATED BY HAND DIGGING.
- 5. DIMENSIONS ARE TO FACE OF CURB, OUTSIDE FACE OF BUILDING,
- 6. ALL PAVING MATERIALS AND OPERATIONS SHALL BE IN CONFORMANCE WITH THE CITY OF TROY AND MICHIGAN DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 7. PARKING STALL SPACES TO BE 4" SOLID WHITE PAINT STRIPES. HANDICAP PARKING TO BE 4" SOLID BLUE PAINT STRIPES W/ BARRIER
- 8. ALL WORK SHALL CONFORM TO THE CURRENT CITY OF TROY STANDARDS AND



CONSTRUCTION.

- EDGE OF PAVEMENT, CENTER OF STRUCTURE OR OTHERWISE INDICATED.
- FREE STRIPPING OF 4" SOLID BLUE @ 2' O.C. ON 45° ANGLE.
- SPECIFICATIONS.



0'	4	0'	8	o'	120
SCALE:	1" = 4	0'			

PLAN

SHEET

C3.0

PAVEMENT

CONC. PAVEMENT / PAVERS





0'	20'	40'	60'
SCALE:	1" = 20'		

PAVING PLAN

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SHEET

ALL TRAFFIC SIGNAGE SHALL COMPLY WITH THE CURRENT MMUTCD STANDARDS

R8-31





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PAVEMENT INFORMATION



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BITUMINOUS PAVEMENT

STAMPED CONC. PAVEMENT / PAVERS

NOTE:



Know what's **below. Call** before you dig.



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GRADING PLAN

SHEET

C4.0

GRADING NOTES

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF TROY AND MICHIGAN DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS.

BENCHMARKS

B.M. #2 -SET BM ON FND IRON ON NW COR OF PROPERTY ON EAST SIDE OF KING ROAD

ELEV. 804.32 NAVD 88

ELEV. 816.74 NAVD 88

B.M. #1 - ARROW ON HYD. ON N. SIDE OF CORPORATE DR. ON SOUTH SIDE OF OF PROPERTY

- 2. SPOT ELEVATIONS INDICATE TOP OF PAVEMENT UNLESS OTHERWISE INDICATED:
 - TC = TOP OF CURB TP = TOP OF PAVEMENT
- 3. CURB AND GUTTER LEGEND
 - (S) = STANDARD CURB AND GUTTER
 - (R) = REVERSE SLOPE CURB AND GUTTER
 - (T)= 5' TRANSITION FROM STANDARD TO REVERSE
 - (E) = CURB END TRANSITION
 - (G) = GUTTER PAN
- 4. REFER TO LANDSCAPE PLAN FOR LANDSCAPE TREATMENT AND PLACEMENT.









GRADING SECTION

C4.

SHEET







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WATER USE CALCULATIONS R.E.U. USAGE AREA UNIT FACTOR 133 rooms 0.38 units per room 50.54 Hotel 50.54 Rounded= 51.00 POPULATION EQUIVALENT = 3.5 Persons per R.E.U. POPULATION = 178.5 Persons AVERAGE FLOW RATE = Gallons per capita per day 90 AVERAGE FLOW = 16,065 GPD = 0.016065 MGD PEAK FACTOR = 18 + SQRT(P) / 4 + SQRT(P) = 4.17TOTAL DESIGN MAXIMUM FLOW = 66,921 GPD = 0.06692 MGD SANITARY USE CALCULATIONS USAGE AREA UNIT FACTOR R.E.U.

Hotel	133	rooms		0.38	units per	room		50.54 50.54
							Rounded=	51.00
POPULATION EQUIVALENT =	3.5	Person	s per R.I	E.U.				
POPULATION =	178.5	Person	s					
AVERAGE FLOW RATE =	90	Gallons	per cap	ita per	day			
AVERAGE FLOW =	16,065	GPD	=	0.024	CFS			
PEAK FACTOR =	18 + SQR	T(P) / 4	+ SQRT	(P)	=	4.17		
PEAK FLOW =	66,920.94	GPD	=	0.100	CFS	6"@2.	00%(cfs) =	1.03



THE LOCATION OF ALL UNDERGROUND UTILITIES AS SHOWN ON THESE DRAWINGS ARE BASED ON RECORDS PROVIDED BY THE UTILITY OWNERS AND VISIBLE EVIDENCE OBTAINED IN THE FIELD. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED TO THE COMPLETENESS OR ACCURACY THEREOF.



SCALE : 1"=20' JOB NO : SHEET TITLE : UTILITY PLAN 60' SHEET 60'

PRELIM SPA

PRELIM SPA

DATE :

DRAWN: ACA

CHECKED: ACA

05/09/19

09/24/19







PLANTING SCHEDULE QUANTITY KEY BOTANICAL NAME COMMON NAME PERENNIAL PLANTINGS 125 HH HEMERCALUS HYPERION HYPERION DAYLILY 18 KRL CALAMGROSTIS X ACUTIFOLIA 'KARL FORSTER' KARL FORSTER'S REED GRASS DECIDUOUS TREES GT GLEDITSIA T.I. 'SKYLINE' SKYLINE HONEYLOCUST TC | TILIA CORDATA 'GREENSPIRE' GREENSPIRE LINDEN GREEN MOUNTAIN SUGAR MAPLE AS ACER SACCHARUM 'GREEN MOUNTAIN' QR QUERCUS RUBRA RED OAK LS LIQUIDAMBAR STYRACIFLUA AMERICAN SWEETGUM PE PLANTUS X EXCLAMATION LONDON PLANETREE SHRUBS VB VIBURNUM DENTATUM 'BLUE MUFFIN' BLUE MUFFIN ARROWWOOD VIBURNU 11 12 GOLDFLAME SPIRAEA SG SPIRAEA JAPONICA 'GOLDFLAME' 12 DWARF BURNING BUSH EC EUONYMUS ALATUS 'COMPACTUM' BM BUXUS MICROPHYLLA JAPONICA 'WINTER GEM' WINTER GEM BOXWOOD BT |BERBERIS THUNBERGII ATROPURPUREA CRIMSON PYGMY BARBERRY 12 JP J. CHINENSIS 'PFITZERANA' PFITZER JUNIPER 11 SY TAXUS X.MEDIA 'SEBIAN' SEBIAN YEW

KEY NOTES

- (S) STEEL EDGING
- $\langle M \rangle$ shredded bark mulch

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SPACING VARIES

2" MIN. ORGANIC

2" DEEP FILLED

WITH SPECIFIED

- UNDISTURBED EARTH

PLANTING MIX

LEAF COMPOST

-0

PERENNIAL PLANTING BED

4" STEEL LANDSCAPE EDGE

NO SCALE

SCALE: NONE

GENERAL NOTES

- 1. CONTRACTOR TO PROVIDE DESIGN AND INSTALLATION OF UNDERGROUND IRRIGATION SYSTEM IN ACCORDANCE WITH PROJECT SPECIFICATIONS AND REGULATORY AGENCY REQUIREMENTS. ALL LANDSCAPING AND GRASS AREAS TO BE IRRIGATED. IRRIGATION CONTROL PANEL SHALL BE LOCATED WITHIN THE HVAC ENCLOSURE.
- 2. ALL GREEN SPACES AND PLANTING AREAS SHALL BE IRRIGATED.
- 3. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SECURE ALL PERMITS AND POST ALL BONDS PRIOR TO CONSTRUCTION.
- 4. REFER TO PLUMBING PLANS FOR LOCATION OF IRRIGATION METER.
- 5. ALL GRASS AREAS TO BE SODDED.
- 6. SPACE ALL SHRUBS AT 5-FEET ON CENTER UNLESS OTHERWISE INDICATED ON THE PLANS
- 7. ALL DISTURBED LAWN AREAS SHALL BE RESTORED TO AT LEAST PREVIOUS CONDITION IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- 8. THE GENERAL CONTRACTOR SHALL INCLUDE TOPSOIL IN BASE BID. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED AS TO THE AMOUNT OF TOPSOIL AVAILABLE ON-SITE.



GUY WIRE (2-#12 TWISTED)-APPROVED TREE WRAP — STEEL T-POST (8 FEET LONG)-5" MIN. SHREDDED BARK TAPER BARK TO TRUNK -MOUNDED EARTH SAUCER -

PLANTING MIXTURE -----

EARTH

REMOVE BURLAP AND LACING FROM TOP 1/3 OF EARTH BALL -SET BALL ON UNDISTURBED -

STAKES 18" BELOW TREE PIT INTO UNDISTURBED SOIL ----

SCALE: NONE

	SIZE
	2YD#2CONT
	3 GAL CONT
	2.5" B&B
Л	24" B&B
	24" B&B



DECIDUOUS TREE PLANTING

DESCRIPTION THIS WORK SHALL CONSIST OF PROVIDING ALL NECESSARY MATERIALS, LABOR, EQUIPMENT, TOOLS AND SUPERVISION REQUIRED FOR THE EXECUTION AND GUARANTEE OF ALL PLANTINGS AND RELATED WORK AS SHOWN ON THE DRAWINGS.

PLANT MATERIALS SHALL CONFORM TO THE SIZES STATED ON THE PLANT LIST AND SHALL BE OF A MINIMUM SIZE OR LARGER. ALL MEASUREMENTS OF SPREAD, CALIPER, BALL SIZE, TRUNK CROWN RATIO, QUALITY DESIGNATIONS, ETC., SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE "ANSI STANDARDS FOR NURSERY STOCK". PLANT MATERIAL SHALL BE NURSERY GROWN AND INSPECTED BY THE OWNER'S REPRESENTATIVE AT THE SITE PRIOR TO PLANTING. THE OWNER'S REPRESENTATIVE RESERVES THE RIGHT TO REJECT ANY PLANT MATERIAL AT ANY TIME.

NURSERY STOCK SHALL BE PREPARED FOR SHIPMENT IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT ANSI SPECIFICATION Z60.1 AND SHALL BE ENCLOSED OR COVERED DURING TRANSPORTATION TO PREVENT DRYING.

SITE PREPARATION

THE CONTRACTOR SHALL VERIFY ALL EXISTING AND PROPOSED UTILITY LOCATIONS PRIOR TO CONSTRUCTION AND PROTECT AND REPAIR UTILITIES ENCOUNTERED DURING CONSTRUCTION WHETHER SHOWN ON THE PLANS OR

INDIVIDUAL HOLES SHALL BE CENTERED AT STAKED PLANT LOCATIONS. CONTRACTOR IS TO STAKE PRIOR TO PLACEMENT OF PLANT MATERIAL AND OBTAIN APPROVAL FROM THE OWNER'S REPRESENTATIVE. PLANTING HOLES SHALL BE DUG LARGE ENOUGH TO PERMIT PLACING PREPARED TOPSOIL 18" LATERALLY BEYOND THE ENDS OF THE ROOT BALLS FOR SHADE AND EVERGREEN TREES AND 6" LATERALLY FOR SHRUBS UNLESS OTHERWISE SPECIFIED.

EXCAVATED MATERIAL SHALL BE REMOVED FROM THE SITE AT THE TIME THE HOLE IS DUG. THE PLANTING HOLE SHALL BE BACKFILLED WITH PREPARED TOPSOIL THE SAME DAY THEY ARE DUG.

TOPSOIL SHALL AT FERTILE, FRIABLE NATURAL TOPSOIL OF CLAY LOAM CHARACTER CONTAINING AT LEAST 5% BUT NOT MORE THAN 20% BY WEIGHT OF ORGANIC MATTER WITH A PH RANGE FROM 6.0 TO 7.0. TOPSOIL SHALL BE FREE OF CLAY LUMPS, COURSE SAND, STONES, PLANT ROOTS, STICKS OR OTHER FOREIGN MATTER.

CARE FOR PLANTS BEFORE PLANTING

PLANTS DESIGNATED "BB" SHALL BE BALLED AND BURLAPPED WITH FIRM NATURAL BALLS OF EARTH. CRACKED, LOOSENED OR BROKEN BALLS SHALL NOT BE PLANTED. THEY SHALL BE MARKED WITH SPRAY PAIN AND IMMEDIATELY REMOVED FROM THE JOB SITE. IMMEDIATELY FOLLOWING DELIVERY AT THE JOB SITE, ALL PLANTS THAT WILL NOT BE PLANTED THAT SAME DAY SHALL BE "HEELED IN" WITH SHREDDED BARK OR MOIST SOIL AND KEPT MOIST UNTIL PLANTED.

THE TRUNKS AND BRANCHES OF ALL TREES SHALL BE PROTECTED FROM INJURY OF ANY KIND DURING ALL OPERATIONS. THE OWNER'S REPRESENTATIVE SHALL REJECT ANY TREES THAT ARE INJURED.

<u>PLANTING</u>

THE CONTRACTOR IS RESPONSIBLE FOR PLANTING MATERIALS PLUMB. SET THE TOP OF THE ROOT BALL AT OR SLIGHTLY HIGHER THAN THE SURROUNDING GRADE. PLANTS SHALL BE FACED TO GIVE THE BEST APPEARANCE OR RELATIONSHIP TO ADJACENT STRUCTURES. NO FILLING WILL BE PERMITTED AROUND TRUNK OR STEMS. WHEN THE PLANT HAS BEEN PROPERLY SET, THE HOLE SHALL BE BACKFILLED TO 1/2 THE DEPTH OF THE BALL WITH PREPARED TOPSOIL MIXTURE, FIRMLY PACKED AND WATERED-IN AT TIME OF PLANTING. LOOSED AND REMOVE BURLAP AND LACING FROM UPPER 1/3 OF THE ROOT BALL. BACKFILL WITH PREPARED TOPSOIL, WHICH AFTER COMPACTION IS FLUSH WITH THE SURROUNDING GROUND.

MULCHING

ALL PLANT MATERIAL SHALL BE ENCIRCLED WITH A 5" MINIMUM COVERING OF NON-DYED SHREDDED BARK MULCH TO 6" OUTSIDE THE PLANTING HOLE, TAPERING MULCH TO 2" AROUND THE TRUNK OF ALL PLANTS. SUBMIT SAMPLE TO THE OWNER'S REPRESENTATIVE FOR APPROVAL BEFORE PLACEMENT. WOOD CHIPS SHALL NOT BE ALLOWED ON THIS JOB.

WRAPPING DECIDUOUS TREES

TREE WRAP SHALL COVER TRUNKS OF ALL DECIDUOUS TREES BEGINNING BELOW THE SOIL LINE JUST ABOVE THE ROOTS. TIE WITH TWINE IN 5 PLACES, INCLUDING THE TOP AND BOTTOM OF WRAPPING. MASKING TAPE OR WIRE WILL NOT BE ALLOWED.

BRACING AND GUYING

GUYING SHALL BE EMPLOYED TO PREVENT LEANING OR LOOSENING OF THE TREE FROM THE BALL. BRACING MATERIAL SHALL BE T-POSTS PAINTED GREEN. GUYING MATERIAL SHALL BE 12 GAUGE WIRE AND GARDEN HOSE 1/2 INCH DIAMETER. GUY WIRE SHALL BE ENCASED IN HOSE TO PREVENT DIRECT CONTACT WITH THE TREE. DECIDUOUS TREES SHALL BE BRACED OR GUYED IMMEDIATELY AFTER THE TREE WRAPPING IS COMPLETE.

STEEL LANDSCAPE EDGING

4" STEEL LANDSCAPE EDGING SHALL BE USED ON THIS PROJECT. ALUMINUM OR PLASTIC EDGING WILL NOT BE ALLOWED.

<u>PRUNING</u>

UPON COMPLETION, ALL PLANT MATERIAL MUST BE PRUNED. THE AMOUNT OF PRUNING SHALL BE LIMITED TO THE MINIMUM NECESSARY TO REMOVE DEAD OR INJURED BRANCHES AND TO COMPENSATE FOR THE LOSS OF ROOTS FROM TRANSPLANTING. ALL CUTS SHALL BE MADE FLUSH LEAVING NOT STUBS. PRUNING PAIN SHALL NOT BE USED.

FINISHING AND CLEANING UP

IMMEDIATELY UPON COMPLETION OF THE WORK, THE CONTRACTOR SHALL CLEAN UP THE AREA OF SURPLUS MATERIALS. THE CONTRACTOR SHALL REPAIR AND RE-ESTABLISH TURF IN RUTTED AREAS.

<u>WARRANTY</u> THE LANDSCAPE INSTALLATION CONTRACTOR SHALL REPLACE ALL UNHEALTHY VEGETATION AND PLANTINGS WITHIN ONE (1) YEAR OF INITIAL PLANTING OR SUBSEQUENT PLANTING PERIOD.

LANDSCAPE REQUIREMENTS

PARKING LOT LANDSCAPING (P)

REQUIRED 1 TREE / 8 PARKING SPACES W/ ISLANDS 200 S.F. MINIMUM REQUIRED PARKING SPACES = 138 138 / 8 = 17.2 = 18 TREES PROVIDED = 18 TREES

GREENBELT LANDSCAPING

REQUIRED 1 TREE / 30 L.F. OF FRONTAGE ON PUBLIC R.O.W.

NEW KING STREET (N) R.O.W. = 550.28'

REQUIRED = 550.28 / 30 = 18.3 = 19 TREESPROVIDED = 19 TREES

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2 EXTERIOR ELEVATION - SOUTH

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1 EXTERIOR ELEVATION - EAST








THE THINK SHOP н



01 ALLURA - CEMENT BOARD SMOOTH VERTICAL PANEL "SNOW"



02 NICHIHA - CEMENT BOARD VINTAGEWOOD "BARK"



04 INTRUMESCENT PAINT -AMMA 2605 "DARK BRONZE"



COLOR TO MARCH ADJACENT WALL



01ALT BRICK - BRICKTECH "WINTERBOURNE"



03 NICHIHA - CEMENT BOARD **ILLUMINATION SERIES** "GECKO"



06- ROOF 154'-6"

04-FOURTH FLOOR 134'-0"

03-THIRD FLOOR 124'-0"

02-SECOND FLOOR 114'-0"

01-GROUND FLOOR 100'-0"





07 PAC-CLAD -CUSTOM FABRICATED METAL METAL SHADOW BOXES AT WINDOWS "DURANAR BONEWHITE"









THE THINK SHOP A R C H I T E C T S

November 7, 2019

City of Troy 500 W. Big Beaver Troy, MI 48084

Re: Courtyard by Marriott 5550 New King Drive Troy, MI 48098

To whom it may concern;

The development is located at the northwest corner of the intersection at Crooks Road and Corporate Drive. The site is entered from New King Drive located to the West of the Property. Corporate offices dominate the area surrounding the site with commercial space located on Crooks Road. The proposed hotel is a 5-story Courtyard by Marriott that by appearance from the road sits 4 stories as the site drops at the entry level a full story. Included in the amenities is a recreational patio located adjacent to the building.

Courtyard by Marriott is a leader in meeting the needs of the modern business traveler. This hotel is no exception to the style and service its clients require. The building will provide amenities such as a fitness center, bistro, meeting rooms, and collaborative lobby spaces for its business guests in addition to a marketplace. The entire development comprises several retail locations that benefit the community as well as the guests of the hotel. Included are a Panera bread, Starbucks Coffee, an orthopedic and spine rehabilitation center as well as locations for small commercial spaces.

This Courtyard is of a contemporary aesthetic that blends itself into the context of the surrounding areas. Primarily constructed utilizing a low maintenance masonry/cement board/glazed façade, the building will then be finished with painted metal and composite wood detailing. The sleek design ties into the contextual fabric of the linear designed masonry glass office buildings that make up its surroundings. The light nature of the exterior colors blends the hotel into the context of the adjacent commercial buildings color palette.

1420 Washington Blvd. Suite 430, Detroit, Michigan 48226 (o)313.974.6456 (e)info@thethinkshop.us www.thethinkshop.us



Large Firm Resources. Personal Attention. "

Memorandum

To:	Jimmy Asmar, Elite Hospitality, LLC
From:	Michael J. Labadie, PE and Jill Bauer, PE, PTOE
Date:	July 19, 2019
RE:	Courtyard Troy, Traffic Impact Assessment and Shared Parking Analysis

ROWE Professional Services Company has completed our traffic impact assessment related to the proposed Courtyard Troy development to be located on the west side of Crooks Road, north of Corporate Drive/I-75 Ramps, in the City of Troy, Oakland County. The current site plan (included in the materials attached to this report) indicates a hotel with 133 rooms, 8,516 square feet of general office space, 2,176 square feet of medical office space, a 2,072-square-foot coffee shop with drive-through window, a 4,414-square-foot Panera bread shop with drive-through window, and a 4,500-square-foot sit-down restaurant with up to 106 seats. This traffic impact assessment has been completed in accordance with the requirements specified by the City of Troy and the Road Commission for Oakland County (RCOC).

Traffic Counts

The original turning movement traffic counts provided by your office for this study were collected during the weekday AM (7 to 9 a.m.) and PM (4 to 6 p.m.) peak periods on October 16, 2018 at the intersections of:

- Crooks Road and Corporate Drive/I-75 ramps;
- Corporate Drive and New King Drive; and
- Southbound Crooks Road and the northbound to southbound crossover north of Corporate Drive.

The existing turning movement traffic counts are shown in Figure 2 and are provided along with the intersection counts attached to this memorandum.

Background Traffic Scenario

Based on information provided by your office, the development is anticipated to be completed in 2020. A background growth rate of 1.5 percent per year was determined to be appropriate in the original study prepared for your development and was utilized again in forecasting normal yearly increases in traffic, which is unrelated to your proposed development.

The background traffic volumes are shown in Figure 3 attached to this memorandum.

Trip Generation

Using the information and methodologies specified in the latest version of *Trip Generation* (10^{th} Edition) published by the Institute of Transportation Engineers (ITE), ROWE forecast the weekday AM and PM peak hour trips associated with the proposed development. The results of the trip generation forecasts for the proposed site are provided in Table 1.

Land		AM	Peak H	our	PM Peak Hour			Week	
Land Use	Use Code	Size	In	Out	Total	In	Out	Total	Day
Business Hotel	312	133 rooms	22	30	52	24	19	43	537
General Office Building	710	8,516 sq. ft.	29	5	34	2	9	11	97
Medical-Dental Office Building	720	2,176 sq. ft.	5	2	7	2	6	8	76
Shopping Center	820	3,232 sq. ft.	2	1	3	21	22	43	583
High-Turnover (Sit-Down) Restaurant	932	4,500 sq. ft.	25	20	45	27	17	44	505
Coffee/Donut Shop with Drive-Through Window	937	2,072 sq. ft.	94	90	184	45	45	90	1,700
Bead/Donut/Bagel Shop with Drive-Through Window	940	4,414 sq. ft.	77	76	153	41	43	84	1,530 ¹
TOTAL TRIPS			254	224	478	162	161	323	5,028
Internal Capture, 6% AM, 22	% PM		15	15	30	35	35	70	
TOTAL EXTERNAL TRIPS			239	209	448	127	126	253	5,028
Shopping Center Pass-By Rate: 34% PM						3	5	8	
High-Turnover (Sit Down) Restaurant Pass-By Rate: 43%						7	6	13	
Fast-Food Restaurant with Drive-Through Window Pass-By Rate: 49% AM; 50% PM ²			80	79	159	38	36	74	
TOTAL NEW TRIPS			159	130	289	79	79	158	5,028

 Table 1

 ITE Trip Generation for Proposed Courtyard Troy Development

1. Daily trip information not provided for Land Use 940. Daily trips estimated to be ten times AM peak period (higher of both peak periods).

 ITE's Trip Generation Handbook, 3rd Edition does not contain information on pass-by rates for Land Use 937: Coffee/Donut Shop with Drive-Through Window or Land Use 940: Coffee/Donut/Bagel Shop with Drive-Through Window. Information for Land Use 934: Fast-Food Restaurant with Drive-Through Window, a similar use, was substituted.

In large, multi-use developments (where there is a mixture of retail, office, and restaurant uses), not all the trips generated are from sources outside the boundaries of the development, but rather some trips are "internally captured" within the site. The methodology presented in Trip Generation Handbook, 3rd Edition was followed to determine an appropriate internal capture rate for the proposed development. The results of this analysis suggest a 6 percent AM peak hour and a 22 percent PM peak hour internal capture rate for the combination of the shopping center, restaurants, medical office, and general office land uses. With the inclusion of the internal capture reductions, the proposed development will generate 448 total trips during the AM peak hour (239 inbound and 209 outbound) and 253 total trips during the PM peak hour (127 inbound and 126 outbound).

Additionally, not all the traffic generated by the proposed development will be new traffic added onto the adjacent roadway network. As with most retail and restaurant facilities, a significant amount of the site-generated traffic is considered "pass-by" traffic. Pass-by trips are trips already present on the adjacent

roadway network, which are interrupted to visit the site. Pass-by trips are accounted for by reducing the number of forecast new trips to be added to the roadway network; however, actual driveway volumes are not reduced. Pass-by trips are normally expressed as a percentage of trips generated by the new development. These pass-by rates are published in the Institute of Transportation Engineer's Trip Generation Handbook, 3rd Edition (September 2017).

Trip Generation Handbook, 3rd Edition suggests a 34 percent PM peak hour pass-by rate for a shopping centers, a 43 percent pass-by rate for high-turnover (sit-down) restaurants, and a 49 percent AM and a 50 percent PM pass-by rate for fast-food restaurants. With the application of the pass-by trip factors, the site-generated trips can be classified as "pass-by" and "new" trips. The proposed development is expected to generate 448 total trips during the AM peak hour and 253 total trips during the PM peak hour. However, only 289 trips during the AM Peak hour and 158 trips during the PM peak hour will be new traffic not currently using the adjacent street network, whose primary purpose is to visit the new development.

Trip Distribution

The existing traffic volumes were used to develop a trip distribution model for the AM and PM peak hours for traffic generated by the proposed development. The existing traffic patterns indicate the following probable distribution for the proposed development:

AM Peak Hour

47% from and 24% to the north 16% from and 46% to the south 32% from and 17% to the east 5% from and 13% to the west

PM Peak Hour

21% from and 46% to the north 41% from and 24% to the south 23% from and 26% to the east 15% from and 4% to the west

The proposed trip distribution for the site is shown in Figure 4 attached to this memorandum. The background traffic volumes were combined with the site generated traffic volumes to obtain the total future traffic volumes, which are shown in Figure 5 attached to this memorandum.

Level of Service Analysis

Level of service (LOS) analyses for existing, background (no build), and total future conditions for the AM and PM peak hours were performed for the intersections of:

- Crooks Road and Corporate Drive/I-75 ramps;
- Corporate Drive and New King Drive; and
- Southbound Crooks Road and the northbound to southbound crossover north of Corporate Drive.

The three proposed site driveway intersections were analyzed under total future conditions.

According to the most recent edition (6th Edition) of the Highway Capacity Manual, level of service is a qualitative measure describing operational conditions of a traffic stream or intersection. Level of service ranges from A to F, with LOS A being the best. LOS D is generally considered to be acceptable. Tables 2 and 3 present the criteria for defining the various levels of service for unsignalized and signalized intersections, respectively.

The operational analysis of the Crooks Road study intersections was performed utilizing 2000 HCM intersection methodology, since the HCM 6th Edition methodology does not support analysis of shared lanes at signalized intersections, or more than three through lanes on an approach to an unsignalized intersection. Although Crooks Road and Corporate Drive are both boulevards, at their intersection direct left-turns are allowed, and there is only a single set of signal heads for each approach (typically at boulevard intersections

there are "near" and "far" heads, with different clearance intervals to ensure vehicles do not become "trapped" in the middle of the intersection). Due to this, the intersection of Crooks Road and Corporate Drive/I-75 ramps was modeled as a single intersection, rather than the typical dual-intersection boulevard configuration specified in Michigan Department of Transportation standards.

Level of Service Criteria (Unsignalized Intersection)					
Level of Service	Average Stopped Delay/Vehicle (seconds)				
А	≤ 10				
В	$> 10 \text{ and } \le 15$				
С	> 15 and \leq 25				
D	> 25 and ≤ 35				
Е	$>$ 35 and \leq 50				
F	> 50				

Table 2				
Level of Service Criteria (Unsignalized Intersection)				

Note: LOS D is considered acceptable in urban/suburban areas.

Level of Service Criteria (Signalized Intersection)					
Level of Service Average Stopped Delay/Vehicle (seconds					
≤ 10					
$> 10 \text{ and } \le 20$					
> 20 and \leq 35					
> 35 and ≤ 55					
> 55 and ≤ 80					
F > 80					

Table 2

Note: LOS D is considered acceptable in urban/suburban areas.

The results of the level of service analyses for the intersections listed above are summarized in Tables 4 through 12.

Signalized Intersection of Crooks Road and Corporate Drive/I-75 Ramps

The City of Troy's engineering consultant, OHM, had asked that the operational analysis of this intersection not be completed via accepted practice with the timing permit provided by the Road Commission for Oakland County, but rather perform field observations of timings during the peak periods and "average" the observed timings. However, the ongoing construction on I-75 in the vicinity of the project and the disruption to normal traffic patterns precluded this from being performed, and the timing permit information was utilized in the operational analysis.

The results of the existing level of service analysis for the signalized intersection of Crooks Road and Corporate Drive/I-75 ramps indicate that, under existing conditions, all approaches to the intersection operate at an LOS D or better during the AM peak hour, except for the westbound approach which operates at an LOS E. During the PM peak hour, all approaches to the intersection operate at an LOS D or better, except for the northbound and eastbound approaches which operate at an LOS E and LOS F, respectively. The overall intersection operates at an LOS D during the AM peak hour and at an LOS E during the PM peak hour.

With the addition of background traffic and accounting for possible SCATS optimization of the intersection signal timing, all approaches to the intersection would operate at an LOS D or better during both peak periods, except for the eastbound approach during the PM peak hour, which would continue to operate at an LOS F. The overall intersection would continue operate at an LOS D during the AM peak hour and at an LOS E during the PM peak hour.

With inclusion of the recommended background improvement of providing a dedicated eastbound left-turn lane with 200 feet of storage and accounting for possible SCATS optimization of the intersection signal timing, all approaches to the intersection would operate at an LOS D or better during both the AM and PM peak hours. The overall intersection would operate at an LOS D during both peak periods.

The intersection would continue to operate in a manner like the background with improvements scenario with the addition of site generated traffic.

The operational results for the intersection of Crooks Road and Corporate Drive/I-75 ramps are presented in Tables 4 and 5.

Level of Service Analysis for Crooks Road and Corporate Drive/1-75 Ramps						
Approach	2018 Existing ¹	2020 Background ²	2020 Background Improved ³	2020 Total Future ⁴		
Northbound Crooks Road	D (39.9)	D (51.1)	D (51.1)	D (53.7)		
Southbound Crooks Road	C (31.8)	D (36.0)	C (34.7)	D (37.5)		
Eastbound Corporate Drive	D (53.3)	D (53.1)	D (52.7)	D (53.0)		
Westbound I-75 Off-Ramp	E (70.9)	D (47.5)	D (47.4)	D (50.4)		
Overall Intersection	D (46.7)	D (42.8)	D (42.1)	D (44.9)		

 Table 4

 AM Peak Hour

 Level of Service Analysis for Crooks Road and Corporate Drive/I-75 Ramps

(XX.X) Average seconds of delay per vehicle.

1. Operational results represent existing splits provided in timing permit.

2. Operational results represent SCATS optimization.

3. Includes background (no build) construction of an eastbound left-turn lane with 200 feet of storage.

4. Total future condition assumes background improvements.

Approach	2018 Existing ¹	2020 Background ²	2020 Background Improved ³	2020 Total Future ⁴
Northbound Crooks Road	E (61.9)	D (51.0)	D (51.0)	D (54.4)
Southbound Crooks Road	C (28.4)	D (52.1)	D (42.1)	D (49.0)
Eastbound Corporate Drive	F (177.4)	F (82.9)	D (52.9)	D (54.4)
Westbound I-75 Off-Ramp	D (51.5)	D (52.5)	D (52.0)	D (54.2)
Overall Intersection	E (70.3)	E (56.5)	D (49.6)	D (53.2)

Table 5PM Peak HourLevel of Service Analysis for Crooks Road and Corporate Drive/I-75 Ramps

(XX.X) Average seconds of delay per vehicle.

1. Operational results represent existing splits provided in timing permit.

- 2. Operational results represent SCATS optimization.
- 3. Includes background (no build) construction of an eastbound left-turn lane with 200 feet of storage.
- 4. Total future condition assumes background improvements.

Unsignalized Intersection of Corporate Drive and New King Drive

The results of the existing level of service analysis for the unsignalized intersection of Corporate Drive and New King Drive indicate that, under existing conditions, all approaches to the intersection operate at an LOS C or better during the AM peak hour, and at an LOS B or better during the PM peak hour.

The intersection would continue to operate in a manner like the existing conditions with the addition of background traffic.

With the addition of site generated traffic to the intersection, all approaches to the intersection would operate at an LOS A during the AM peak hour, except for the southbound approach, which would operate at an LOS F. During the PM peak hour, all approaches to the intersection would operate at an LOS C or better.

Several mitigation measures were considered for the intersection of Corporate Drive and New King Drive to address the forecast poor operation of the intersection during the AM peak hour. The southbound New King Drive approach already has separate left-turn and right-turn lanes, additional lanes would not substantially improve the forecast operation of the intersection, and dual left-turn lanes would likely increase the sideswipe crash experience at the intersection given the intersection is located on a curve. Forecast volumes at the intersection would not be enough to warrant signalization, which would also be difficult to properly coordinate with the existing traffic signal at Crooks Road. Forecast volumes would also not be enough to warrant conversion to all-way stop control at the intersection, although this would result in acceptable operation at the intersection during both peak periods.

The operational results for the intersection of Corporate Drive and New King Drive are presented in Tables 6 and 7.

AM Peak Hour						
Level of Service Analysis for Corporate Drive and New King Drive						
Ammusash	2018	2020	2020			
Approach	Existing	Background	Total Future			
Eastbound Corporate Drive	A (5.6)	A (5.7)	A (6.0)			
Westbound Corporate Drive	A (-)	A (-)	A (-)			
Southbound New King Drive	C (17.3)	C (18.1)	F (55.7)			

Table 6

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

PM Peak Hour						
Level of Service Analysis for Corporate Drive and New King Drive						
Annuagh	2018	2020	2020			
Арргоасп	Existing	Background	Total Future			
Eastbound Corporate Drive	A (0.5)	A (0.5)	A (0.7)			
Westbound Corporate Drive	$\Delta(-)$	A(-)	$\Delta(-)$			

B (13.0)

B (13.3)

C (21.0)

Table 7

(XX.X) Average seconds of delay per vehicle.

Southbound New King Drive

(-) Approach is unopposed and experiences no delay.

Unsignalized Intersection of Southbound Crooks Road and the Northbound to Southbound Crossover North of Corporate Drive

The results of the existing level of service analysis for the unsignalized intersection of southbound Crooks Road and the northbound to southbound crossover north of Corporate Drive indicate that, under existing conditions, all approaches to the intersection operate at an LOS C or better during the AM peak hour, and at an LOS B or better during the PM peak hour.

The intersection would continue to operate in a manner like the existing conditions with the addition of both background and site generated traffic. Therefore, the proposed development would have a minimal impact on the operation of this intersection.

The operational results for the intersection of southbound Crooks Road and the northbound to southbound crossover north of Corporate Drive are presented in Tables 8 and 9.

Table 8 **AM Peak Hour** Level of Service Analysis for Southbound Crooks Road and the Northbound to Southbound Crossover North of Corporate Drive

Approach	2018 Existing	2020 Background	2020 Total Future	
Southbound Crooks Road	A (-)	A (-)	A (-)	
Westbound Crossover	C (16.6)	C (17.1)	C (18.9)	

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

Table 9					
PM Peak Hour					
Level of Service Analysis for Southbound Crooks Road and the					
Northbound to Southbound Crossover North of Corporate Drive					
	0010	2020	2020		

Approach	2018 Existing	2020 Background	2020 Total Future
Southbound Crooks Road	A (-)	A (-)	A (-)
Westbound Crossover	B (10.6)	B (10.7)	B (11.1)

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

Unsignalized Intersection of Southbound Crooks Road and the East Site Driveway

The east site driveway will be located on the west side of Crooks Road approximately 520 feet north of Corporate Drive. The results of the level of service analysis for the unsignalized intersection of Crooks Road and the east site driveway indicate that, under total future conditions, the eastbound driveway approach would operate at an LOS C during the AM peak hour and at an LOS B during the PM peak hour. The southbound Crooks Road approach would operate at an LOS A during both peak periods.

The Road Commission for Oakland County standards for right-turn treatments at driveways was not evaluated for the proposed east site driveway, since there is already an existing continuous right-turn lane at the location of the proposed east site driveway. Left-turn warrants were not reviewed since Crooks Road is a boulevard at this location and direct left-turns into the driveway would not be possible.

The operational results for the intersection of Crooks Road and the east site driveway are presented in Table 10.

Level of Service Analysis for Crooks Road and the East Site Driveway													
Approach	2020 AM Peak Hour	2020 PM Peak Hour											
Southbound Crooks Road	A (-)	A (-)											
Eastbound (East) Site Driveway	C (20.6)	B (10.7)											

Table 10 AM and PM Peak Hours evel of Service Analysis for Crooks Road and the East Site Drivewa

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

Unsignalized Intersection of Westbound Corporate Drive and the South Site Driveway

The south site driveway will be located on the north side of Corporate Drive approximately 360 feet west of Crooks Road. The results of the level of service analysis for the unsignalized intersection of Corporate Drive and the south site driveway indicate that, under total future conditions, the southbound driveway approach would operate at an LOS B during the AM peak hour and at an LOS A during the PM peak hour. The westbound Corporate Drive approach would operate at an LOS A during both peak periods.

The Road Commission for Oakland County standards for right-turn treatments at driveways was evaluated for the proposed south site driveway. Left-turn warrants were not reviewed since Corporate Drive is a boulevard at this location and direct left-turns into the driveway would not be possible. The Average Daily Traffic (ADT) on westbound Corporate Drive at the location of the proposed south site driveway would be approximately 3,900 vehicles per day. The peak hour right-turns would be 78. Based on this, only a right-

turn taper is required at the south site driveway. The RCOC turn lane warrant analysis sheets are included in the appendix materials attached to this letter.

The operational results for the intersection of Corporate Drive and the south site driveway are presented in Table 11.

AM and PM Peak Hours													
Level of Service Analysis for Corporate Drive and the South Site Driveway													
Annacah	2020	2020											
Approach AM Peak Hour PM Peak Hour													
Westbound Corporate Drive	A (-)	A (-)											
Southbound (South) Site Driveway B (13.3) A (9.9)													

Table 11	Table 11
AM and PM Peak Hours	AM and PM Peak Hours
aval of Somuce Analysis for Cornerate Drive and the South Site Driveway	A nalyzig for Cornerate Drive and the South Site Driveway

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

Unsignalized Intersection of New King Drive and the West Site Driveway

The west site driveway will be located on the east side of New King Drive approximately 650 feet north of Corporate Drive. The results of the level of service analysis for the unsignalized intersection of New King Drive and the west site driveway indicate that, under total future conditions, the westbound driveway approach would operate at an LOS B during both the AM and PM peak hours. The New King Drive approaches would operate at an LOS A during both peak periods.

The Road Commission for Oakland County standards for right-turn treatments at driveways was evaluated for the proposed west site driveway. Left-turn warrants were not reviewed since there is an existing center two-way left-turn lane on New King Drive at the proposed west side driveway location. The ADT on New King Drive at the location of the proposed west site driveway would also be estimated to be approximately 3,900 vehicles per day. The peak hour right-turns would be 36. Based on this, only a right-turn taper is required at the south site driveway. The RCOC turn lane warrant analysis sheets are included in the appendix materials attached to this letter.

The operational results for the intersection of New King Drive and the west site driveway are presented in Table 12.

Level of Service Analysis for New King Drive and the west Site Driveway													
Approach	2020 AM Peak Hour	2020 PM Peak Hour											
Northbound New King Drive	A (-)	A (-)											
Southbound New King Drive	A (0.0)	A (0.0)											
Westbound (West) Site Driveway	B (12.0)	B (10.7)											

Table 12 **AM and PM Peak Hours**

(XX.X) Average seconds of delay per vehicle.

(-) Approach is unopposed and experiences no delay.

Shared Parking Analysis

A shared parking analysis was completed for the site in accordance with both Urban Land Institute (ULI) and City of Troy standards. The City of Troy allows for a shared parking analysis based on parking supply rates specified in the City Ordinance and ULI rates for hourly, daily, and monthly variations in parking

demand for the various land uses specified. Additionally, ULI rates for employees versus visitors were also utilized for the various land uses within the proposed site. Finally, as specified by the City of Troy engineering consultant OHM, a maximum occupancy of 95 percent of the 266 spaces on the site, or 252 spaces, would be allowed for the shared parking analysis.

This analysis was performed in order to determine the maximum possible size of a potential family restaurant located in the southeast quadrant of the site, based on the proposed parking supply. All the other uses and sizes on the site were assumed to be fixed (mix of retail, general office, medical office, hotel, and fast-food restaurants), and only the number of seats in a family-style restaurant would be varied in order to determine the maximum possible size for this restaurant.

The ULI shared parking methodologies were implemented to determine an accurate parking demand for the proposed site. It is common for sites with a mixture of uses to have varying peak hourly and monthly parking demands, as well as being shared destinations (i.e. a customer who shops at the retail stores may also visit a restaurant). Therefore, a shared parking analysis is necessary to accurately determine the amount in which the number of required parking spaces could be reduced by, as compared to the sum of the individual parking requirements for each land use on the site.

ROWE analyzed the differences in activity patterns, which include monthly and time of day variances, for each land use. This is because the combination of land uses within the proposed Troy Plaza site do not have peak utilization during the same time periods. Time-of-Day factors for weekdays and weekends and monthly adjustment factors for the various land uses can be found in the attached ULI Shared Parking spreadsheets.

The final step of the analysis was to determine the critical parking periods for the site. All hours of operation from 6 a.m. to midnight were analyzed for each month of the year (including an additional late December analysis). The result showed that the month of July has the highest peak demands of the year. Furthermore, the overall peak time of the day for the entire site was determined to be 12 p.m. (noon) for both weekdays and weekends. The overall peak period of the proposed site would be noon in July (both weekday and weekend were the same), resulting in a maximum demand of 252 required parking spaces. Detailed tables can be found in the attached ULI Shared Parking spreadsheets.

Based on the above analysis, the maximum possible size for a potential family-style restaurant on the site would be 106 seats.

Conclusions and Recommendations

The proposed Courtyard Troy development in the City of Troy consists of a hotel with 133 rooms, 8,516 sq. ft. of general office space, 2,176 sq. ft. of medical office space, a 2,072 sq. ft. coffee shop with drive-through window, a 4,414 sq. ft. Panera bread shop with drive-through window, and a 4,500 sq. ft. sit-down restaurant with up to 106 seats. The proposed development will have access to southbound Crooks Road, westbound Corporate Drive, and New King Drive via a single driveway on each roadway.

The proposed mixed-use development is forecast to generate 289 new trips during the AM peak hour (159 inbound and 130 outbound from the site) and 158 new trips during the PM peak hour (79 inbound and 79 outbound from the site).

An operational analysis was performed for the existing, background, and total future conditions for the intersections of:

• Crooks Road and Corporate Drive/I-75 ramps;

- Corporate Drive and New King Drive; and
- Southbound Crooks Road and the northbound to southbound crossover north of Corporate Drive.

The proposed site driveways to southbound Crooks Road, westbound Corporate Drive, and New King Drive were analyzed under total future conditions.

A review of Road Commission for Oakland County standards indicated that only a right-turn taper would be warranted on westbound Corporate Drive at the south site driveway.

To address impacts from existing and background traffic volumes, it is recommended that an eastbound left-turn lane with 200 feet of storage be provided at the intersection of Corporate Drive with Crooks Road.

Poor operation of the intersection of Corporate Drive and New King Drive is forecast during the AM peak period under total future conditions. Several mitigation measures were considered; however, they would either not be warranted or substantially improve the operation of the intersection.

Based on a shared parking analysis completed for the site in accordance with City of Troy and ULI standards, a family-style restaurant with a maximum of 106 seats could be developed in the southeast portion of the site.

Attachments

REPORT FIGURES











TRAFFIC COUNTS

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Crooks Road &	A.M. Peak		PHF		0.76			0.92			0.95			0.95	
Corp\I-75 Ramp	10/16/18	2018	Existing	27	112	19	644	478	554	0	605	210	539	1662	342
		2020	Background	28	115	20	663	492	571	0	623	216	555	1712	352
		Total	Background	28	115	20	663	492	571	0	623	216	555	1712	352
A.IVI.		Site	Generated	51	8	20		51			25		14	39	55
		To	otal Future	79	123	40	663	543	571	0	648	216	569	1751	407

Growth Rate: 1.5%

Buildout Year: 2020 Count Year: 2018

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Corp. Drive &	A.M. Peak		PHF		0.88			0.95						0.75	
New King Drive	10/16/18	2018	Existing	140	138			647	173				20		28
		2020	2020 Background		142	0	0	666	178	0	0	0	21	0	29
		Total Background		144	142	0	0	666	178	0	0	0	21	0	29
		Site Generated		8				6	28				79		11
		Total Future		152	142	0	0	672	206	0	0	0	100	0	40

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
SB Crooks Rd &	A.M. Peak		PHF					0.77						0.95	
NB to SB X-Ovr	10/16/18	2018	Existing				65							2478	
		2020	Background	0	0	0	67	0	0	0	0	0	0	2552	0
		Total	Background	0	0	0	67	0	0	0	0	0	0	2552	0
		Site	Generated				45							75	
		To	Total Future		0	0	112	0	0	0	0	0	0	2627	0

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Crooks Road &	P.M. Peak		PHF		0.93			0.95			0.95			0.95	
Corp\I-75 Ramp	10/16/18	2018	Existing	171	496	73	410	119	568	0	1482	499	255	716	74
		2020	Background	176	511	75	422	123	585	0	1527	514	263	738	76
		Total	Background	176	511	75	422	123	585	0	1527	514	263	738	76
		Site	Generated	67	7	7		18			32		13	13	33
		To	otal Future	243	518	82	422	141	585	0	1559	514	276	751	109

Growth Rate: 1.5%

Buildout Year: 2020 Count Year: 2018

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Corp. Drive &	P.M. Peak		PHF		0.89			0.80						0.68	
New King Drive	10/16/18	2018	Existing	38	667			176	17				73		61
		2020	Background	39	687	0	0	181	18	0	0	0	75	0	63
		Total	Background	39	687	0	0	181	18	0	0	0	75	0	63
		Site Generated		12				1	11				81		2
		To	otal Future	51	687	0	0	182	29	0	0	0	156	0	65

Intersection	Time period	Year	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
SB Crooks Rd &	P.M. Peak		PHF					0.79						0.95	
NB to SB X-Ovr	10/16/18	2018	Existing				38							1007	
		2020	Background	0	0	0	39	0	0	0	0	0	0	1038	0
		Total	Background	0	0	0	39	0	0	0	0	0	0	1038	0
		Site	Generated				64							17	
		To	tal Future	0	0	0	103	0	0	0	0	0	0	1055	0

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815

Crooks Road & Corporate Drive/I-75 Ramp City Of Troy Weather Condition; Clear Board # 2

Crooks Rd I-75 Off Ramp Crooks Road Corporate Drive Southbound Westbound Northbound Eastbound	Peds 0	Int. Total
Southbound Westbound Northbound Eastbound	Peds 0	Int. Total
	Peds 0	Int. Total
Start Time Right Thru Left Peds Right Thru Left Peds Right Thru Left Peds Right Thru Left	0	
07:00 AM 113 365 123 0 133 102 135 0 26 94 0 0 4 21 4	-	1120
07:15 AM 103 353 125 0 124 114 165 0 33 96 0 0 4 32 6	0	1155
07:30 AM 74 424 142 0 142 130 180 0 60 153 0 0 5 38 9	0	1357
07:45 AM 66 439 143 0 150 118 190 0 43 144 0 0 5 29 6	0	1333
Total 356 1581 533 0 549 464 670 0 162 487 0 0 18 120 25	0	4965
09:00 AM 06 429 122 0 129 115 127 0 49 162 0 0 5 24 6	0	1202
08.00 AW 106 361 122 0 136 113 137 0 46 105 0 0 3 24 0	0	1292
08.13 AM 82 380 132 0 124 113 137 0 35 143 0 0 4 21 0	0	1210
08.35 AM 76 356 99 0 130 101 146 0 59 160 0 0 5 32 7	0	1171
Total 360 1544 492 0 520 434 561 0 214 607 0 0 18 106 24	0	4880
	Ũ	
*** BREAK ***		
04:00 PM 19 159 68 0 81 25 89 0 85 297 0 0 10 70 25	0	928
04:15 PM 21 172 78 0 129 35 76 0 97 339 0 0 15 105 30	0	1097
04:30 PM 17 188 65 0 129 27 113 0 91 370 0 0 16 110 43	0	1169
04:45 PM 22 180 70 0 140 25 107 0 138 366 0 0 18 123 45	0	1234
Total 79 699 281 0 479 112 385 0 411 1372 0 0 59 408 143	0	4428
05:00 PM 18 172 58 0 145 29 111 0 111 389 0 0 21 134 44	0	1232
05:15 PM 17 176 62 0 154 38 79 0 159 357 0 0 18 129 39	0	1228
05:30 PM 15 155 62 0 133 34 105 0 115 373 0 0 15 124 33	0	1164
05:45 PM 14 156 48 0 156 27 114 0 82 361 0 0 11 126 36	0	1131
Total 64 659 230 0 588 128 409 0 467 1480 0 0 65 513 152	0	4755
Grand Total 859 4483 1536 0 2136 1138 2025 0 1254 3946 0 0 160 1147 344	0	19028
Apprch % 12.5 65.2 22.3 0 40.3 21.5 38.2 0 24.1 75.9 0 0 9.7 69.5 20.8	Õ	
Total % 4.5 23.6 8.1 0 11.2 6 10.6 0 6.6 20.7 0 0 0.8 6 1.8	0	

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Crooks Road & Corporate Drive/I-75 Ramp City Of Troy Weather Condition; Clear Board # 2

	Crooks Rd I-75 Off Ramp										Cr	ooks F	Road		Corporate Drive]	
		Sc	outhbo	und			Westbound					N	orthbo	und		Eastbound					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	nalysis	From (07:00 A	AM to 1	1:45 AN	1 - Pea	k 1 of 1	1													
Peak Hour fo	r Entire	e Inters	ection	Begins	s at 07:3	0 AM															
07:30 AM	74	424	142	0	640	142	130	180	0	452	60	153	0	0	213	5	38	9	0	52	1357
07:45 AM	66	439	143	0	648	150	118	190	0	458	43	144	0	0	187	5	29	6	0	40	1333
08:00 AM	96	438	122	0	656	138	115	137	0	390	48	163	0	0	211	5	24	6	0	35	1292
08:15 AM	106	361	132	0	599	124	115	137	0	376	59	145	0	0	204	4	21	6	0	31	1210
Total Volume	342	1662	539	0	2543	554	478	644	0	1676	210	605	0	0	815	19	112	27	0	158	5192
% App. Total	13.4	65.4	21.2	0		33.1	28.5	38.4	0		25.8	74.2	0	0		12	70.9	17.1	0		
PHF	.807	.946	.942	.000	.969	.923	.919	.847	.000	.915	.875	.928	.000	.000	.957	.950	.737	.750	.000	.760	.957



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Crooks Road & Corporate Drive/I-75 Ramp City Of Troy Weather Condition; Clear Board # 2

		С			1-75	5 Off R	amp		Crooks Road]								
	Southbound						Westbound					Northbound						Eastbound					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total		
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																							
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:3	0 PM																	
04:30 PM	17	188	65	0	270	129	27	113	0	269	91	370	0	0	461	16	110	43	0	169	1169		
04:45 PM	22	180	70	0	272	140	25	107	0	272	138	366	0	0	504	18	123	45	0	186	1234		
05:00 PM	18	172	58	0	248	145	29	111	0	285	111	389	0	0	500	21	134	44	0	199	1232		
05:15 PM	17	176	62	0	255	154	38	79	0	271	159	357	0	0	516	18	129	39	0	186	1228		
Total Volume	74	716	255	0	1045	568	119	410	0	1097	499	1482	0	0	1981	73	496	171	0	740	4863		
% App. Total																							
PHF	.841	.952	.911	.000	.960	.922	.783	.907	.000	.962	.785	.952	.000	.000	.960	.869	.925	.950	.000	.930	.985		



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Crooks Road & Corporate Drive/I-75 Ramp City Of Troy Weather Condition; Clear Board # 2



2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815

Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1

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00000555
9/18/2018
1

	Groups Printed- Unshifted - Bank 1																
	1	New King	g Drive			I-75 R	amp						(
		Southb		Westb	ound			Northb	ound								
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	3	0	4	0	23	98	0	0	0	0	0	0	0	29	27	0	184
07:15 AM	3	0	2	0	29	119	0	0	0	0	0	0	0	33	29	0	215
07:30 AM	6	0	4	0	37	139	0	0	0	0	0	0	0	41	38	0	265
07:45 AM	5	0	5	0	45	169	0	0	0	0	0	0	0	35	42	0	301
Total	17	0	15	0	134	525	0	0	0	0	0	0	0	138	136	0	965
08:00 AM	10	0	6	0	41	174	0	0	0	0	0	0	0	27	34	0	292
08:15 AM	7	0	4	0	64	158	0	0	0	0	0	0	1	39	38	0	311
08:30 AM	6	0	5	0	23	184	0	0	0	0	0	0	0	31	26	0	275
08:45 AM	4	0	6	0	26	160	0	0	0	0	0	0	0	34	31	0	261
Total	27	0	21	0	154	676	0	0	0	0	0	0	1	131	129	0	1139
*** BREAK ***																	
04:00 PM	10	0	0	0	7	19	0	0	0	0	0	0	0	98	6	0	140
04:15 PM	19	0	11	0	3	23	0	0	0	0	0	0	0	106	11	0	173
04:30 PM	17	0	14	0	5	17	0	0	0	0	0	0	0	131	8	0	192
04:45 PM	15	0	12	0	4	18	0	0	0	0	0	0	0	195	7	0	251
Total	61	0	37	0	19	77	0	0	0	0	0	0	0	530	32	0	756
05:00 PM	12	0	22	0	3	35	0	0	0	0	0	0	0	157	13	0	242
05:15 PM	8	0	16	0	5	37	0	0	0	0	0	0	0	177	12	0	255
05:30 PM	26	0	23	0	5	41	0	0	0	0	0	0	0	154	6	0	255
05:45 PM	17	0	21	0	9	39	0	0	0	0	0	0	0	138	6	0	230
Total	63	0	82	0	22	152	0	0	0	0	0	0	0	626	37	0	982
Grand Total	168	0	155	0	329	1430	0	0	0	0	0	0	1	1425	334	0	3842
Apprch %	52	0	48	0	18.7	81.3	0	0	0	0	0	0	0.1	81	19	0	
Total %	4.4	0	4	0	8.6	37.2	0	0	0	0	0	0	0	37.1	8.7	0	
Unshifted	168	0	155	0	329	1430	0	0	0	0	0	0	1	1425	334	0	3842
% Unshifted	100	0	100	0	100	100	0	0	0	0	0	0	100	100	100	0	100
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1



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Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1

		New	/ King	Drive		I-75 Ramp											Corporate Drive					
		Sc	outhbo	und		Westbound						No	und									
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total	
Peak Hour Analysis From 07:00 AM to 11:45 AM - Peak 1 of 1																						
Peak Hour for Entire Intersection Begins at 07:45 AM																						
07:45 AM	5	0	5	0	10	45	169	0	0	214	0	0	0	0	0	0	35	42	0	77	301	
08:00 AM	10	0	6	0	16	41	174	0	0	215	0	0	0	0	0	0	27	34	0	61	292	
08:15 AM	7	0	4	0	11	64	158	0	0	222	0	0	0	0	0	1	39	38	0	78	311	
08:30 AM	6	0	5	0	11	23	184	0	0	207	0	0	0	0	0	0	31	26	0	57	275	
Total Volume	28	0	20	0	48	173	685	0	0	858	0	0	0	0	0	1	132	140	0	273	1179	
% App. Total	58.3	0	41.7	0		20.2	79.8	0	0		0	0	0	0		0.4	48.4	51.3	0			
PHF	.700	.000	.833	.000	.750	.676	.931	.000	.000	.966	.000	.000	.000	.000	.000	.250	.846	.833	.000	.875	.948	
Unshifted	28	0	20	0	48	173	685	0	0	858	0	0	0	0	0	1	132	140	0	273	1179	
% Unshifted	100	0	100	0	100	100	100	0	0	100	0	0	0	0	0	100	100	100	0	100	100	
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	



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Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1

		New	v King	Drive		I-75 Ramp											Corporate Drive					
		Sc	outhbo	und		Westbound						No	und									
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total	
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																						
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:4	5 PM																
04:45 PM	15	0	12	0	27	4	18	0	0	22	0	0	0	0	0	0	195	7	0	202	251	
05:00 PM	12	0	22	0	34	3	35	0	0	38	0	0	0	0	0	0	157	13	0	170	242	
05:15 PM	8	0	16	0	24	5	37	0	0	42	0	0	0	0	0	0	177	12	0	189	255	
05:30 PM	26	0	23	0	49	5	41	0	0	46	0	0	0	0	0	0	154	6	0	160	255	
Total Volume	61	0	73	0	134	17	131	0	0	148	0	0	0	0	0	0	683	38	0	721	1003	
% App. Total	45.5	0	54.5	0		11.5	88.5	0	0		0	0	0	0		0	94.7	5.3	0		1	
PHF	.587	.000	.793	.000	.684	.850	.799	.000	.000	.804	.000	.000	.000	.000	.000	.000	.876	.731	.000	.892	.983	
Unshifted	61	0	73	0	134	17	131	0	0	148	0	0	0	0	0	0	683	38	0	721	1003	
% Unshifted	100	0	100	0	100	100	100	0	0	100	0	0	0	0	0	0	100	100	0	100	100	
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	



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Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1



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Corporate Drive & New King Drive City of Troy Weather Condition; Clear Board # 1


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							Groups	Printed	 Unshift 	ed							
		SB Croo	oks Rd		N	IB to SB	X-Over										
		Southb	ound			Westb	ound			Northb	ound			Eastbo	bund		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	0	588	0	0	0	13	0	0	0	0	0	0	0	0	0	0	601
07:15 AM	0	569	0	0	0	12	0	0	0	0	0	0	0	0	0	0	581
07:30 AM	0	623	0	0	0	17	0	0	0	0	0	0	0	0	0	0	640
07:45 AM	0	621	0	0	0	21	0	0	0	0	0	0	0	0	0	0	642
Total	0	2401	0	0	0	63	0	0	0	0	0	0	0	0	0	0	2464
08:00 AM	0	638	0	0	0	16	0	0	0	0	0	0	0	0	0	0	654
08:15 AM	0	587	0	0	0	11	0	0	0	0	0	0	0	0	0	0	598
08:30 AM	0	588	0	0	0	17	0	0	0	0	0	0	0	0	0	0	605
08:45 AM	76	521	0	0	0	9	0	0	0	0	0	0	5	0	0	0	611
Total	76	2334	0	0	0	53	0	0	0	0	0	0	5	0	0	0	2468
*** BREAK ***																	
04:00 PM	0	234	0	0	0	11	0	0	0	0	0	0	0	0	0	0	245
04:15 PM	0	262	0	0	0	9	0	0	0	0	0	0	0	0	0	0	271
04:30 PM	0	255	0	0	0	/	0	0	0	0	0	0	0	0	0	0	262
04:45 PM	0	238	0	0	0	12	0	0	0	0	0	0	0	0	0	0	250
Iotal	0	989	0	0	0	39	0	0	0	0	0	0	0	0	0	0	1028
05.00 PM	0	248	0	0	0	10	0	0	0	0	0	0	0	0	0	0	258
05:15 PM	0	240	0	0	0	8	0	0	0	0	0	0	0	0	0	0	253
05:30 PM	0	245	0	0	0	7	0	0	0	0	0	0	0	0	0	0	233
05:45 PM	1/	210	0	0	0	8	0	0	0	0	0	0	0	0	0	0	232
Total	14	028	0	0	0	33		0	0	0	0	0	0	0		0	975
Total	17	520	0	0	0	00	0	0	0	0	0	0	0	U	0	0	575
Grand Total	90	6652	0	0	0	188	0	0	0	0	0	0	5	0	0	0	6935
Apprch %	1.3	98.7	Ő	Õ	Ő	100	Ő	õ	Ő	Ő	Ő	õ	100	Ō	Ő	Õ	
Total %	1.3	95.9	0	0	0	2.7	Ó	0	0	0	Ó	0	0.1	0	0	0	

Stellar Development, LLC.

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815



Stellar Development, LLC.

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815



Stellar Development, LLC.

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815

		SB	Crook	s Rd			NB t	o SB X	(-Over												1
		Sc	outhbo	und			N	/estbo	und			N	orthbo	und			E	astbou	ind		1
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	nalysis	From (07:00 Å	AM to 1	1:45 AN	1 - Pea	k 1 of '	1													
Peak Hour fo	r Entire	e Inters	ection	Begins	at 07:3	0 AM															
07:30 AM	0	623	0	0	623	0	17	0	0	17	0	0	0	0	0	0	0	0	0	0	640
07:45 AM	0	621	0	0	621	0	21	0	0	21	0	0	0	0	0	0	0	0	0	0	642
08:00 AM	0	638	0	0	638	0	16	0	0	16	0	0	0	0	0	0	0	0	0	0	654
08:15 AM	0	587	0	0	587	0	11	0	0	11	0	0	0	0	0	0	0	0	0	0	598
Total Volume	0	2469	0	0	2469	0	65	0	0	65	0	0	0	0	0	0	0	0	0	0	2534
% App. Total	0	100	0	0		0	100	0	0		0	0	0	0		0	0	0	0		1
PHF	.000	.967	.000	.000	.967	.000	.774	.000	.000	.774	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.969



Stellar Development, LLC. 2600 Auburn Road

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815

		SB	Crook	s Rd			NB to	o SB X	-Over												
		So	outhbo	und			W	estbou	und			N	orthbo	und			E	astbou	Ind		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From 1	12:00 F	PM to 0	5:45 PN	1 - Pea	k 1 of 1														
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:1	5 PM															
04:15 PM	0	262	0	0	262	0	9	0	0	9	0	0	0	0	0	0	0	0	0	0	271
04:30 PM	0	255	0	0	255	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	262
04:45 PM	0	238	0	0	238	0	12	0	0	12	0	0	0	0	0	0	0	0	0	0	250
05:00 PM	0	248	0	0	248	0	10	0	0	10	0	0	0	0	0	0	0	0	0	0	258
Total Volume	0	1003	0	0	1003	0	38	0	0	38	0	0	0	0	0	0	0	0	0	0	1041
% App. Total	0	100	0	0		0	100	0	0		0	0	0	0		0	0	0	0		1
PHF	.000	.957	.000	.000	.957	.000	.792	.000	.000	.792	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.960



Stellar Development, LLC. 2600 Auburn Road

2600 Auburn Road Auburn Hills, Michigan 481326 (810) 444-7815



LEVEL OF SERVICE

OUTPUT REPORTS

HCM Signalized Intersection Capacity Analysis2018 Existing AM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		± ↑	1	ሻ	đ î ja	1		***	1	ካካ	***	1
Traffic Volume (vph)	27	112	19	644	478	554	0	605	210	539	1662	342
Future Volume (vph)	27	112	19	644	478	554	0	605	210	539	1662	342
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.97	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3689	1667	1695	3232	1517		5353	1667	3614	5353	1667
Flt Permitted		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3689	1667	1695	3232	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	36	147	25	700	520	602	0	637	221	567	1749	360
RTOR Reduction (vph)	0	0	23	0	16	135	0	0	53	0	0	180
Lane Group Flow (vph)	0	183	2	469	922	280	0	637	168	567	1749	180
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		11.0	11.0	32.9	32.9	58.0		23.2	56.1	25.1	55.1	55.1
Effective Green, g (s)		11.0	11.0	32.9	32.9	58.0		23.2	56.1	25.1	55.1	55.1
Actuated g/C Ratio		0.09	0.09	0.27	0.27	0.48		0.19	0.47	0.21	0.46	0.46
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		338	152	464	886	733		1034	779	755	2457	765
v/s Ratio Prot		c0.05		0.28	c0.29	0.08		0.12	0.06	0.16	c0.33	
v/s Ratio Perm			0.00			0.10			0.04			0.11
v/c Ratio		0.54	0.02	1.01	1.04	0.38		0.62	0.22	0.75	0.71	0.24
Uniform Delay, d1		52.1	49.6	43.5	43.5	19.6		44.3	18.9	44.5	26.1	19.7
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		1.8	0.0	44.5	41.3	0.3		2.7	0.1	6.8	1.8	0.7
Delay (s)		53.9	49.6	88.1	84.9	20.0		47.1	19.1	51.3	27.9	20.4
Level of Service		D	D	F	H	В		D	В	D	C	С
Approach Delay (s)		53.3			70.9			39.9			31.8	
Approach LOS		D			E			D			С	
Intersection Summary												
HCM 2000 Control Delay			46.7	Н	CM 2000) Level of	Service		D			
HCM 2000 Volume to Capacity	/ ratio		0.86									
Actuated Cycle Length (s)			120.0	S	um of los	st time (s)			27.8			
Intersection Capacity Utilizatio	n		78.6%	IC	CU Level	of Service)		D			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis2020 No Build AM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		∱ }	1	7	đ þ	1		***	1	ሻሻ	***	1
Traffic Volume (vph)	28	115	20	663	492	571	0	623	216	555	1712	352
Future Volume (vph)	28	115	20	663	492	571	0	623	216	555	1712	352
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.97	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3689	1667	1695	3232	1517		5353	1667	3614	5353	1667
Flt Permitted		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3689	1667	1695	3232	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	37	151	26	721	535	621	0	656	227	584	1802	371
RTOR Reduction (vph)	0	0	24	0	16	152	0	0	54	0	0	178
Lane Group Flow (vph)	0	188	2	483	950	276	0	656	173	584	1802	193
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		11.3	11.3	37.6	37.6	63.7		17.2	54.8	26.1	50.1	50.1
Effective Green, g (s)		11.3	11.3	37.6	37.6	63.7		17.2	54.8	26.1	50.1	50.1
Actuated g/C Ratio		0.09	0.09	0.31	0.31	0.53		0.14	0.46	0.22	0.42	0.42
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		347	156	531	1012	805		767	761	786	2234	695
v/s Ratio Prot		c0.05		0.28	c0.29	0.07		0.12	0.07	0.16	c0.34	
v/s Ratio Perm			0.00			0.11			0.03			0.12
v/c Ratio		0.54	0.02	0.91	0.94	0.34		0.86	0.23	0.74	0.81	0.28
Uniform Delay, d1		51.9	49.3	39.6	40.1	16.1		50.2	19.8	43.8	30.7	23.0
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		1.7	0.0	19.4	15.5	0.3		11.7	0.2	6.3	3.2	1.0
Delay (s)		53.6	49.3	59.0	55.6	16.4		61.9	19.9	50.1	33.9	24.0
Level of Service		D	D	E	E	В		E	В	D	С	С
Approach Delay (s)		53.1			47.5			51.1			36.0	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			42.8	Н	CM 2000) Level of S	Service		D			
HCM 2000 Volume to Capaci	ty ratio		0.89									
Actuated Cycle Length (s)			120.0	S	um of los	st time (s)			27.8			
Intersection Capacity Utilizati	on		80.3%	IC	CU Level	of Service			D			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

2020 No Build AM Imp.

1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	††	1	ľ	et îr	1		111	1	ኘኘ	^	1
Traffic Volume (vph)	28	115	20	663	492	571	0	623	216	555	1712	352
Future Volume (vph)	28	115	20	663	492	571	0	623	216	555	1712	352
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor	1.00	0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.97	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1863	3725	1667	1695	3232	1517		5353	1667	3614	5353	1667
Flt Permitted	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1863	3725	1667	1695	3232	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	37	151	26	721	535	621	0	656	227	584	1802	371
RTOR Reduction (vph)	0	0	24	0	16	150	0	0	54	0	0	176
Lane Group Flow (vph)	37	151	2	483	950	278	0	656	173	584	1802	195
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)	10.3	10.3	10.3	37.6	37.6	64.7		17.2	54.8	27.1	51.1	51.1
Effective Green, g (s)	10.3	10.3	10.3	37.6	37.6	64.7		17.2	54.8	27.1	51.1	51.1
Actuated g/C Ratio	0.09	0.09	0.09	0.31	0.31	0.54		0.14	0.46	0.23	0.43	0.43
Clearance Time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)	159	319	143	531	1012	817		767	761	816	2279	709
v/s Ratio Prot	0.02	c0.04		0.28	c0.29	0.08		0.12	0.07	0.16	c0.34	
v/s Ratio Perm			0.00			0.11			0.03			0.12
v/c Ratio	0.23	0.47	0.02	0.91	0.94	0.34		0.86	0.23	0.72	0.79	0.28
Uniform Delay, d1	51.2	52.3	50.2	39.6	40.1	15.6		50.2	19.8	42.9	29.8	22.4
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.8	1.1	0.0	19.4	15.5	0.3		11.7	0.2	5.3	2.9	1.0
Delay (s)	51.9	53.4	50.3	59.0	55.6	15.9		61.9	19.9	48.2	32.7	23.4
Level of Service	D	D	D	E	E	В		E	В	D	С	С
Approach Delay (s)		52.7			47.4			51.1			34.7	
Approach LOS		D			D			D			С	
Intersection Summary												
HCM 2000 Control Delay			42.1	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity	/ ratio		0.87									
Actuated Cycle Length (s)			120.0	S	um of los	t time (s)			27.8			
Intersection Capacity Utilization	n		80.3%	IC	CU Level	of Service			D			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis2020 Build AM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	^	1	ሻ	đ þ	1		^	1	ሻሻ	^	1
Traffic Volume (vph)	79	123	40	663	543	571	0	648	216	569	1751	407
Future Volume (vph)	79	123	40	663	543	571	0	648	216	569	1751	407
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor	1.00	0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.97	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1863	3725	1667	1695	3244	1517		5353	1667	3614	5353	1667
Flt Permitted	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1863	3725	1667	1695	3244	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	104	162	53	721	590	621	0	682	227	599	1843	428
RTOR Reduction (vph)	0	0	48	0	14	72	0	0	54	0	0	203
Lane Group Flow (vph)	104	162	5	497	980	369	0	682	173	599	1843	225
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)	11.6	11.6	11.6	37.9	37.9	63.4		17.2	55.1	25.5	49.5	49.5
Effective Green, g (s)	11.6	11.6	11.6	37.9	37.9	63.4		17.2	55.1	25.5	49.5	49.5
Actuated g/C Ratio	0.10	0.10	0.10	0.32	0.32	0.53		0.14	0.46	0.21	0.41	0.41
Clearance Time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)	180	360	161	535	1024	801		767	765	767	2208	687
v/s Ratio Prot	c0.06	0.04		0.29	c0.30	0.10		0.13	0.07	0.17	c0.34	
v/s Ratio Perm			0.00			0.15			0.03			0.14
v/c Ratio	0.58	0.45	0.03	0.93	0.96	0.46		0.89	0.23	0.78	0.83	0.33
Uniform Delay, d1	51.9	51.2	49.1	39.7	40.3	17.6		50.5	19.6	44.6	31.6	23.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.4	0.9	0.1	22.5	18.5	0.4		14.6	0.2	7.8	3.9	1.3
Delay (s)	56.3	52.1	49.2	62.3	58.7	18.1		65.0	19.7	52.4	35.5	25.2
Level of Service	E	D	D	E	E	В		E	В	D	D	С
Approach Delay (s)		53.0			50.4			53.7			37.5	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			44.9	Н	CM 2000) Level of	Service		D			
HCM 2000 Volume to Capaci	ty ratio		0.91									
Actuated Cycle Length (s)			120.0	S	um of los	st time (s)			27.8			
Intersection Capacity Utilization	on		82.7%	IC	CU Level	of Service)		E			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis2018 Existing PM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		t₽.	1	ň	đ þ	1		***	1	ሻሻ	***	1
Traffic Volume (vph)	171	496	73	410	119	568	0	1482	499	255	716	74
Future Volume (vph)	171	496	73	410	119	568	0	1482	499	255	716	74
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.92	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3678	1667	1695	3066	1517		5353	1667	3614	5353	1667
Flt Permitted		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3678	1667	1695	3066	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	184	533	78	432	125	598	0	1560	525	268	754	78
RTOR Reduction (vph)	0	0	66	0	161	59	0	0	47	0	0	40
Lane Group Flow (vph)	0	717	12	294	401	240	0	1560	478	268	754	38
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	. 4	4		. 8	8	. 1		2	. 8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		19.9	19.9	25.9	25.9	45.1		37.2	63.1	19.2	63.2	63.2
Effective Green, g (s)		19.9	19.9	25.9	25.9	45.1		37.2	63.1	19.2	63.2	63.2
Actuated g/C Ratio		0.15	0.15	0.20	0.20	0.35		0.29	0.49	0.15	0.49	0.49
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		563	255	337	610	526		1531	809	533	2602	810
v/s Ratio Prot		c0.19		c0.17	0.13	0.07		c0.29	0.12	c0.07	0.14	
v/s Ratio Perm			0.01			0.09			0.17			0.02
v/c Ratio		1.27	0.05	0.87	0.66	0.46		1.02	0.59	0.50	0.29	0.05
Uniform Delay, d1		55.1	47.0	50.4	48.0	32.9		46.4	24.1	51.0	20.0	17.6
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		136.5	0.1	21.1	2.6	0.6		27.9	1.2	3.4	0.3	0.1
Delay (s)		191.6	47.0	71.6	50.5	33.6		74.3	25.3	54.4	20.3	17.7
Level of Service		F	D	E	D	С		E	С	D	С	В
Approach Delay (s)		177.4			51.5			61.9			28.4	
Approach LOS		F			D			E			С	
Intersection Summary												
HCM 2000 Control Delay			70.3	Н	CM 2000) Level of	Service		E			
HCM 2000 Volume to Capacit	y ratio		0.93									
Actuated Cycle Length (s)			130.0	S	um of los	st time (s)			27.8			
Intersection Capacity Utilization	on		88.5%	IC	U Level	of Service)		E			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis2020 No Build PM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		A ₽	1	5	đ b	1		***	1	ካካ	***	1
Traffic Volume (vph)	176	511	75	422	123	585	0	1527	514	263	738	76
Future Volume (vph)	176	511	75	422	123	585	0	1527	514	263	738	76
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.92	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3678	1667	1695	3066	1517		5353	1667	3614	5353	1667
Flt Permitted		0.99	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3678	1667	1695	3066	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	189	549	81	444	129	616	0	1607	541	277	777	80
RTOR Reduction (vph)	0	0	65	0	160	90	0	0	44	0	0	45
Lane Group Flow (vph)	0	738	16	302	419	218	0	1607	497	277	777	35
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		25.9	25.9	26.8	26.8	36.1		40.2	67.0	9.3	56.3	56.3
Effective Green, g (s)		25.9	25.9	26.8	26.8	36.1		40.2	67.0	9.3	56.3	56.3
Actuated g/C Ratio		0.20	0.20	0.21	0.21	0.28		0.31	0.52	0.07	0.43	0.43
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		732	332	349	632	421		1655	859	258	2318	721
v/s Ratio Prot		c0.20		c0.18	0.14	0.04		c0.30	0.12	c0.08	0.15	
v/s Ratio Perm			0.01			0.11			0.18			0.02
v/c Ratio		1.01	0.05	0.87	0.66	0.52		0.97	0.58	1.07	0.34	0.05
Uniform Delay, d1		52.0	42.1	49.9	47.5	39.6		44.3	21.7	60.4	24.4	21.3
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		35.3	0.1	19.5	2.6	1.1		16.2	0.9	76.9	0.4	0.1
Delay (s)		87.3	42.1	69.3	50.1	40.7		60.6	22.7	137.3	24.8	21.5
Level of Service		F	D	E	D	D		E	С	F	С	С
Approach Delay (s)		82.9			52.5			51.0			52.1	
Approach LOS		F			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			56.5	H	CM 2000) Level of S	Service		E			
HCM 2000 Volume to Capaci	ty ratio		0.96									
Actuated Cycle Length (s)			130.0	Si	um of los	st time (s)			27.8			
Intersection Capacity Utilization	on		90.5%	IC	U Level	of Service			E			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

2020 No Build PM Imp.

1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	^	1	5	đ þ	1		***	1	ሻሻ	***	1
Traffic Volume (vph)	176	511	75	422	123	585	0	1527	514	263	738	76
Future Volume (vph)	176	511	75	422	123	585	0	1527	514	263	738	76
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor	1.00	0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.92	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1863	3725	1667	1695	3066	1517		5353	1667	3614	5353	1667
Flt Permitted	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1863	3725	1667	1695	3066	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	189	549	81	444	129	616	0	1607	541	277	777	80
RTOR Reduction (vph)	0	0	66	0	160	108	0	0	44	0	0	45
Lane Group Flow (vph)	189	549	15	302	419	200	0	1607	497	277	777	35
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)	24.6	24.6	24.6	26.8	26.8	37.4		40.2	67.0	10.6	57.6	57.6
Effective Green, g (s)	24.6	24.6	24.6	26.8	26.8	37.4		40.2	67.0	10.6	57.6	57.6
Actuated g/C Ratio	0.19	0.19	0.19	0.21	0.21	0.29		0.31	0.52	0.08	0.44	0.44
Clearance Time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)	352	704	315	349	632	436		1655	859	294	2371	738
v/s Ratio Prot	0.10	c0.15		c0.18	0.14	0.04		c0.30	0.12	c0.08	0.15	
v/s Ratio Perm			0.01			0.09			0.18			0.02
v/c Ratio	0.54	0.78	0.05	0.87	0.66	0.46		0.97	0.58	0.94	0.33	0.05
Uniform Delay, d1	47.6	50.1	43.1	49.9	47.5	38.0		44.3	21.7	59.4	23.6	20.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.6	5.5	0.1	19.5	2.6	0.8		16.2	0.9	39.6	0.4	0.1
Delay (s)	49.1	55.6	43.2	69.3	50.1	38.8		60.6	22.7	99.0	24.0	20.7
Level of Service	D	E	D	E	D	D		E	С	F	С	С
Approach Delay (s)		52.9			52.0			51.0			42.1	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			49.6	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity	/ ratio		0.89									
Actuated Cycle Length (s)			130.0	Si	um of los	t time (s)			27.8			
Intersection Capacity Utilization	n		85.6%	IC	U Level	of Service			E			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis2020 Build PM1002: Corporate Drive (Pushbuttons)/I-75 Ramps (Pushbuttons) & Crooks Road (Northbound)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲.	^	1	۲.	4 þ	1		^	1	ሻሻ	^	1
Traffic Volume (vph)	243	518	82	422	141	585	0	1559	514	276	751	109
Future Volume (vph)	243	518	82	422	141	585	0	1559	514	276	751	109
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor	1.00	0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.92	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1863	3725	1667	1695	3073	1517		5353	1667	3614	5353	1667
Flt Permitted	0.95	1.00	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1863	3725	1667	1695	3073	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	261	557	88	444	148	616	0	1641	541	291	791	115
RTOR Reduction (vph)	0	0	71	0	133	71	0	0	44	0	0	65
Lane Group Flow (vph)	261	557	17	311	456	237	0	1641	497	291	791	50
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)	25.0	25.0	25.0	27.0	27.0	37.0		40.2	67.2	10.0	57.0	57.0
Effective Green, g (s)	25.0	25.0	25.0	27.0	27.0	37.0		40.2	67.2	10.0	57.0	57.0
Actuated g/C Ratio	0.19	0.19	0.19	0.21	0.21	0.28		0.31	0.52	0.08	0.44	0.44
Clearance Time (s)	7.1	7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)	358	716	320	352	638	431		1655	861	278	2347	730
v/s Ratio Prot	0.14	c0.15		c0.18	0.15	0.04		c0.31	0.12	c0.08	0.15	
v/s Ratio Perm			0.01			0.11			0.18			0.03
v/c Ratio	0.73	0.78	0.05	0.88	0.71	0.55		0.99	0.58	1.05	0.34	0.07
Uniform Delay, d1	49.3	49.9	42.8	50.0	47.9	39.4		44.7	21.6	60.0	24.1	21.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.3	5.3	0.1	22.1	3.8	1.5		20.2	0.9	66.7	0.4	0.2
Delay (s)	56.6	55.2	42.9	72.0	51.7	41.0		64.9	22.6	126.7	24.4	21.3
Level of Service	E	E	D	E	D	D		E	С	F	C	С
Approach Delay (s)		54.4			54.2			54.4			49.0	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			53.2	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacit	y ratio		0.92									
Actuated Cycle Length (s)			130.0	S	um of los	t time (s)			27.8			
Intersection Capacity Utilization	n		87.1%	IC	CU Level	of Service			E			
Analysis Period (min)			15									

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44₽			- 11	1				- ሽ		1
Traffic Vol, veh/h	140	138	0	0	647	173	0	0	0	20	0	28
Future Vol, veh/h	140	138	0	0	647	173	0	0	0	20	0	28
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage,	,# -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	95	95	95	92	92	92	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	159	157	0	0	681	182	0	0	0	27	0	37

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Conflicting Flow All	863	0	-	-	-	0		1062	-	341	
Stage 1	-	-	-	-	-	-		681	-	-	
Stage 2	-	-	-	-	-	-		381	-	-	
Critical Hdwy	4.14	-	-	-	-	-		6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-		5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-		6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-		3.67	-	3.32	
Pot Cap-1 Maneuver	775	-	0	0	-	-		251	0	655	
Stage 1	-	-	0	0	-	-		451	0	-	
Stage 2	-	-	0	0	-	-		625	0	-	
Platoon blocked, %		-			-	-					
Mov Cap-1 Maneuver	775	-	-	-	-	-		195	0	655	
Mov Cap-2 Maneuver	-	-	-	-	-	-		195	0	-	
Stage 1	-	-	-	-	-	-		350	0	-	
Stage 2	-	-	-	-	-	-		625	0	-	
Approach	ED			\\/D				CD			
	ED							17.0			
HCIVI Control Delay, s	5.0			U				17.3			
HCM LOS								C			
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	SBLn1	SBLn2				
Capacity (veh/h)		775	-	-	-	195	655				
HCM Lane V/C Ratio		0.205	-	-	-	0.137	0.057				
HCM Control Delay (s)	10.8	0.2	-	-	26.4	10.8				
HCM Lane LOS		В	А	-	-	D	В				
HCM 95th %tile Q(veh	I)	0.8	-	-	-	0.5	0.2				

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		441>			- 11	1				٦		1
Traffic Vol, veh/h	144	142	0	0	666	178	0	0	0	21	0	29
Future Vol, veh/h	144	142	0	0	666	178	0	0	0	21	0	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage	,# -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	95	95	95	92	92	92	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	164	161	0	0	701	187	0	0	0	28	0	39

Major/Minor	Major1		Ν	Aajor2				Minor2			
Conflicting Flow All	888	0	-	-	-	0		1093	-	351	
Stage 1	-	-	-	-	-	-		701	-	-	
Stage 2	-	-	-	-	-	-		392	-	-	
Critical Hdwy	4.14	-	-	-	-	-		6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-		5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-		6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-		3.67	-	3.32	
Pot Cap-1 Maneuver	758	-	0	0	-	-		241	0	645	
Stage 1	-	-	0	0	-	-		440	0	-	
Stage 2	-	-	0	0	-	-		616	0	-	
Platoon blocked, %		-			-	-					
Mov Cap-1 Maneuver	758	-	-	-	-	-		184	0	645	
Mov Cap-2 Maneuver	-	-	-	-	-	-		184	0	-	
Stage 1	-	-	-	-	-	-		335	0	-	
Stage 2	-	-	-	-	-	-		616	0	-	
Approach	EB			WB				SB			
HCM Control Delay, s	5.7			0				18.1			
HCM LOS								С			
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	BLn1	SBLn2				
Capacity (veh/h)		758	-	-	-	184	645				
HCM Lane V/C Ratio		0.216	-	-	-	0.152	0.06				
HCM Control Delay (s)		11.1	0.2	-	-	28	10.9				
HCM Lane LOS		В	A	-	-	D	В				
HCM 95th %tile Q(veh)	0.8	-	-	-	0.5	0.2				

Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		441			- 11	1				<u>۲</u>		1
Traffic Vol, veh/h	152	142	0	0	672	206	0	0	0	100	0	40
Future Vol, veh/h	152	142	0	0	672	206	0	0	0	100	0	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage	, # -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	95	95	95	92	92	92	75	75	75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	173	161	0	0	707	217	0	0	0	133	0	53

Major/Minor	Major1		ľ	Major2			Minor2			
Conflicting Flow All	924	0	-	-	- ()	1117	-	354	
Stage 1	-	-	-	-	-	-	707	-	-	
Stage 2	-	-	-	-	-	-	410	-	-	
Critical Hdwy	4.14	-	-	-	-	-	6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-	3.67	-	3.32	
Pot Cap-1 Maneuver	735	-	0	0	-	-	233	0	642	
Stage 1	-	-	0	0	-	-	437	0	-	
Stage 2	-	-	0	0	-	-	603	0	-	
Platoon blocked, %		-			-	-				
Mov Cap-1 Maneuver	735	-	-	-	-	-	173	0	642	
Mov Cap-2 Maneuver	• -	-	-	-	-	-	173	0	-	
Stage 1	-	-	-	-	-	-	324	0	-	
Stage 2	-	-	-	-	-	-	603	0	-	
Approach	ED			\//D			CD			
Approach							55.7			
HUM Control Delay, s	6 0			U			55. <i>1</i>			
HGM LOS							F			
Minor Lane/Major Mvr	mt	EBL	EBT	WBT	WBR SBLn	I SBLn2				
Capacity (veh/h)		735	-	-	- 173	3 642				
HCM Lane V/C Ratio		0.235	-	-	- 0.77	0.083				
HCM Control Delay (s	5)	11.4	0.2	-	- 73.6	5 11.1				
HCM Lane LOS		В	А	-	- F	= B				

0.3

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0.9

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HCM 95th %tile Q(veh)

Intersection Delay, s/veh Intersection LOS

14.9

В

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		413			^	1				7		1
Traffic Vol, veh/h	152	142	0	0	672	206	0	0	0	100	0	40
Future Vol, veh/h	152	142	0	0	672	206	0	0	0	100	0	40
Peak Hour Factor	0.88	0.88	0.88	0.95	0.95	0.95	0.92	0.92	0.92	0.75	0.75	0.75
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	173	161	0	0	707	217	0	0	0	133	0	53
Number of Lanes	0	3	0	0	2	1	0	0	0	1	0	1
Approach	EB				WB					SB		
Opposing Approach	WB				EB							
Opposing Lanes	3				3					0		
Conflicting Approach Left	SB									WB		
Conflicting Lanes Left	2				0					3		
Conflicting Approach Right					SB					EB		
Conflicting Lanes Right	0				2					3		
HCM Control Delay	13.5				15.8					13.2		
HCM LOS	В				С					В		

Lane	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	
Vol Left, %	84%	0%	0%	0%	0%	0%	100%	0%	
Vol Thru, %	16%	100%	100%	100%	100%	0%	0%	0%	
Vol Right, %	0%	0%	0%	0%	0%	100%	0%	100%	
Sign Control	Stop								
Traffic Vol by Lane	180	57	57	336	336	206	100	40	
LT Vol	152	0	0	0	0	0	100	0	
Through Vol	28	57	57	336	336	0	0	0	
RT Vol	0	0	0	0	0	206	0	40	
Lane Flow Rate	205	65	65	354	354	217	133	53	
Geometry Grp	8	8	8	8	8	8	8	8	
Degree of Util (X)	0.426	0.126	0.095	0.611	0.611	0.226	0.299	0.102	
Departure Headway (Hd)	7.482	7.054	5.295	6.216	6.216	3.752	8.081	6.875	
Convergence, Y/N	Yes								
Сар	480	505	671	580	580	948	443	518	
Service Time	5.262	4.834	3.073	3.979	3.979	1.513	5.866	4.66	
HCM Lane V/C Ratio	0.427	0.129	0.097	0.61	0.61	0.229	0.3	0.102	
HCM Control Delay	15.8	10.9	8.6	18.3	18.3	7.6	14.3	10.4	
HCM Lane LOS	С	В	А	С	С	А	В	В	
HCM 95th-tile Q	2.1	0.4	0.3	4.1	4.1	0.9	1.2	0.3	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		441>			^	1				٦		1
Traffic Vol, veh/h	38	667	0	0	176	17	0	0	0	73	0	61
Future Vol, veh/h	38	667	0	0	176	17	0	0	0	73	0	61
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage,	, # -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	80	80	80	92	92	92	68	68	68
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	43	749	0	0	220	21	0	0	0	107	0	90

Major/Minor	Major1		Ν	Aajor2				Minor2			
Conflicting Flow All	241	0	-	-	-	0		606	-	110	
Stage 1	-	-	-	-	-	_		220	-	-	
Stage 2	-	-	-	-	-	-		386	-	-	
Critical Hdwy	4.14	-	-	-	-	-		6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-		5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-		6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-		3.67	-	3.32	
Pot Cap-1 Maneuver	1323	-	0	0	-	-		456	0	922	
Stage 1	-	-	0	0	-	-		766	0	-	
Stage 2	-	-	0	0	-	-		621	0	-	
Platoon blocked, %		-			-	-					
Mov Cap-1 Maneuver	1323	-	-	-	-	-		430	0	922	
Mov Cap-2 Maneuver	-	-	-	-	-	-		430	0	-	
Stage 1	-	-	-	-	-	-		723	0	-	
Stage 2	-	-	-	-	-	-		621	0	-	
Approach	EB			WB				SB			
HCM Control Delay, s	0.5			0				13			
HCM LOS								В			
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR S	SBLn1	SBLn2				
Capacity (veh/h)		1323	-	-	-	430	922				
HCM Lane V/C Ratio		0.032	-	-	-	0.25	0.097				
HCM Control Delay (s))	7.8	0.1	-	-	16.1	9.3				
HCM Lane LOS		А	А	-	-	С	А				
HCM 95th %tile Q(veh)	0.1	-	-	-	1	0.3				

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		441>			- 11	1				٦		1
Traffic Vol, veh/h	39	687	0	0	181	18	0	0	0	75	0	63
Future Vol, veh/h	39	687	0	0	181	18	0	0	0	75	0	63
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage	, # -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	80	80	80	92	92	92	68	68	68
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	44	772	0	0	226	23	0	0	0	110	0	93

Major/Minor	Major1		Ν	/lajor2				Minor2			
Conflicting Flow All	249	0	-	-	-	0		623	-	113	
Stage 1	-	-	-	-	-	-		226	-	-	
Stage 2	-	-	-	-	-	-		397	-	-	
Critical Hdwy	4.14	-	-	-	-	-		6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-		5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-		6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-		3.67	-	3.32	
Pot Cap-1 Maneuver	1314	-	0	0	-	-		446	0	918	
Stage 1	-	-	0	0	-	-		761	0	-	
Stage 2	-	-	0	0	-	-		613	0	-	
Platoon blocked, %		-			-	-					
Mov Cap-1 Maneuver	1314	-	-	-	-	-		420	0	918	
Mov Cap-2 Maneuver	-	-	-	-	-	-		420	0	-	
Stage 1	-	-	-	-	-	-		716	0	-	
Stage 2	-	-	-	-	-	-		613	0	-	
Approach	EB			WB				SB			
HCM Control Delay, s	0.5			0				13.3			
HCM LOS								В			
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1	SBLn2				
Capacity (veh/h)		1314	-	-	-	420	918				
HCM Lane V/C Ratio		0.033	-	-	-	0.263	0.101				
HCM Control Delay (s))	7.8	0.1	-	-	16.6	9.4				
HCM Lane LOS		А	А	-	-	С	А				
HCM 95th %tile Q(veh)	0.1	-	-	-	1	0.3				

Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44₽			- 11	1				<u>۲</u>		1
Traffic Vol, veh/h	51	687	0	0	182	29	0	0	0	156	0	65
Future Vol, veh/h	51	687	0	0	182	29	0	0	0	156	0	65
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	5	-	-	0	-	-	-	135	-	0
Veh in Median Storage,	# -	0	-	-	0	-	-	16965	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	80	80	80	92	92	92	68	68	68
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
M∨mt Flow	57	772	0	0	228	36	0	0	0	229	0	96

Major/Minor	Major1			Major2			Minor2			
Conflicting Flow All	264	0	-	-	-	0	651	-	114	
Stage 1	-	-	-	-	-	-	228	-	-	
Stage 2	-	-	-	-	-	-	423	-	-	
Critical Hdwy	4.14	-	-	-	-	-	6.29	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.84	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.04	-	-	
Follow-up Hdwy	2.22	-	-	-	-	-	3.67	-	3.32	
Pot Cap-1 Maneuver	1297	-	0	0	-	-	430	0	917	
Stage 1	-	-	0	0	-	-	759	0	-	
Stage 2	-	-	0	0	-	-	594	0	-	
Platoon blocked, %		-			-	-				
Mov Cap-1 Maneuver	1297	-	-	-	-	-	397	0	917	
Mov Cap-2 Maneuver	-	-	-	-	-	-	397	0	-	
Stage 1	-	-	-	-	-	-	701	0	-	
Stage 2	-	-	-	-	-	-	594	0	-	
Annroach	EB			\//R			SB			
HCM Control Dology				0						
HCM LOS	0.7			0			21			
							U			
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR SBL	n1 SBLn2				
Capacity (veh/h)		1297	-	-	- 3	97 917				
HCM Lane V/C Ratio		0.044	-	-	- 0.5	78 0.104				
HCM Control Delay (s	;)	7.9	0.2	-	- 25	5.8 9.4				
HCM Lane LOS	-	А	Α	-	-	D A				

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HCM 95th %tile Q(veh)

Intersection 14.2

В

Intersection Delay, s/veh Intersection LOS

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		413			^	1				7		1
Traffic Vol, veh/h	51	687	0	0	182	29	0	0	0	156	0	65
Future Vol, veh/h	51	687	0	0	182	29	0	0	0	156	0	65
Peak Hour Factor	0.89	0.89	0.89	0.80	0.80	0.80	0.92	0.92	0.92	0.68	0.68	0.68
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	57	772	0	0	228	36	0	0	0	229	0	96
Number of Lanes	0	3	0	0	2	1	0	0	0	1	0	1
Approach	EB				WB					SB		
Opposing Approach	WB				EB							
Opposing Lanes	3				3					0		
Conflicting Approach Left	SB									WB		
Conflicting Lanes Left	2				0					3		
Conflicting Approach Right					SB					EB		
Conflicting Lanes Right	0				2					3		
HCM Control Delay	14.1				12.1					16.1		
HCM LOS	В				В					С		

Lane	EBLn1	EBLn2	EBLn3	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	
Vol Left, %	27%	0%	0%	0%	0%	0%	100%	0%	
Vol Thru, %	73%	100%	100%	100%	100%	0%	0%	0%	
Vol Right, %	0%	0%	0%	0%	0%	100%	0%	100%	
Sign Control	Stop								
Traffic Vol by Lane	188	275	275	91	91	29	156	65	
LT Vol	51	0	0	0	0	0	156	0	
Through Vol	137	275	275	91	91	0	0	0	
RT Vol	0	0	0	0	0	29	0	65	
Lane Flow Rate	212	309	309	114	114	36	229	96	
Geometry Grp	8	8	8	8	8	8	8	8	
Degree of Util (X)	0.393	0.561	0.41	0.239	0.239	0.051	0.502	0.177	
Departure Headway (Hd)	6.789	6.651	4.883	7.573	7.573	5.081	7.881	6.678	
Convergence, Y/N	Yes								
Сар	533	547	741	477	477	707	461	540	
Service Time	4.489	4.351	2.583	5.288	5.288	2.796	5.581	4.378	
HCM Lane V/C Ratio	0.398	0.565	0.417	0.239	0.239	0.051	0.497	0.178	
HCM Control Delay	13.8	17.5	10.9	12.7	12.7	8.1	18.3	10.8	
HCM Lane LOS	В	С	В	В	В	А	С	В	
HCM 95th-tile Q	1.9	3.4	2	0.9	0.9	0.2	2.8	0.6	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ካካ							1111	
Traffic Volume (veh/h)	0	0	0	65	0	0	0	0	0	0	2478	0
Future Volume (Veh/h)	0	0	0	65	0	0	0	0	0	0	2478	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.77	0.77	0.77	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	84	0	0	0	0	0	0	2608	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	2608	2608	652	652	2608	0	2608			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2608	2608	652	652	2608	0	2608			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	76	100	100	100			100		
cM capacity (veh/h)	12	24	411	353	24	1084	162			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	42	42	652	652	652	652						
Volume Left	42	42	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	353	353	1700	1700	1700	1700						
Volume to Capacity	0.12	0.12	0.38	0.38	0.38	0.38						
Queue Length 95th (ft)	10	10	0	0	0	0						
Control Delay (s)	16.6	16.6	0.0	0.0	0.0	0.0						
Lane LOS	С	С										
Approach Delay (s)	16.6		0.0									
Approach LOS	С											
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utiliz	zation		61.4%	IC	CU Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ሻሻ							1111	
Traffic Volume (veh/h)	0	0	0	67	0	0	0	0	0	0	2552	0
Future Volume (Veh/h)	0	0	0	67	0	0	0	0	0	0	2552	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.77	0.77	0.77	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	87	0	0	0	0	0	0	2686	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	2686	2686	672	672	2686	0	2686			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2686	2686	672	672	2686	0	2686			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	75	100	100	100			100		
cM capacity (veh/h)	10	21	399	342	21	1084	151			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	44	44	672	672	672	672						
Volume Left	44	44	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	342	342	1700	1700	1700	1700						
Volume to Capacity	0.13	0.13	0.40	0.40	0.40	0.40						
Queue Length 95th (ft)	11	11	0	0	0	0						
Control Delay (s)	17.1	17.1	0.0	0.0	0.0	0.0						
Lane LOS	С	С										
Approach Delay (s)	17.1		0.0									
Approach LOS	С											
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utiliz	ation		63.0%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ሻሻ							1111	
Traffic Volume (veh/h)	0	0	0	112	0	0	0	0	0	0	2627	0
Future Volume (Veh/h)	0	0	0	112	0	0	0	0	0	0	2627	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.77	0.77	0.77	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	145	0	0	0	0	0	0	2765	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	2765	2765	691	691	2765	0	2765			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	2765	2765	691	691	2765	0	2765			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	56	100	100	100			100		
cM capacity (veh/h)	9	19	387	331	19	1084	141			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	72	72	691	691	691	691						
Volume Left	72	72	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	331	331	1700	1700	1700	1700						
Volume to Capacity	0.22	0.22	0.41	0.41	0.41	0.41						
Queue Length 95th (ft)	21	21	0	0	0	0						
Control Delay (s)	18.9	18.9	0.0	0.0	0.0	0.0						
Lane LOS	С	С										
Approach Delay (s)	18.9		0.0									
Approach LOS	С											
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utiliz	ation		64.6%	IC	CU Level o	of Service			С			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ሻሻ							1111	
Traffic Volume (veh/h)	0	0	0	38	0	0	0	0	0	0	1007	0
Future Volume (Veh/h)	0	0	0	38	0	0	0	0	0	0	1007	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.79	0.79	0.79	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	48	0	0	0	0	0	0	1060	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	1060	1060	265	265	1060	0	1060			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1060	1060	265	265	1060	0	1060			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	178	223	733	666	223	1084	653			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	24	24	265	265	265	265						
Volume Left	24	24	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	666	666	1700	1700	1700	1700						
Volume to Capacity	0.04	0.04	0.16	0.16	0.16	0.16						
Queue Length 95th (ft)	3	3	0	0	0	0						
Control Delay (s)	10.6	10.6	0.0	0.0	0.0	0.0						
Lane LOS	В	В										
Approach Delay (s)	10.6		0.0									
Approach LOS	В											
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utiliz	ation		60.6%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ሻሻ							1111	
Traffic Volume (veh/h)	0	0	0	39	0	0	0	0	0	0	1038	0
Future Volume (Veh/h)	0	0	0	39	0	0	0	0	0	0	1038	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.79	0.79	0.79	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	49	0	0	0	0	0	0	1093	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	1093	1093	273	273	1093	0	1093			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1093	1093	273	273	1093	0	1093			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	93	100	100	100			100		
cM capacity (veh/h)	169	213	724	658	213	1084	634			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	24	24	273	273	273	273						
Volume Left	24	24	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	658	658	1700	1700	1700	1700						
Volume to Capacity	0.04	0.04	0.16	0.16	0.16	0.16						
Queue Length 95th (ft)	3	3	0	0	0	0						
Control Delay (s)	10.7	10.7	0.0	0.0	0.0	0.0						
Lane LOS	В	В										
Approach Delay (s)	10.7		0.0									
Approach LOS	В											
Intersection Summary												
Average Delav			0.5									
Intersection Capacity Utiliza	tion		62.2%	IC	CU Level o	of Service			В			
Analysis Period (min)	· · · ·		15						_			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				ካካ							1111	
Traffic Volume (veh/h)	0	0	0	103	0	0	0	0	0	0	1055	0
Future Volume (Veh/h)	0	0	0	103	0	0	0	0	0	0	1055	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.79	0.79	0.79	0.92	0.92	0.92	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	130	0	0	0	0	0	0	1111	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (ft)								746				
pX, platoon unblocked												
vC, conflicting volume	1111	1111	278	278	1111	0	1111			0		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1111	1111	278	278	1111	0	1111			0		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	80	100	100	100			100		
cM capacity (veh/h)	164	208	720	653	208	1084	624			1622		
Direction, Lane #	WB 1	WB 2	SB 1	SB 2	SB 3	SB 4						
Volume Total	65	65	278	278	278	278						
Volume Left	65	65	0	0	0	0						
Volume Right	0	0	0	0	0	0						
cSH	653	653	1700	1700	1700	1700						
Volume to Capacity	0.10	0.10	0.16	0.16	0.16	0.16						
Queue Length 95th (ft)	8	8	0	0	0	0						
Control Delay (s)	11.1	11.1	0.0	0.0	0.0	0.0						
Lane LOS	В	В										
Approach Delay (s)	11.1		0.0									
Approach LOS	В											
Intersection Summary												
Average Delay			1.2									
Intersection Capacity Utiliz	ation		63.1%	IC	CU Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			tttts.	
Traffic Volume (veh/h)	0	113	0	0	2614	125
Future Volume (Veh/h)	0	113	0	Ū	2614	125
Sign Control	Stop		· ·	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.95	0.95
Hourly flow rate (vph)	0	123	0	0	2752	132
Pedestrians	-		-	-		
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				,		
Upstream signal (ft)				526		
pX, platoon unblocked				,		
vC, conflicting volume	2818	754	2884			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2818	754	2884			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	65	100			
cM capacity (veh/h)	14	352	126			
Direction. Lane #	FB 1	SB 1	SB 2	SB 3	SB 4	
Volume Total	123	786	786	786	525	
Volume Left	0	0	0	0	0	
Volume Right	123	0	0	0	132	
cSH	352	1700	1700	1700	1700	
Volume to Capacity	0.35	0.46	0 46	0 46	0.31	
Queue Length 95th (ft)	38	0.10	0	0.10	0	
Control Delay (s)	20.6	0.0	0.0	0.0	0.0	
Lane LOS	20.0 C	0.0	0.0	0.0	0.0	
Approach Delay (s)	20.6	0.0				
Approach LOS	C	0.0				
Interpretion Summer	Ŭ					
Average Delsu			0.0			
Average Delay	ation		0.8			4 Can de a
Analysis Deried (min)	allon		31.3% 1E	IC		DI SEIVICE
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			tttt	•==
Traffic Volume (veh/h)	0	42	0	0	1094	64
Future Volume (Veh/h)	0	42	0	0	1094	64
Sign Control	Ston	15	Ŭ	Free	Free	01
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0 92	0 92	0.92	0.95	0.95
Hourly flow rate (yph)	0.02	46	0.02	0.02	1152	67
Pedestrians	Ū	10	Ŭ	Ū	1102	01
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage yeb)				NULLE	NULLE	
Instream signal (ft)				526		
nX platoon unblocked				520		
vC conflicting volume	1196	300	1210			
	1100	JZZ	1213			
vC1, stage 1 confi vol						
	1186	300	1210			
tC single (s)	68	60	1213			
tC, single (s) $tC = 2 \text{ stars}(s)$	0.0	0.5	4.1			
tC, Z stage (s)	3.5	33	2.2			
n^{0} quoto froc %	100	0.0	100			
oM consoity (yoh/h)	100	95	568			
	102	0/4	500			
Direction, Lane #	EB 1	SB 1	SB 2	SB 3	SB 4	
Volume Total	46	329	329	329	232	
Volume Left	0	0	0	0	0	
Volume Right	46	0	0	0	67	
cSH	674	1700	1700	1700	1700	
Volume to Capacity	0.07	0.19	0.19	0.19	0.14	
Queue Length 95th (ft)	5	0	0	0	0	
Control Delay (s)	10.7	0.0	0.0	0.0	0.0	
Lane LOS	В					
Approach Delay (s)	10.7	0.0				
Approach LOS	В					
Intersection Summary						
Average Delay			0.4			
Intersection Canacity Litilize	ation		26.1%	IC		of Service
Analysis Period (min)			15			

Int Delay, s/veh	0.1							
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		^	朴朴			1		
Traffic Vol, veh/h	0	242	872	78	0	6		
Future Vol, veh/h	0	242	872	78	0	6		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Stop	Stop		
RT Channelized	-	None	-	None	-	None		
Storage Length	-	-	-	-	-	0		
Veh in Median Storage,	# -	0	0	-	0	-		
Grade, %	-	0	0	-	0	-		
Peak Hour Factor	76	76	95	95	92	92		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	0	318	918	82	0	7		

Major/Minor	Major1	ľ	Major2	Mir	nor2	
Conflicting Flow All	-	0	-	0	-	500
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	7.14
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.92
Pot Cap-1 Maneuver	0	-	-	-	0	442
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	-	-	-	-	-	442
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0	-	13.3	
HCM LOS					В	
Minor Lane/Maior Myr	nt	FRT	WRT	WBR SB	ln1	
Canacity (yeh/h)				-	1/2	
HCM Lane V/C Ratio		_	_	- 0	442 015	
HCM Control Delay (s)	-	-	- 0.	12.2	
HCM Lane LOS)		_	_	R	
HCM 95th %tile Q(veh	1)	-	_	-	0	

Int Delay, s/veh	0						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		^	朴朴			1	
Traffic Vol, veh/h	0	843	210	40	0	1	
Future Vol, veh/h	0	843	210	40	0	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	-	0	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	93	93	80	80	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	906	263	50	0	1	

Major/Minor	Major1	ľ	Major2	N	linor2			
Conflicting Flow All	-	0	-	0	-	157		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Critical Hdwy	-	-	-	-	-	7.14		
Critical Hdwy Stg 1	-	-	-	-	-	-		
Critical Hdwy Stg 2	-	-	-	-	-	-		
Follow-up Hdwy	-	-	-	-	-	3.92		
Pot Cap-1 Maneuver	0	-	-	-	0	732		
Stage 1	0	-	-	-	0	-		
Stage 2	0	-	-	-	0	-		
Platoon blocked, %		-	-	-				
Mov Cap-1 Maneuver	r –	-	-	-	-	732		
Mov Cap-2 Maneuver	r -	-	-	-	-	-		
Stage 1	-	-	-	-	-	-		
Stage 2	-	-	-	-	-	-		
Approach	EB		WB		SB			
HCM Control Delay, s	s 0		0		9.9			
HCM LOS					А			
Minor Lane/Major Mv	mt	EBT	WBT	WBR S	BLn1			
Capacity (veh/h)		-	-	_	732			
HCM Lane V/C Ratio		-	-	-	0.001			
HCM Control Delay (s	5)	-	-	-	9.9			
HCM Lane LOS	,	-	-	-	A			
HCM 95th %tile Q(ve	h)	-	-	-	0			

Int Delay, s/veh	2.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	-
Lane Configurations	Y		et 👘		۲,	•	
Traffic Vol, veh/h	90	0	322	36	0	50)
Future Vol, veh/h	90	0	322	36	0	50)
Conflicting Peds, #/hr	0	0	0	0	0	0)
Sign Control	Stop	Stop	Free	Free	Free	Free	9
RT Channelized	-	None	-	None	-	None	;
Storage Length	0	-	-	-	100	-	-
Veh in Median Storage	,# 0	-	0	-	-	0)
Grade, %	0	-	0	-	-	0)
Peak Hour Factor	92	92	92	92	75	75	;
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	98	0	350	39	0	67	•

Major/Minor	Minor1	Ν	1ajor1	М	ajor2		
Conflicting Flow All	437	370	0	0	389	0	
Stage 1	370	-	-	-	-	-	
Stage 2	67	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	- 2	2.218	-	
Pot Cap-1 Maneuver	577	676	-	-	1170	-	
Stage 1	699	-	-	-	-	-	
Stage 2	956	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	577	676	-	-	1170	-	
Mov Cap-2 Maneuver	611	-	-	-	-	-	
Stage 1	699	-	-	-	-	-	
Stage 2	956	-	-	-	-	-	
Approach	WB		NB		SB		

Approach	WB	NB	SB	
HCM Control Delay, s	12	0	0	
HCM LOS	В			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT	
Capacity (veh/h)	-	-	611	1170	-	
HCM Lane V/C Ratio	-	-	0.16	-	-	
HCM Control Delay (s)	-	-	12	0	-	
HCM Lane LOS	-	-	В	Α	-	
HCM 95th %tile Q(veh)	-	-	0.6	0	-	

Int Delay, s/veh	2.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4		٦	1
Traffic Vol, veh/h	83	0	57	23	0	138
Future Vol, veh/h	83	0	57	23	0	138
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	86	86	68	68
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	90	0	66	27	0	203

Major/Minor	Minor1	N	lajor1	М	ajor2	
Conflicting Flow All	283	80	0	0	93	0
Stage 1	80	-	-	-	-	-
Stage 2	203	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	- 2	2.218	-
Pot Cap-1 Maneuver	707	980	-	-	1501	-
Stage 1	943	-	-	-	-	-
Stage 2	831	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	707	980	-	-	1501	-
Mov Cap-2 Maneuver	720	-	-	-	-	-
Stage 1	943	-	-	-	-	-
Stage 2	831	-	-	-	-	-
A					00	

Approach	WB	NB	SB	
HCM Control Delay, s	10.7	0	0	
HCM LOS	В			

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT	
Capacity (veh/h)	-	- 720	1501	-	
HCM Lane V/C Ratio	-	- 0.125	-	-	
HCM Control Delay (s)	-	- 10.7	0	-	
HCM Lane LOS	-	- B	А	-	
HCM 95th %tile Q(veh)	-	- 0.4	0	-	

INTERNAL CAPTURE

WORKSHEETS
	NCHRP 684 Internal Trip C	ap	ture Estimation Tool	
Project Name:	Troy Plaza Mixed-Use Development		Organization:	ROWE Professional Services Company
Project Location:	City of Troy, Oakland County, MI		Performed By:	J. Bauer
Scenario Description:	Site Build Out		Date:	7/13/2019
Analysis Year:	2020		Checked By:	M. Labadie
Analysis Period:	AM Street Peak Hour		Date:	7/15/2019

	Table 1	-A: Base Vehic	e-Trip Generatio	ı Es	timates (Single-Use Sit	e Estimate)		
	Developme	ent Data (<i>For In</i>	formation Only)		Estimated Vehicle-Trips ³			
Land Use	ITE LUCs ¹	Quantity	Units		Total	Entering	Exiting	
Office	710, 720	10,692	sq. ft.		41	34	7	
Retail	820	3,232	sq. ft.		3	2	1	
Restaurant	932,937,940	10,986	sq. ft.		382	196	186	
Cinema/Entertainment					0			
Residential					0			
Hotel	312	133	rooms		52	22	30	
All Other Land Uses ²					0			
					478	254	224	

-							
		Table 2-A:	Mode Split and Vehi	icle C	Occupancy Estimates	5	
Land Lico		Entering Tr	ips			Exiting Trips	
Land Ose	Veh. Occ. ⁴	% Transit	% Non-Motorized		Veh. Occ.4	% Transit	% Non-Motorized
Office							
Retail							
Restaurant							
Cinema/Entertainment							
Residential							
Hotel							
All Other Land Uses ²							

	Table 3	3-A: Average La	and Use Interchan	ge Distances (Feet Walking	g Distance)				
Origin (From)		Destination (To)							
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office									
Retail									
Restaurant									
Cinema/Entertainment									
Residential									
Hotel									

		Table 4-A: I	nternal Person-Tri	p Origin-Destination Matrix	*				
Origin (From)	Destination (To)								
Oligin (Flom)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		1	4	0	0	0			
Retail	0		0	0	0	0			
Restaurant	5	0		0	0	1			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	0	0	0		0			
Hotel	1	0	3	0	0				

Table 5-A	: Computatio	ons Summary		Table 6-A: Internal Trip Capture Percentages by Land Use		
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting Tri
All Person-Trips	478	254	224	Office	18%	71%
Internal Capture Percentage	6%	6%	7%	Retail	50%	0%
•			•	Restaurant	4%	3%
External Vehicle-Trips ⁵	448	239	209	Cinema/Entertainment	N/A	N/A
External Transit-Trips ⁶	0	0	0	Residential	N/A	N/A
External Non-Motorized Trips ⁶	0	0	0	Hotel	5%	13%

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

	NCHRP 684 Internal Trip Capture Estimation Tool						
Project Name:	Troy Plaza Mixed-Use Development		Organization:	ROWE Professional Services Company			
Project Location:	City of Troy, Oakland County, MI		Performed By:	J. Bauer			
Scenario Description:	Site Build Out		Date:	7/13/2019			
Analysis Year:	2020		Checked By:	M. Labadie			
Analysis Period:	PM Street Peak Hour		Date:	7/15/2019			

	Table 1-	P: Base Vehicl	e-Trip Generation	ı Es	timates (Single-Use Si	te Estimate)		
L and Llas	Development Data (For Information Only)				Estimated Vehicle-Trips ³			
Land Ose	ITE LUCs ¹	Quantity	Units	1	Total	Entering	Exiting	
Office	710, 720	10,692	sq. ft.	1	19	4	15	
Retail	820	3,232	sq. ft.	1	43	21	22	
Restaurant	932,937,940	10,986	sq. ft.	1	218	113	105	
Cinema/Entertainment				1	0			
Residential				1	0			
Hotel	312	133	rooms	1	43	24	19	
All Other Land Uses ²				1	0			
					323	162	161	

		Table 2-P:	Mode Split and Veh	icle	Occupancy Estimates		
Land Lisa		Entering Tri	ps			Exiting Trips	
Land Ose	Veh. Occ. ⁴	% Transit	% Non-Motorized	Γ	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office							
Retail							
Restaurant							
Cinema/Entertainment							
Residential							
Hotel							
All Other Land Uses ²							

	Table 3	3-P: Average L	and Use Interchan	ge Distances (Feet Walking	J Distance)	
Origin (From)				Destination (To)		
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

		Table 4-P: Ir	nternal Person-Tri	p Origin-Destination Matrix	*				
Origin (From)	Destination (To)								
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		2	1	0	0	0			
Retail	0		6	0	0	1			
Restaurant	1	11		0	0	7			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	0	0	0		0			
Hotel	0	0	6	0	0				

Table 5-P	: Computatio	ons Summary		Table 6-P: Internal Trip Capture Percentages by Land Use		
	Total	Entering	Exiting	Land Use	Entering Trips	Exiting
All Person-Trips	323	162	161	Office	25%	20
Internal Capture Percentage	22%	22%	22%	Retail	62%	329
				Restaurant	12%	189
External Vehicle-Trips ⁵	253	127	126	Cinema/Entertainment	N/A	N/A
External Transit-Trips ⁶	0	0	0	Residential	N/A	N/A
External Non-Motorized Trips ⁶	0	0	0	Hotel	33%	329

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers. ²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be ⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

ROAD COMMISSION FOR OAKLAND COUNTY

RIGHT-TURN TREATMENT

WARRANT ANALYSIS

ROAD COMMISSION FOR OAKLAND COUNTY PERMIT RULES, SPECIFICATIONS AND GUIDELINES



ROAD COMMISSION FOR OAKLAND COUNTY DEPARTMENT OF CUSTOMER SERVICES PERMITS DIVISION 2420 PONTIAC LAKE ROAD WATERFORD, MI 48328

MARCH 14, 2013

FIGURE 6-3



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SHARED PARKING

WORKSHEETS

						ULI Table	2-2 Ratios	Adj. Parking Rates b	ased on ULI Table 2-2
			Required ULI		W	/eekday	Weekend	Weekday	Weekend
ULI Land Use	Size	City Parking Requirement	Parking Rate		Visitor	/ Employee	Visitor / Employee	Visitor / Employee	Visitor / Employee
Hotel - Business	133 rooms 5	employ. 1 per room & 1 per employ.	138 1.04	1.0400	n/a	/ n/a	n/a / n/a	1.00 / 0.04	1.00 / 0.04
Office < 25ksf	6813 sq. ft.	1 per 300 sq. ft. & per	23 3.33	80%	8%	/ 92%	8% / 92%	0.27 / 3.06	0.27 / 3.06
Medical/Dental Office	1850 sq. ft.	1 per 200 sq. ft. & per	9 5.00	85%	67%	/ 33%	67% / 33%	3.35 / 1.65	3.35 / 1.65
Community Shopping Center	3232 sq. ft.	1 per 250 sq. ft. & per	13 4.00		81%	/ 19%	80% / 20%	3.24 / 0.76	3.20 / 0.80
Family Restaurant	110 seats	sq.ft. 1 per 2 seats & per	55 0.50		86%	/ 14%	85% / 15%	0.43 / 0.07	0.42 / 0.08
	2072 sq. ft.	1 per 70 sq.ft. & per	30 14.29 1	_ 14.29	85%	/ 15%	86% / 14%	12.15 / 2.14	12.29 / 2.00
Fast Food Restaurant	4414 sq. ft.	1 per 70 sq.ft. & per	63 93 14.29 70	1000	85%	/ 15%	86% / 14%	12.15 / 2.14	12.29 / 2.00
	0 sq. ft.	1 per 70 sq.ft. & per	0 14.29		85%	/ 15%	86% / 14%	12.15 / 2.14	12.29 / 2.00
	21514	Parking provided on site plan: 266	331						

Description: Mixed-Use Development

ksf = thousand square feet

Max Parking Spore Week-day Week-day Week-day Week-day Week-day Community, Stopping Center (400 kef)	Projected Parking Supply:	ly: 252					Mode Ad	ljustment		Noncaptive Ratio			
Land UeeOutputVerticityDeptitePointPoi				Max Parki	ng Spaces	Wee	kday	Wee	kend	Weekday Weekend			kend
Community Stopping Camer (400 km) 3.322 af CLA 100 100% <th>Land Use</th> <th>Qua</th> <th>ntity</th> <th>Weekday</th> <th>Weekend</th> <th>Daytime</th> <th>Evening</th> <th>Daytime</th> <th>Evening</th> <th>Daytime</th> <th>Evening</th> <th>Daytime</th> <th>Evening</th>	Land Use	Qua	ntity	Weekday	Weekend	Daytime	Evening	Daytime	Evening	Daytime	Evening	Daytime	Evening
Employee P 2 3 100%	Community Shopping Center (<400 ksf)	3,232	sf GLA	10	10	100%	100%	100%	100%	100%	100%	100%	100%
Regional Shopping Center (MOD to BOD set) of CAL 0 0 100%	Employee			2	3	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Regional Shopping Center (400 to 600 ksf)		sf GLA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Super Regenal Shopping Centre (r600 ksf) of (A) 0.0 0.00% 100%<	Employee			0	0	100%	100%	100%	100%	100%	100%	100%	100%
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Super Regional Shopping Center (>600 ksf)		sf GLA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Fine/Casal Dring Restaurant pf GFA 0 0 100% 10	Employee			0	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 00 100%	Fine/Casual Dining Restaurant		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Family Restaurant 100 Seats 440 450 100%	Employee			0	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 7 8 100% 1	Family Restaurant	106	Seats	46	45	100%	100%	100%	100%	100%	100%	100%	100%
Fact Food Restaurant. 6.488 of GFA 79 80 100%	Employee			7	8	100%	100%	100%	100%	100%	100%	100%	100%
Employee 14 13 100% <th< td=""><td>Fast Food Restaurant</td><td>6,486</td><td>sf GFA</td><td>79</td><td>80</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></th<>	Fast Food Restaurant	6,486	sf GFA	79	80	100%	100%	100%	100%	100%	100%	100%	100%
Nighteub. end/cyce IGFA 0. 0. 100%	Employee			14	13	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Nightclub		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Cineptoxe esats 0 0 100% <th< td=""><td>Employee</td><td></td><td></td><td>0</td><td>0</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></th<>	Employee			0	0	100%	100%	100%	100%	100%	100%	100%	100%
Embloyee 0 0 100% 1	Cineplex		seats	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Performing Arts Theater seats 0 0 100% <td>Employee</td> <td></td> <td></td> <td>0</td> <td>0</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td>	Employee			0	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Performing Arts Theater		seats	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Aren Seats 0 0 100%	Employee		<u> </u>	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Arena		seats	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Par 6 actional Stadium seats 0 0 100% <td>Employee</td> <td></td> <td>oouto</td> <td><u>0</u></td> <td><u>0</u></td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td>	Employee		oouto	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 00% 100% <th< td=""><td>Pro Football Stadium</td><td></td><td>seats</td><td>0</td><td>0</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></th<>	Pro Football Stadium		seats	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Pro Basehall Stadium seats 0 0 100%	Employee		oouto	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Pro Baseball Stadium		seats	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Iteatin Chub af GFA 0 0 100%	Employee		oouto	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Health Club		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Convention Center sf GLA 0 0 100%	Employee			<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Construction Construction<	Convention Center		sf GL A	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Hotel-Eusiness 133 133 133 133 100%	Employee		01 02/1	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
HoteLeisure rooms 0 0 100%	Hotel-Business	133	rooms	133	133	100%	100%	100%	100%	100%	100%	100%	100%
Restaurant/Lounge si GFA 0 0 100%	Hotel-Leisure	100	rooms	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Conference Ctr/Banquet (20 to 50 sq ft/guest room) sf GFA 0 0 100% 1	Restaurant/Lounge		sf GFA	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Conventions Space (>50 sq ft/guest room) sf GLA 0 0 100%	Conference Ctr/Banquet (20 to 50 sq ft/quest room)		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 5 5 100% 1	Convention Space (>50 sq ft/quest room)		sf GLA	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Besidential, Rental, Shared Spaces units 0 0 100%	Employee		01 02/1	5	5	100%	100%	100%	100%	100%	100%	100%	100%
Reserved 1 sp/unit 0 100%	Residential Rental Shared Spaces		units	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Guest Initis 0 100% <th< td=""><td>Reserved</td><td>1</td><td>sn/unit</td><td><u>0</u></td><td><u>0</u></td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></th<>	Reserved	1	sn/unit	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Besidential, Owned, Shared Spaces units 0 0 100% <	Guest		units	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Reserved 15p/unit 0 0 100% <	Residential Owned Shared Spaces		units	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Guest Inits 0 100%	Reserved	1	sn/unit	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Odfice 25 ksf 6,813 sf GFA 2 2 100%	Guest		units	<u>0</u>	<u>0</u>	100%	100%	100%	100%	100%	100%	100%	100%
Employee 21 21 100% <th< td=""><td>Office <25 ksf</td><td>6.813</td><td>sf GFA</td><td>2</td><td>2</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></th<>	Office <25 ksf	6.813	sf GFA	2	2	100%	100%	100%	100%	100%	100%	100%	100%
Lings/sec Li Li <thli< th=""> Li Li <</thli<>	Employee	0,010		21	21	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 100% <t< td=""><td>Office 25 to 100 ksf</td><td></td><td>sf GFA</td><td>0</td><td>0</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></t<>	Office 25 to 100 ksf		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Linpipe Image: Constraint of the second	Employee			<u> </u>	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 00% 10	Office 100 to 500 ksf		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Diffice 50 0 100% 1	Employee			<u> </u>	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Office >500 ksf		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Data Processing Office sf GFA 0 0 100% <td>Employee</td> <td></td> <td></td> <td><u> </u></td> <td>0</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td> <td>100%</td>	Employee			<u> </u>	0	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 100% 1	Data Processing Office		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Interfere Interfere <t< td=""><td>Employee</td><td></td><td><u></u></td><td>0</td><td><u> </u></td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td><td>100%</td></t<>	Employee		<u></u>	0	<u> </u>	100%	100%	100%	100%	100%	100%	100%	100%
Employee 3 3 100% 1	Medical/Dental Office	1 850	sf GFA	6	6	100%	100%	100%	100%	100%	100%	100%	100%
Bank (Branch) with Drive-In sf GFA 0 0 100% 10	Employee	.,000	<u> 3, 3, 7</u>	3	3	100%	100%	100%	100%	100%	100%	100%	100%
Employee 0 0 0 100%	Bank (Branch) with Drive-In		sf GFA	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Subtotal Customer/Guest Spaces 276 276 276 Subtotal Employee/Resident Spaces 52 53 Subtotal Reserved Spaces 0 0 Total Parking Spaces 328 329	Employee		5, 5, 7	0	0	100%	100%	100%	100%	100%	100%	100%	100%
Subtotal Employee/Resident Spaces 52 53 Subtotal Reserved Spaces 0 0 Total Parking Spaces 328 329	Subtotal Customer/Guest Spaces	I	I	276	276	10070	10070	10070	10070	10070	10070	10070	10070
Subtotal Reserved Spaces 0 0 Total Parking Spaces 328 329	Subtotal Employee/Resident Spaces			52	52								
Total Parking Spaces 328 329	Subtotal Reserved Spaces			0	0	1							
	Total Parking Spaces			328	329	1							

Recommended I	Parking Ra	tios										
Spaces required per unit land use												
Land Llag	Wee	kday	Wee	kend	Unit							
Lanu Ose	Visitor	Employee	Visitor	Employee								
Community Shopping Center (<400 ksf)	2.90	0.70	3.20	0.80	/ksf GLA							
Regional Shopping Center (400 to 600 ksf)	Linear 2.9<	<x<3.2< td=""><td></td><td></td><td>/ksf GLA</td></x<3.2<>			/ksf GLA							
Super Regional Shopping Center (>600 ksf)	3.20	0.80	3.60	0.90	/ksf GLA							
Fine/Casual Dining Restaurant	15.25	2.75	17.00	3.00	/ksf GLA							
Family Restaurant	9.00	1.50	12.75	2.25	/ksf GLA							
Fast Food Restaurant	12.75	2.25	12.00	2.00	/ksf GLA							
Nightclub	15.25	1.25	17.50	1.50	/ksf GLA							
Cineplex	0.19	0.01	0.26	0.01	/seat							
Performing Arts Theater	0.30	0.07	0.33	0.07	/seat							
Arena	0.27	0.03	0.30	0.03	/seat							
Pro Football Stadium	0.30	0.01	0.30	0.01	/seat							
Pro Baseball Stadium	0.31	0.01	0.34	0.01	/seat							
Health Club	6.60	0.40	5.50	0.25	/ksf GLA							
Convention Center	5.50	0.50	5.50	0.50	/ksf GLA							
Hotel-Business	1.00	0.25	0.90	0.18	/room							
Hotel-Leisure	0.90	0.25	1.00	0.18	/room							
Restaurant/Lounge	10.00		10.00		/ksf GLA							
Conference Ctr/Banquet (20 to 50 sq ft/guest room)	30.00		30.00		/ksf GLA							
Convention Space (>50 sq ft/guest room)	20.00		10.00		/ksf GLA							
Residential, Rental, Shared Spaces *	0.15	1.50	0.15	1.50	/unit							
Residential, Owned, Shared Spaces *	0.15	1.7	0.15	1.7	/unit							
Office <25 ksf	0.30	3.5	0.03	0.35	/unit							
Office 25 to 100 ksf	Linear 0.3	<x<0.25< td=""><td></td><td></td><td>/ksf GLA</td></x<0.25<>			/ksf GLA							
Office 100 to 500 ksf	Linear 0.25	5 <x<0.2< td=""><td></td><td></td><td>/ksf GLA</td></x<0.2<>			/ksf GLA							
Office >500 ksf	0.20	2.60	0.02	0.26	/ksf GLA							
Data Processing Office	0.25	5.75	0.03	0.58	/ksf GLA							
Medical/Dental Office	3.00	1.50	3.00	1.50	/ksf GLA							
Bank (Branch) with Drive-In	3.00	1.60	3.00	1.60	/ksf GLA							

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		Maximum Parkir	ng				
		N	/eekdays			Weekend	
	Quantity	Base Ratio	Unit	Max Spaces	Base Ratio	Unit	Max Spaces
Community Shopping Center (<400 ksf)	3,232	3.24	/ksf GLA	10	3.20	/ksf GLA	10
Employee		0.76	/ksf GLA	2	0.80	/ksf GLA	3
Regional Shopping Center (400 to 600 ksf)	0	0.00	/ksf GLA	0	0.00	/ksf GLA	0
Employee		0.00	/ksf GLA	0	0.00	/ksf GLA	0
Super Regional Shopping Center (>600 ksf)	0	3.20	/ksf GLA	0	3.60	/ksf GLA	0
Employee		0.80	/ksf GLA	0	0.90	/ksf GLA	0
Fine/Casual Dining Restaurant	0	15.25	/ksf GLA	0	17.00	/ksf GLA	0
Employee		2.75	/ksf GLA	0	3.00	/ksf GLA	0
Family Restaurant	106	0.43	Seats	46	0.42	Seats	45
Employee		0.07	Seats	7	0.08	Seats	8
Fast Food Restaurant	6,486	12.15	/ksf GLA	79	12.29	/ksf GLA	80
Employee		2.14	/ksf GLA	14	2.00	/ksf GLA	13
Nightclub	0	15.25	/ksf GLA	0	17.50	/ksf GLA	0
Employee		1.25	/ksf GLA	0	1.50	/ksf GLA	0
Cineplex	0	0.19	/seat	0	0.26	/seat	0
Employee		0.01	/seat	0	0.01	/seat	0
Performing Arts Theater	0	0.30	/seat	0	0.33	/seat	0
Employee		0.07	/seat	0	0.07	/seat	0
Arena	0	0.27	/seat	0	0.30	/seat	0
Employee		0.03	/seat	0	0.03	/seat	0
Pro Football Stadium	0	0.30	/seat	0	0.30	/seat	0
Employee	0	0.00	/seat	Ő	0.00	/seat	0
Pro Baseball Stadium	0	0.31	/seat	Ő	0.34	/seat	0
Employee	0	0.01	/ceat	0	0.04	/soat	0
Health Club	0	6.60	/kef GLA	0	5.50	/kef GLA	0
Employee	0	0.00	/kef GLA	0	0.25	/kef GLA	0
Convention Center	0	5.50	/kef GLA	0	5.50	/kef GLA	0
Employee	0	0.50	/kal OLA	0	0.50	/kal OLA	0
Hotel-Business	133	1.00	/roome	133	1.00	Iroome	133
Hotel-Dusiness	0	0.90	/roome	0	1.00	Iroome	0
Restaurant/Lounde	0	10.00	/kef GLA	0	10.00	/kef GLA	0
Conference Ctr/Banquet (20 to 50 sq ft/quest ro	0	30.00	/kal OLA	0	30.00	/kal OLA	0
Convention Space (>50 sq fi/quest room)	0	20.00	/kal OLA	0	10.00	/kal OLA	0
Convention Opace (> 30 3q togdest toolin)	400	20.00	AND CLA	5	10.00	/kai OLA	6
Employee	133	0.04	humit	5	0.04	/rooms	5
Residential, Rental, Shared Spaces	0	0.50	/unit	0	0.50	/unit	0
Reserved	0	1.0	/unit	0	0.45	/unit	0
Guest	0	0.15	/unit	0	0.15	/unit	0
Residential, Owned, Shared Spaces	0	0.70	/unit	0	0.70	/unit	0
Reserved	0	1.0	Junit	0	1.0	/unit	0
Offere (25 line)	0	0.15	/unit	0	0.15		0
Childe <25 ksi	0,013	0.27	/KSI GLA	2	0.27	/KSI GLA	2
Employee	0	3.06	/KSI GLA	21	3.00	/KSI GLA	21
Office 25 to 100 kst	0	3.33	/KST GLA	0	3.33	KST GLA	0
Employee		0.00	/KST GLA	0	0.00	KST GLA	0
Unice Too to 500 Kst	U	0.00	/KST GLA	U	0.00	/KST GLA	U
Employee		0.00	/KST GLA	U	0.00	/KST GLA	U
Unice >500 kst	U	0.20	/KST GLA	0	0.02	/KST GLA	U
Employee	-	2.60	/kst GLA	0	0.26	/kst GLA	0
Data Processing Office	0	0.25	/kst GLA	0	0.03	/kst GLA	0
Employee		5.75	/kst GLA	0	0.58	/kst GLA	0
Medical/Dental Office	1,850	3.35	/ksf GLA	6	3.35	/ksf GLA	6
Employee		1.65	/ksf GLA	3	1.65	/ksf GLA	3
Bank (Branch) with Drive-In	0	3.00	/ksf GLA	0	3.00	/ksf GLA	0
Employee		1.60	/ksf GLA	0	1.60	/ksf GLA	0
Subtotal Customer/Guest Spaces				274			274
Subtotal Employee/Resident Spaces				54			55
Subtotal Reserved Spaces				0			0
Total Parking Spaces				328			329

Combined Monthly Adjustments													
	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Late Dec
Community Shopping Center (<400 ksf)	56%	57%	64%	63%	66%	67%	64%	69%	64%	66%	72%	100%	80%
Employee	80%	80%	80%	80%	80%	80%	80%	80%	80%	80%	90%	100%	90%
Regional Shopping Center (400 to 600 ksf)	56%	57%	64%	63%	66%	67%	64%	69%	64%	66%	72%	100%	80%
Employee	80%	80%	80%	80%	80%	80%	80%	80%	80%	80%	90%	100%	90%
Super Regional Shopping Center (>600 ksf)	56%	57%	64%	63%	66%	67%	64%	69%	64%	66%	72%	100%	80%
Employee	80%	80%	80%	80%	80%	80%	80%	80%	80%	80%	90%	100%	90%
Fine/Casual Dining Restaurant	85%	86%	95%	92%	96%	95%	98%	99%	91%	96%	93%	100%	95%
Employee	95%	95%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
amily Restaurant	85%	86%	95%	92%	96%	95%	98%	99%	91%	96%	93%	100%	95%
Employee	95%	95%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Fast Food Restaurant	85%	86%	95%	92%	96%	95%	98%	99%	91%	96%	93%	100%	95%
Employee	95%	95%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Nightclub	84%	86%	98%	90%	90%	91%	94%	96%	92%	98%	96%	100%	95%
Employee	90%	90%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Cineplex Weekday	27%	21%	20%	19%	27%	41%	55%	40%	15%	15%	25%	23%	100%
Employee	50%	50%	50%	50%	50%	75%	75%	75%	50%	50%	50%	50%	100%
Cineplex Weekend	71%	59%	67%	58%	71%	82%	92%	75%	51%	62%	78%	67%	100%
Employee	80%	80%	80%	80%	80%	100%	100%	90%	80%	80%	80%	80%	100%
Performing Arts Theater	90%	90%	90%	90%	90%	90%	90%	90%	90%	90%	90%	100%	100%
Employee	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Arena	90%	100%	100%	100%	100%	75%	0%	0%	60%	65%	90%	95%	95%
Employee	100%	100%	100%	100%	100%	75%	10%	10%	75%	75%	100%	100%	100%
Pro Football Stadium	0%	0%	0%	0%	0%	0%	0%	67%	0%	0%	0%	100%	100%
Employee	10%	10%	10%	10%	10%	10%	10%	100%	10%	10%	10%	100%	100%
Pro Baseball Stadium	0%	0%	0%	100%	100%	100%	100%	100%	100%	100%	0%	0%	0%
Employee	10%	10%	10%	100%	100%	100%	100%	100%	100%	100%	10%	10%	10%
Logith Club	10.0%	0.5%	06%	70%	669/	669/	65%	709/	90%	969/	959/	00%	0.5%
Employee	100%	100%	05%	90%	759/	75%	75%	90%	00%	05%	05%	100%	100%
Employee Constanting Constant	750/	100%	95%	60%	75%	75%	/5%	0U%	90%	95%	95%	100%	100%
Convention Center	75%	100%	90%	55%	60% 70%	50%	43%	/5%	60%	05%	100%	60%	0%
Employee	007/0	100%	100%	00%	70%	400%	00%	000/	90%	95%	100%	70%	10%
Hotel-Business	7 176	60%	9176	90%	92%	100%	90%	92%	93%	93%	0170	67%	50%
Hotel-Leisure	90%	100%	100%	100%	90%	90%	100%	100%	75%	75%	75%	50%	100%
Restaurani/Lounge	65%	60%	95%	92%	90%	95%	96%	99%	91%	90%	93%	100%	95%
Conterence Ctr/Banquet (20 to 50 sq ft/guest ro	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Convention Space (>50 sq ft/guest room)	75%	100%	90%	55%	60%	50%	45%	75%	80%	85%	100%	60%	0%
Employee	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Residential, Rental, Shared Spaces	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Reserved	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Guest	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Residential, Owned, Shared Spaces	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Reserved	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Guest	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Uttice <25 ksf	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Uttice 25 to 100 ksf	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Office 100 to 500 ksf	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Office >500 ksf	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Data Processing Office	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Medical/Dental Office	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Bank (Branch) with Drive-In	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
Employee	100%	100%	100%	100%	100%	100%	95%	95%	100%	100%	100%	100%	80%
FINAL CALCULATIONS Peak Mo: Parking Demand - Weekday	July 216	227	244	239	246	250	251	249	240	247	234	235	209
Parking Demand - Weekend	215	228	245	241	246	251	252	250	241	247	236	238	210
Month Index	January	February	March	April	May	June	July		September	October	November	December	Late December
Neekday Peak	251	July			,		,						

Weekday Peak Weekend Peak 251 July 252 July

Table Project: Stellar Plaza Courtyard Description: Mixed-Use Development

SHARED PARKING DEMAND SUMMARY

PEAK MONTH: JULY -- PEAK PERIOD: 12 PM, WEEKEND

					Weekday	/				Weeken	d		Weekday			Weekend		
					Non-					Non-			Peak Hr	Peak Mo	Estimated	Peak Hr	Peak Mo	Estimated
	Pro	oject Data	Base	Mode	Captive	Project		Base	Mode	Captive	Project		Adj	Adj	Parking	Adj	Adj	Parking
Land Use	Quantity	Unit	Rate	Adj	Ratio	Rate	Unit	Rate	Adj	Ratio	Rate	Unit	12 PM	July	Demand	12 PM	July	Demand
Community Shopping Center (<400 ksf)	3,232	sf GLA	3.24	1.00	1.00	3.24	/ksf GLA	3.20	1.00	1.00	3.20	/ksf GLA	0.95	0.64	6	0.80	0.64	5
Employee			0.76	1.00	1.00	0.76	/ksf GLA	0.80	1.00	1.00	0.80	/ksf GLA	1.00	0.80	2	1.00	0.80	2
Family Restaurant	106	Seats	0.43	1.00	1.00	0.43	Seats	0.42	1.00	1.00	0.42	Seats	1.00	0.98	45	1.00	0.98	44
Employee			0.07	1.00	1.00	0.07	Seats	0.08	1.00	1.00	0.08	Seats	1.00	1.00	7	1.00	1.00	8
Fast Food Restaurant	6,486	sf GFA	12.15	1.00	1.00	12.15	/ksf GLA	12.29	1.00	1.00	12.29	/ksf GLA	1.00	0.98	77	1.00	0.98	78
Employee			2.14	1.00	1.00	2.14	/ksf GLA	2.00	1.00	1.00	2.00	/ksf GLA	1.00	1.00	14	1.00	1.00	13
Hotel-Business	133	rooms	1.00	1.00	1.00	1.00	/rooms	1.00	1.00	1.00	1.00	/rooms	0.55	0.98	72	0.55	0.98	72
Employee			0.04	1.00	1.00	0.04	/rooms	0.04	1.00	1.00	0.04	/rooms	1.00	1.00	5	1.00	1.00	5
Office <25 ksf	6,813	sf GFA	0.27	1.00	1.00	0.27	/unit	0.27	1.00	1.00	0.27	/unit	0.15	0.95	0	0.90	0.95	2
Medical/Dental Office	1,850	sf GFA	3.35	1.00	1.00	3.35	/ksf GLA	3.35	1.00	1.00	3.35	/ksf GLA	0.30	0.95	2	0.30	0.95	2
Employee			1.65	1.00	1.00	1.65	/ksf GLA	1.65	1.00	1.00	1.65	/ksf GLA	1.00	0.95	3	1.00	0.95	3
													Cus	stomer	202	Cus	stomer	203
													Em	ployee	49	Em	oloyee	49
													Res	served	0	Res	served	0
													Т	otal	251	Т	otal	252

SITE PLAN





Large Firm Resources. Personal Attention. "

Memorandum

To:	Jimmy Asmar, Elite Hospitality, LLC
From:	Michael J. Labadie, PE and Jill Bauer, PE, PTOE
Date:	August 14, 2019
RE:	Courtyard Troy, Traffic Impact Assessment – Amendment

ROWE Professional Services Company has completed our traffic impact assessment amendment related to the proposed Courtyard Troy development to be located on the west side of Crooks Road, north of Corporate Drive/I-75 Ramps, in the City of Troy, Oakland County. The current site plan (included in the materials attached to this report) indicates a hotel with 133 rooms, 8,516 square feet of general office space, 2,176 square feet of medical office space, a 2,072-square-foot coffee shop with drive-through window, a 4,414-square-foot Panera bread shop with drive-through window, and a 4,500 square-foot sit-down restaurant with up to 106 seats. This traffic impact assessment amendment has been completed in response to comments received from the City of Troy engineering consultant OHM, in a memorandum dated August 1, 2019.

Level of Service Analysis

An additional Level of service (LOS) analysis for the AM and PM peak hours were performed for the intersection of Crooks Road and Corporate Drive/I-75 ramps under future (Build) conditions, without the recommended No Build improvement of providing an eastbound left-turn only lane to address the current and forecasted poor operation at the intersection.

According to the most recent edition (6th Edition) of the Highway Capacity Manual, level of service is a qualitative measure describing operational conditions of a traffic stream or intersection. Level of service ranges from A to F, with LOS A being the best. LOS D is generally considered to be acceptable. Table 1 presents the criteria for defining the various levels of service for signalized intersections.

The operational analysis of the Crooks Road and Corporate Drive/I-75 ramps intersection was performed utilizing 2000 HCM intersection methodology, since the HCM 6th Edition methodology does not support analysis of shared lanes at signalized intersections, or more than three through lanes on an approach to an unsignalized intersection.

Although Crooks Road and Corporate Drive are both boulevards, direct left-turns are allowed at their intersection and there is only a single set of signal heads for each approach. (Typically at boulevard intersections, there are "near" and "far" heads with different clearance intervals to ensure vehicles do not become "trapped" in the middle of the intersection.) Due to this, the intersection of Crooks Road and Corporate Drive/I-75 ramps was modeled as a single intersection rather than the typical dual-intersection boulevard configuration specified in Michigan Department of Transportation standards.

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Jimmy Asmar, Elite Hospitality, LLC August 14, 2019 Page 2

Level of Servi	ce Criteria (Signalized Intersection)
Level of Service	Average Stopped Delay/Vehicle (seconds)
А	≤ 10
В	$> 10 \text{ and } \le 20$
С	> 20 and ≤ 35
D	$>$ 35 and \leq 55
E	> 55 and ≤ 80
F	> 80

	Table 1
Level of Servi	ce Criteria (Signalized Intersection)
el of Service	Average Stopped Delay/Vehicle
ver of Service	(seconds)

Note: LOS D is considered acceptable in urban/suburban areas.

The results of the level of service analyses for the intersection of Crooks Road and Corporate Drive/I-75 ramps are summarized in Tables 2 and 3.

Signalized Intersection of Crooks Road and Corporate Drive/I-75 Ramps

The City of Troy's engineering consultant, OHM, has asked that the operational analysis of this intersection not be completed via accepted practice with the timing permit provided by the Road Commission for Oakland County, but rather perform field observations of timings during the peak periods and "average" the observed timings. However, the ongoing construction on I-75 in the vicinity of the project and the disruption to normal traffic patterns precluded this from being performed, and the timing permit information was utilized in the operational analysis.

The results of the existing level of service analysis for the signalized intersection of Crooks Road and Corporate Drive/I-75 ramps indicate that, under existing conditions, all approaches to the intersection operate at an LOS D or better during the AM peak hour, except for the westbound approach which operates at an LOS E. During the PM peak hour, all approaches to the intersection operate at an LOS D or better, except for the northbound and eastbound approaches which operate at an LOS E and LOS F, respectively. The overall intersection operates at an LOS D during the AM peak hour and at an LOS E during the PM peak hour.

With the addition of background traffic and accounting for possible SCATS optimization of the intersection signal timing, all approaches to the intersection would operate at an LOS D or better during both peak periods, except for the eastbound approach during the PM peak hour, which would continue to operate at an LOS F. The overall intersection would continue operate at an LOS D during the AM peak hour and at an LOS E during the PM peak hour.

With inclusion of the recommended background improvement of providing a dedicated eastbound left-turn lane with 200 feet of storage and accounting for possible SCATS optimization of the intersection signal timing, all approaches to the intersection would operate at an LOS D or better during both the AM and PM peak hours. The overall intersection would operate at an LOS D during both peak periods.

Including site generated traffic and accounting for possible SCATS optimization of the intersection timing, but not including the recommended background improvement of providing a dedicated eastbound left-turn lane with 200 feet of storage, all approaches to the intersection would operate at an LOS D during the AM peak hour. During the PM peak hour, the westbound approach would operate at an LOS D, and the northbound, southbound, and eastbound approaches would operate at an LOS E. The overall intersection would operate at an LOS D during the AM peak hour and at an LOS E during the PM peak hour.

Jimmy Asmar, Elite Hospitality, LLC August 14, 2019 Page 3

The intersection would continue to operate in a manner like the background with improvements scenario with the addition of site generated traffic.

The operational results for the intersection of Crooks Road and Corporate Drive/I-75 ramps are presented in Tables 2 and 3.

Level of Servi	ce Analysis f	for Crooks Road	and Corporate	Drive/I-75 Ramj	DS
			2020	2020	
Approach	2018	2020	Background	Total Future	2020
	Existing ¹	Background ²	Improved ³	w/o Imp.	Total Future ⁵
Northbound Crooks Road	D (39.9)	D (51.1)	D (51.1)	D (53.7)	D (53.7)
Southbound Crooks Road	C (31.8)	D (36.0)	C (34.7)	D (39.6)	D (37.5)
Eastbound Corporate Drive	D (53.3)	D (53.1)	D (52.7)	D (54.8)	D (53.0)
Westbound I-75 Off-Ramp	E (70.9)	D (47.5)	D (47.4)	D (50.5)	D (50.4)
Overall Intersection	D (46.7)	D (42.8)	D (42.1)	D (46.1)	D (44.9)

Table 2 AM Peak Hour Level of Service Analysis for Crooks Road and Corporate Drive/I-75 Ramps

(XX.X)Average seconds of delay per vehicle.

1. Operational results represent existing splits provided in timing permit.

2. Operational results represent SCATS optimization.

3. Includes background (No Build) construction of an eastbound left-turn lane with 200 feet of storage.

4. Does not include recommended background (no Build) improvements to address existing and background poor operation of intersection during PM peak hour.

5. Total future condition assumes background improvements.

Level of Servi	ce Analysis f	for Crooks Road	and Corporate	Drive/I-75 Ramp	DS
			2020	2020	
Approach	2018	2020	Background	Total Future	2020
	Existing ¹	Background ²	Improved ³	w/o Imp.	Total Future⁴
Northbound Crooks Road	E (61.9)	D (51.0)	D (51.0)	E (74.9)	D (54.4)
Southbound Crooks Road	C (28.4)	D (52.1)	D (42.1)	E (74.7)	D (49.0)
Eastbound Corporate Drive	F (177.4)	F (82.9)	D (52.9)	E (70.1)	D (54.4)
Westbound I-75 Off-Ramp	D (51.5)	D (52.5)	D (52.0)	D (54.0)	D (54.2)
Overall Intersection	E (70.3)	E (56.5)	D (49.6)	E (69.5)	D (53.2)

Table 3 PM Peak Hour Level of Service Analysis for Crooks Road and Cornerate Drive/L-75 Ramps

(XX.X) Average seconds of delay per vehicle.

1. Operational results represent existing splits provided in timing permit.

2. Operational results represent SCATS optimization.

3. Includes background (No Build) construction of an eastbound left-turn lane with 200 feet of storage.

4. Does not include recommended background (no Build) improvements to address existing and background poor operation of intersection during PM peak hour.

5. Total future condition assumes background improvements.

Conclusions and Recommendations

The proposed Courtyard Troy consists of a hotel with 133 rooms, 8,516 square feet of general office space, 2,176 square feet of medical office space, a 2,072 square-foot coffee shop with drive-through window, a 4,414-square-foot Panera bread shop with drive-through window, and a 4,500-square-foot sit-down

Jimmy Asmar, Elite Hospitality, LLC August 14, 2019 Page 4

restaurant with up to 106 seats. The proposed development will have access to southbound Crooks Road, westbound Corporate Drive, and New King Drive via a single driveway on each roadway.

The proposed mixed-use development is forecast to generate 289 new trips during the AM peak hour (159 inbound and 130 outbound from the site) and 158 new trips during the PM peak hour (79 inbound and 79 outbound from the site).

A revised operational analysis was performed for the total future conditions for the intersection of Crooks Road and Corporate Drive/I-75 ramps.

To address impacts from existing and background traffic volumes, it is recommended that an eastbound left-turn lane with 200 feet of storage be provided at the intersection of Corporate Drive with Crooks Road.

Attachments

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LEVEL OF SERVICE

OUTPUT REPORTS

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		-î†	1	٢	4î h	1		***	1	ሻሻ	***	1
Traffic Volume (vph)	79	123	40	663	543	571	0	648	216	569	1751	407
Future Volume (vph)	79	123	40	663	543	571	0	648	216	569	1751	407
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.97	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.98	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3654	1667	1695	3244	1517		5353	1667	3614	5353	1667
Flt Permitted		0.98	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3654	1667	1695	3244	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	104	162	53	721	590	621	0	682	227	599	1843	428
RTOR Reduction (vph)	0	0	47	0	14	74	0	0	54	0	0	206
Lane Group Flow (vph)	0	266	6	497	980	367	0	682	173	599	1843	222
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		12.9	12.9	37.9	37.9	62.1		17.2	55.1	24.2	48.2	48.2
Effective Green, g (s)		12.9	12.9	37.9	37.9	62.1		17.2	55.1	24.2	48.2	48.2
Actuated g/C Ratio		0.11	0.11	0.32	0.32	0.52		0.14	0.46	0.20	0.40	0.40
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		392	179	535	1024	785		767	765	728	2150	669
v/s Ratio Prot		c0.07		0.29	c0.30	0.09		0.13	0.07	0.17	c0.34	
v/s Ratio Perm			0.00			0.15			0.03			0.13
v/c Ratio		0.68	0.03	0.93	0.96	0.47		0.89	0.23	0.82	0.86	0.33
Uniform Delay, d1		51.6	48.0	39.7	40.3	18.4		50.5	19.6	45.8	32.8	24.8
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		4.6	0.1	22.5	18.5	0.4		14.6	0.2	10.2	4.7	1.3
Delay (s)		56.2	48.0	62.3	58.7	18.9		65.0	19.7	56.0	37.4	26.1
Level of Service		Е	D	Е	E	В		E	В	Е	D	С
Approach Delay (s)		54.8			50.5			53.7			39.6	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay			46.1	Н	CM 2000) Level of	Service		D			
HCM 2000 Volume to Capacit	y ratio		0.94									
Actuated Cycle Length (s)			120.0	S	um of los	st time (s)			27.8			
Intersection Capacity Utilizatio	n		82.7%	IC	CU Level	of Service)		E			
Analysis Period (min)			15									

c Critical Lane Group

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		-۠	1	7	et îr	1		***	1	ሻሻ	***	1
Traffic Volume (vph)	243	518	82	422	141	585	0	1559	514	276	751	109
Future Volume (vph)	243	518	82	422	141	585	0	1559	514	276	751	109
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Lane Util. Factor		0.95	1.00	0.91	0.86	0.91		0.91	1.00	0.97	0.91	1.00
Frt		1.00	0.85	1.00	0.92	0.85		1.00	0.85	1.00	1.00	0.85
Flt Protected		0.98	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		3667	1667	1695	3073	1517		5353	1667	3614	5353	1667
Flt Permitted		0.98	1.00	0.95	0.99	1.00		1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		3667	1667	1695	3073	1517		5353	1667	3614	5353	1667
Peak-hour factor, PHF	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	261	557	88	444	148	616	0	1641	541	291	791	115
RTOR Reduction (vph)	0	0	68	0	154	81	0	0	46	0	0	69
Lane Group Flow (vph)	0	818	20	311	435	227	0	1641	495	291	791	46
Turn Type	Split	NA	Perm	Split	NA	pm+ov		NA	pm+ov	Prot	NA	Perm
Protected Phases	4	4		8	8	1		2	8	1	6	
Permitted Phases			4			8			2			6
Actuated Green, G (s)		29.9	29.9	27.0	27.0	35.1		37.2	64.2	8.1	52.1	52.1
Effective Green, g (s)		29.9	29.9	27.0	27.0	35.1		37.2	64.2	8.1	52.1	52.1
Actuated g/C Ratio		0.23	0.23	0.21	0.21	0.27		0.29	0.49	0.06	0.40	0.40
Clearance Time (s)		7.1	7.1	7.1	7.1	6.8		6.8	7.1	6.8	6.8	6.8
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	4.0	4.0
Lane Grp Cap (vph)		843	383	352	638	409		1531	823	225	2145	668
v/s Ratio Prot		c0.22		c0.18	0.14	0.03		c0.31	0.12	c0.08	0.15	
v/s Ratio Perm			0.01			0.12			0.17			0.03
v/c Ratio		0.97	0.05	0.88	0.68	0.55		1.07	0.60	1.29	0.37	0.07
Uniform Delay, d1		49.6	39.0	50.0	47.5	40.7		46.4	23.7	61.0	27.4	24.0
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		23.9	0.1	22.1	3.0	1.6		44.9	1.2	160.9	0.5	0.2
Delay (s)		73.5	39.1	72.0	50.6	42.4		91.3	24.9	221.9	27.9	24.2
Level of Service		E	D	E	D	D		F	С	F	С	С
Approach Delay (s)		70.1			54.0			74.9			74.7	
Approach LOS		E			D			E			E	
Intersection Summary												
HCM 2000 Control Delay			69.5	H	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capacit	y ratio		1.01									
Actuated Cycle Length (s)			130.0	Si	um of los	st time (s)			27.8			
Intersection Capacity Utilization	on		93.8%	IC	U Level	of Service			F			
Analysis Period (min)			15									

c Critical Lane Group



memorandum

Date: August 28, 2019

To: Bill Huotari, PE

cc: Sara Merrill, PE, PTOE, Stephen Dearing, PE, PTOE

From: Stephan Maxe, PE

Re: Troy Plaza Courtyard Traffic Impact Study Review

I have reviewed the traffic impact study amendment for the proposed Courtyard hotel and various other developments, located in the Troy Plaza site in the northwest quadrant of the Crooks Road and Corporate Drive intersection. The proposed development includes a Courtyard Hotel, Retail, Office Building, Fast-Food Restaurant and High Turnover Restaurant. The study was prepared by Rowe Professional Services and is dated August 14th, 2019.

OHM accepts the findings of the traffic impact study. The study shows that there are lengthy delays at the Crooks Road/I-75 off-ramp/Corporate Drive intersection and the development will significantly add to the delays at this intersection. The suggested eastbound left turn lane with 200' of storage on Corporate Drive will improve intersection operations to an acceptable level of service.

OHM has no additional comments.



memorandum

Date: October 1, 2019

To: Bill Huotari, PE

CC: Sara Merrill, PE, PTOE

From: Stephan Maxe, PE

Re: Plaza Courtyard Preliminary Site Plan – 3rd Review

I have reviewed the revised preliminary site plan for the proposed Courtyard hotel, located in the Troy Plaza site in the northwest quadrant of the Crooks Road and Corporate Drive intersection. The proposed development includes a 5-story, 133 room hotel, with parking on site. The plans were prepared by Stellar Development, LLC., and dated September 24th, 2019.

At this time, OHM <u>recommends approval</u> of the preliminary site plan.

OHM offers the following comments:

- 1. The preliminary site plan comments previously noted have been adequately addressed.
- 2. The shared parking study and updated traffic impact study have been reviewed and accepted.

STATE OF MICHIGAN COUNTY OF OAKLAND CITY OF TROY

CONDITIONAL REZONING AGREEMENT

This Development Agreement ("Agreement"), dated _____, 2012 is entered into by and between Tinelle Properties, LLC, a Michigan Limited Liability Company, whose address is 29850 Northwestern Hwy., Suite 200, Southfield, MI 48034 ("Developer"), and the City of Troy, a Michigan municipal corporation, having its principal offices at 500 W. Big Beaver Road, Troy, Michigan 48084 ("City").

RECITALS:

A. Developer is the owner of certain real property located in the City of Troy, Oakland County, Michigan, containing approximately 6.157 acres, as more particularly described on Exhibit A attached hereto (the "Property").

B. Developer has applied for Conditional Rezoning pursuant to Troy's Zoning Ordinance, Chapter 39, Section 16.04 from Planned Unit Development ("PUD") district to one section, designated as Phase I in the attached Exhibit B, as a Community Business ("CB") district and one section, designated Phase II in the attached Exhibit B, as a Office Mixed Use ("OM"),. (See attached Exhibit B for details of split.) That Ordinance requires that an applicant for conditional rezoning prepare a Conditional Rezoning Agreement ("Agreement") which requires that specific provisions be included in the Agreement.

C. As part of approval of this Agreement, which will result in a rezoning of the Property from PUD district to an CB and a OM district, Developer has offered and agrees to make the improvements, proceed with the project and comply with any document submission dates and/or project completion dates, as described in this Agreement and any incorporated documents. Any conditions, representations or promises included in the Agreement have been voluntarily offered by the Developer to induce the City to rezone the land to the proposed classifications. The Developer and the City agree that the rezoning and the terms of this Agreement provides for: (i) the promotion of the public health, safety and welfare; (ii) compatibility with the Master Plan; (iii) compliance with all terms and conditions of the zoning district to which the land is to

be rezoned, except as otherwise allowed in the Agreement; (iv) the accommodation of service and facility loads for public services and facilities affected by the proposed development; (v) compatibility with adjacent uses of land; and (vi) other legitimate objectives authorized under the Michigan Municipal Planning Act, MCL 125.3101, et. seq. and Chapter 39, Section 16.04 of the City of Troy Zoning Ordinance. The burden of the conditions on the Developer is roughly proportionate to the burdens being created by the development, and to the benefit which will accrue to Property as a result of the requirements represented in the project and/or development.

D. For the purpose of confirming the rights, obligations and restrictions in connection with the improvements and development to be undertaken on the Property, the parties have entered into this Agreement. The rezoning of the Property to CB and OM district shall become effective as set out in Section 3.1 of this Agreement. Once this Agreement is approved by the Troy City Council, it shall be binding upon the City, the Developer, the owner, the owner of units within the Development, any owners associations, and their agents, successors and assigns. The City's Zoning District Map shall be updated to reflect the amendment to the Zoning Ordinance and the District Map within a reasonable period of time after the effective date of the rezoning.

NOW, THEREFORE, the Developer and the City, for the good and valuable consideration outlined in this Agreement, the receipt and sufficiency of which are hereby acknowledged, agree as follows:

ARTICLE I

GENERAL TERMS

1.1 This Agreement, including all incorporated documents, shall run with the land. The land is the Property described in the attached Exhibit A. Reference to "Developer" in this Agreement, and/or any incorporated documents, shall include the owner of the Property, Developer's agents, successors and assigns. It is the intent of Developer and the City to put all future owners of the Property, all future leaseholders and/or all parties in interest on notice of the rights, obligations and restrictions contained herein by recording this Agreement, including the site plan(s), if any, and any incorporated documents with the Oakland County Register of Deeds. If the project or development plan includes any type of condominium element, any termination of any condominium community or association shall not nullify or void this Agreement. As part of this Agreement and pursuant to State statute, if a Master Deed is not controlling for all or any portion of a condominium project, the terms and conditions of this Agreement shall be considered "Deed Restrictions" for any successors or assigns of the Property.

1.2 The Property shall be developed and improved in accordance with the following which shall be referred to herein as the "Conditional Rezoning Agreement Documents":

- A. Chapter 39, Section 16.04 of the City's Zoning Ordinance, and amendments, if any.
- B. This Conditional Rezoning Agreement.
- C. Chapter 39, Section 4.13, which set out the requirements for development in a CB district classification, and Chapter 39, Section 4.17 which sets out the requirements for development in a OM district classification, unless those requirements are amended by the Conditional Rezoning Agreement.
- D. Any other documents that are incorporated into this Agreement as Exhibit C.
- E. The site plan, as approved by the Planning Commission, attached as Exhibit D, is incorporated herein by reference.

1.3 The Ordinance amendment granting the conditional rezoning reclassifies the zoning of the Property to CB and OM districts and constitutes the land use authorization for the Property, and all use improvement of the Property shall be in substantial conformity with the provisions of the Zoning Ordinance applicable to the zoning district and this Agreement.

ARTICLE II

CONDITIONS FOR REZONING

2.1 In consideration for the City's rezoning of the Property from its current classification of PUD district to a future classifications of CB and OM districts, the Developer agrees to be bound by the following conditions:

- A. The development of the first two buildings of Phase I, (CB district), within six (6) months of obtaining building permits; including the parking, drive-aisles and building pad for corner building.
- B. The Future Restaurant Building Pad of Phase I will not be marketed to a bank, a chain drug store or any other vehicular intensive use; but will be considered for all uses permitted in a CB district.

- C. The details of the Future Restaurant Building Pad of Phase I shall be subject to planning approval and be modified by administrative approval with-in their jurisdiction;
- D. The OM parcel (designated as Phase II) will be limited to and developed as either an office building or a hotel as presented in the approval package;
- E. Prior to the development of Phase II, Developer shall provide the City with a traffic and parking study analysis.
- F. The 25'0" high standard light poles will be turned off and/or reduced after 11:00 p.m. until sunrise. As to Phase I, no additional lighting is proposed to be building mounted to illuminate the site.

2.2 Developer represents and confirms that the Property shall not be used or developed in a manner that is inconsistent with conditions placed on rezoning as set out in this Agreement.

2.3 Developer shall be subject to the expiration provisions of Section 16.04. E.of the Zoning Ordinance and Section 5.2 of this Agreement.

ARTICLE III

<u>REZONING</u>

3.1 Directly after approval of this Agreement, City Council shall pass a Resolution rezoning the Property from PUD district zoning classification to an CB and a OM district classifications. That Resolution shall also state that the Zoning Map shall be amended to reflect a new zoning classification. The Planning Director shall take necessary action to amend the Zoning Map to the new classification along with a relevant designation that will provide reasonable notice of the Conditional Rezoning Agreement. The Conditional Rezoning Approval and the amendment to Zoning Map shall not become effective until the Conditional Rezoning Agreement is recorded with the Oakland County Register of Deeds and a certified copy of the Agreement is filed with the City Clerk.

ARTICLE IV

DEVELOPER'S RIGHTS, OBLIGATIONS AND PROPERTY RESTRICTIONS

4.1 Developer shall have the right to develop the Property in accordance with the Conditional Rezoning Agreement Documents and shall receive Preliminary and Final approval in accordance with the City's Zoning Ordinance and this Agreement. If development and/or actions are undertaken on or with respect to the Property in violation of the Conditional Rezoning Agreement, such development and/or actions shall constitute a violation of the City of Troy Code of Ordinances and deemed a nuisance per se. In such cases the City may issue a stop work order relative to the property into compliance with the Conditional Rezoning Agreement, the City may withhold, or, following notice and an opportunity to be heard revoke permits and certificates, in addition to or in lieu of such other lawful action to achieve compliance.

4.2 All development, use, and improvement of the Property shall be subject to and in accordance with this Agreement, the Conditional Rezoning Agreement Documents, all applicable City Ordinances, and shall also be subject to and in accordance with all other approvals and permits required under applicable City Ordinances and State law.

4.3 Developer shall comply with the City Code of Ordinances, make any necessary application for permits and obtain any necessary permits for the development of the property including signage.

ARTICLE V

THE CITY'S RIGHTS AND OBLIGATIONS

5.1 The action of the City in entering into this Conditional Rezoning Agreement is based upon the understanding that the intent and spirit of the police power objectives of the City relative to the Property are embodied in the Conditional Rezoning Agreement Documents and those powers are assured based upon the development and/or undertakings on the Property. The City is thus achieving its police power objective and has not, by this Agreement, bargained away or otherwise compromised any of its police power objectives.

5.2 Conditional Rezoning Approval shall expire following a period of two (2) years from the effective date of the rezoning as set out above unless progress has been diligently pursued and substantial completion has occurred in accordance with permits issued by the City. The City shall have the sole discretion to determine if progress has been diligently pursued by the Developer of the Property. The City, through its employees and agents, shall at all times be allowed to enter onto the Property to determine if the progress of the development. 5.3 The City may initiate legal action for the enforcement of any of the provisions, requirements, and obligations set forth in the Conditional Rezoning Agreement. In the event the City obtains any relief as a result of such litigation, Developer shall pay all court costs and attorney fees incurred by the City in connection with such suit;

5.4 If the Developer is developing the Property in non-compliance with the Conditional Rezoning Agreement, the City may issue a stop work order as to any or all aspects of the Development, may deny the issuance of any requested building permit or certificate of occupancy within any part or all of the Development regardless of whether the Developer is the named applicant for such permit or certificate of occupancy, and may suspend further inspections of any or all aspects of the Development.

5.5 To the extent the Conditional Rezoning Agreement Documents deviate from the City of Troy Development Standards, Zoning Ordinance or other City ordinances, or any amendments thereto, the Conditional Rezoning Agreement Documents shall control. All improvements constructed in accordance with the Conditional Rezoning Agreement Documents shall be deemed to be conforming under the Zoning Ordinance and in compliance with all ordinances of the City.

ARTICLE VI

MISCELLANEOUS PROVISIONS

6.1 This Agreement may not be modified, replaced, amended or terminated except as provided for in this Agreement.

6.2 This Agreement shall be governed by and construed in accordance with the laws of the State of Michigan.

6.3 If there is a conflict between the terms of any of the Conditional Rezoning Agreement Documents, such documents shall control in the following order: (a) Chapter 39, Section 16.04 of the City's Zoning Ordinance, and amendments, if any (b) this Agreement and any Conditional Rezoning Agreement Documents. Where there is a question with regard to applicable regulations for a particular aspect of the Development, or with regard to clarification, interpretation, or definition of terms or regulations, and there are no apparent express provisions of the Conditional Rezoning Agreement Documents which apply, the City in the reasonable exercise of its discretion, shall determine the regulations of the City's Zoning Ordinance, as that Ordinance may have been amended, or other Ordinances which shall be applicable provided such determination is not inconsistent with the nature and intent of the Conditional Rezoning Agreement Documents.

After consulting with their respective attorneys, Developer and City 6.4 confirm that this Agreement is authorized by and consistent will all applicable state and federal law and the United States and Michigan Constitutions, that the terms of this Agreement are reasonable, that they shall be estopped from taking a contrary position in the future, and that each shall be entitled to injunctive relief to prohibit any actions by the other inconsistent with the terms of this Agreement. Developer and the City fully accept and agree to the final terms, conditions, requirements and obligations of the Agreement and all Conditional Rezoning Agreement Documents, and shall not be permitted in the future to claim that the effect of the Agreement and the Conditional Rezoning Agreement Documents result in an unreasonable limitation upon uses of all or a portion of the Property, or claim that enforcement of any of the Agreement and the Conditional Rezoning Agreement Documents causes an inverse condemnation or taking of all or a portion of the Property. Furthermore, it is agreed that the improvements and undertakings set forth in the Agreement and the Conditional Rezoning Agreement Documents are roughly proportional to the burden being created by the development, and to the benefit which will accrue to the Property as a result of the requirements represented by the development.

6.5 This Agreement may be executed in multiple counterparts, each of which shall be deemed an original and all of which shall constitute one agreement. The signature of any party to any counterpart shall be deemed to be a signature to, and may be appended to, any other counterpart.

6.6 This Agreement shall be binding on, and shall inure to the benefit of the parties and their respective successors and assigns.

6.7 Developer acknowledges that if Troy City Council does not pass a resolution authorizing the rezoning of the property and/or approving the Conditional Rezoning Agreement, then the Troy Planning Commission's approval of the site plan and/or special use or conditional rezoning becomes null and void.

THIS AGREEMENT was executed by the respective parties on the date specified with the notarization of their name.

"Developer" Tinelle Properties, LLC By: mer Asmar

Its: Managing Member

STATE OF MICHIGAN)) ss. COUNTY OF OAKLAND)

The foregoing instrument was acknowledged before by Amer Asmar this day of September, 2012

Patrick J. Fisher Notary Public, State of Michigan County of Oakland My Commission Exp. April 21, 2018 Acting in the County of

_____, Notary Public Oakland County, Michigan My Commission Expires: Apr, 1212018

CITY OF TROY, a Michigan municipal corporation

By: Janice Daniels Its: Mayor

By: Aileen Bittner Its: City Clerk

STATE OF MICHIGAN)) ss. COUNTY OF OAKLAND)

The foregoing instrument was acknowledged before me this _____ day of 20____

, Notary Public Oakland County, Michigan My Commission Expires:

PREPARED BY:

Ira S. Auslander_ 26261 Evergreen Suite 130 Southfield, Michigan, 48076

WHEN RECORDED RETURN TO:

City Clerk City of Troy 500 W. Big Beaver Road Troy, Michigan 48084

<u>EXHIBIT A</u>

LEGAL DESCRIPTION OF THE PROPERTY



Exhibit A

LEGAL DESCRIPTION - PARCELS 1 AND 2

LEGAL DESCRIPTION - PARCEL 1

A PARCEL OF LAND LOCATED IN THE NORTHEAST 1/4 OF SECTION 8, TOWN 2 NORTH, RANGE 11 EAST, CITY OF TROY, OAKLAND COUNTY, MICHIGAN, DESCRIBED AS: BEGINNING AT A POINT DISTANT N.02°30'26"W., 225.27' FEET ALONG THE EAST LINE OF SECTION 8 AND S.87°29'34"W., 90.00 FEET TO A POINT ON THE WEST RIGHT-OF-WAY LINE OF CROOKS ROAD (150' WIDE) FROM THE EAST 1/4 CORNER OF SAID SECTION 8; THENCE S.87°29'34"W., 248.90 FEET; THENCE 1.10 FEET ALONG THE ARC OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 352.68 FEET, CHORD BEARING S.87°34'11"W., 1.10 FEET; THENCE N.02°30'26"W., 490.18 FEET; THENCE N.87°29'34"E., 250.00 FEET; THENCE S.02°30'26"E., 490.18 FEET ALONG THE WEST RIGHT-OF-WAY LINE OF SAID CROOKS ROAD TO THE POINT OF BEGINNING.

CONTAINING 122,545.00 SQUARE FEET OR 2.813 ACRES.

LEGAL DESCRIPTION - PARCEL 2

A PARCEL OF LAND LOCATED IN THE NORTHEAST 1/4 OF SECTION 8, TOWN 2 NORTH, RANGE 11 EAST, CITY OF TROY, OAKLAND COUNTY, MICHIGAN, DESCRIBED AS: COMMENCING AT THE EAST 1/4 CORNER OF SAID SECTION 8; THENCE N.02*30'26"W., 225.27' FEET ALONG THE EAST LINE OF SECTION 8; THENCE S.87*29'34"W., 338.90 FEET; THENCE 1.10 FEET ALONG THE ARC OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 352.68 FEET, CHORD BEARING S.87*34'11"W., 1.10 FEET TO THE POINT OF BEGINNING; THENCE 552.95 FEET ALONG THE ARC OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 352.68 FEET, CHORD BEARING N.47*25'29"W., 498.03 FEET; THENCE N.02*30'26"W., 137.51 FEET; THENCE N.87*29'34"E., 351.65 FEET; THENCE S.02*30'26"E., 490.18 FEET TO THE POINT OF BEGINNING.

CONTAINING 145,681.45 SQUARE FEET OR 3.344 ACRES.



PREPARED FOR: TINELLE PROPERTIES, LLC

REVISED	DRAWN	JOB NO.	SHEET
10-01-12	MRC	C380-02	2 of 3



EXHIBIT B

DESIGNATIONS OF PHASE I and PHASE II



DECIDUO	US TREE PL	ANT LIST:			
QUANTITY	KEY SYMBOL	COMMON NAME	SCIENTIFIC NAME	SIZE	SPEC
2	AS2.5	Sugar Maple	Acer saccharum 'Green Mountain'	2.5" Cal.	B&B
2	QR2.5	Red Oak	Quercus rubra	2.5" Cal.	B&B
4	TOTAL DEC.				
EVERGRE	EN TREE PL	ANT LIST:			
QUANTITY	KEY SYMBOL	COMMON NAME	SCIENTIFIC NAME	SIZE	SPEC
7	AC8	Concolor Fir	Abies concolor	8' Ht.	B&B
7	PA8	Norway Spruce	Picea abies	8' Ht.	B&B
7	PC8	Columnar Norway Spruce	Picea abies 'Cupressina'	8' Ht.	B&B
7	PG8	Black Hills Spruce	Picea glauca 'Densata'	8' Ht.	B&B
5	PO8	Serbian Spruce	Picea omorika	8' Ht.	B&B
6	PB8	Lacebark Pine	Pinus bungeana	8' Ht.	B&B
10	PN8	Austrian Pine	Pinus nigra	8' Ht.	B&B
7	PS8	Eastern White pine	Pinus strobus	8' Ht.	B&B
56	TOTAL EVG.				



WOODLAND TR	EES			
WOODLAND TR	EES REMOVE	<u>D:</u> 1	(REPLAC	CE AT
6''	DBH x 0.5 =		3''	REP
WOODLAND TR	EES SAVED:	21	(CREDIT	OF 2
197	DBH x 2 =		394''	CRE
	3 -	394	=	
" <u>" DB</u> ł	H REQUIRED I	FOR WOOD	LAND REI	PLAC
LANDMARK TRI	EES			
		<u>۰</u>		

LANDMARK TREES REMOVED:			1	(REPLAC	EAI
22'' [DBH x 1 =			22''	REP
LANDMARK TREE	ES SAVED		0	(CREDIT	OF 2
"[DBH x 2 =				CRE
	22	-	0	=	





ALTERNATE #1

THIRD FLOOR ALTERNATE Scale: 1/4" = 1'-0"



THIRD FLOOR PLAN Scale: 1/4" = 1'-0"

SECOND FLOOR Scale: 1/4" = 1'-0"



Scale: 1/4" = 1'-0"



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Client:

GFA Development

Project Title: Square Lake

Troy, MI

sheet Title: Unit Plans

Project Number: 16327 Drawn By: JB Checked By: AJM Approved By: AJM Date:1-13-17

Owner Review Owner Review For Site Plan Approval For Site Plan Approval 2017-01-13 2017-01-26 2018-03-27 2019-10-02

Sheet Number:



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BACK Scale: 1/4" = 1'-0"

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		L	L	_



SIDE Scale: 1/4" = 1'-0"



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Client:

GFA Development

Project Title: Square Lake

Troy, MI

Sheet Title: SIDE ELEVATION

Project Number: 16327 Drawn By: JB Checked By: AJM Approved By: AJM Date:1.13.17

Issued: Owner Review Owner Review For Site Plan Approval For Site Plan Approval

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2017-01-13 2017-01-26 2018-03-27 2019-10-02

Sheet Number:



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REAR Scale: 1/4" = 1'-0"







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Client:

GFA Development

Project Title: The Landings

Troy, MI

Sheet Title: SIDE ELEVATION

Project Number: 16327 Drawn By: JB Checked By: AJM Approved By: AJM Date:1.13.17

Issued: Owner Review Owner Review For Site Plan Approval For Site Plan Approval For Site Plan Approval

2017-01-13 2017-01-26 2018-03-27 2019-10-02

2019-11-7

Sheet Number:



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