

500 West Big Beaver Troy, MI 48084 troymi.gov

248.524.3364 planning@troymi.gov

PLANNING COMMISSION MEETING AGENDA REGULAR MEETING

Carlton Faison, Chairman, Tom Krent, Vice Chairman Ollie Apahidean, Karen Crusse, Barbara Fowler, Michael W. Hutson, David Lambert, Sadek Rahman and John J. Tagle

December 10, 2019

7:00 P.M.

Council Chambers

- 1. ROLL CALL
- 2. <u>APPROVAL OF AGENDA</u>
- 3. <u>APPROVAL OF MINUTES</u> November 26, 2019
- 4. <u>PUBLIC COMMENT</u> For Items Not on the Agenda

SPECIAL USE

- <u>PUBLIC HEARING SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File Number SU JPLN2019-0033)</u> Proposed Bethesda Romanian Pentecostal Church Addition, North of Long Lake, East of John R, South of Tucker (2075 E Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District
- <u>PUBLIC HEARING SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (SP JPLN2019-0037)</u> Proposed Bostick 801, LLC Redevelopment, East side of Crooks, south of Big Beaver (801 W. Big Beaver), Section 28, Currently Zoned BB (Big Beaver Road) District

CONDITIONAL REZONING

 <u>PUBLIC HEARING – CONDITIONAL REZONING (CR JPLN2019-003)</u> – Proposed Livernois Court, West of Livernois, North of Big Beaver, (88-20-22-301-008 and 88-20-22-301-009), Section 22, From R-1C (One Family Residential), to BB (Big Beaver Road) District.

PUBLIC HEARINGS

- 8. <u>PUBLIC HEARING ZONING ORDINANCE TEXT AMENDMENT (File Number ZOTA 254)</u> Cluster Square Footage
- 9. <u>PUBLIC HEARING ZONING ORDINANCE TEXT AMENDMENT (File Number ZOTA 255)</u> Transitions in NN (Neighborhood Node) Zoning District

OTHER BUSINESS

- 10. PLANNING COMMISSION MEETING SCHEDULE FOR 2020
- 11. <u>PUBLIC COMMENT</u> Items on Current Agenda
- 12. PLANNING COMMISSION COMMENT

13. ADJOURN

NOTICE: People with disabilities needing accommodations for effective participation in this meeting should contact the City Clerk by e-mail at <u>clerk@troymi.gov</u> or by calling (248) 524-3317 at least two working days in advance of the meeting. An attempt will be made to make reasonable accommodations.

Vice Chair Krent called the Regular meeting of the Troy City Planning Commission to order at 7:00 p.m. on November 26, 2019 in the Council Chamber of the Troy City Hall.

1. ROLL CALL

<u>Present:</u> Ollie Apahidean Michael W. Hutson Tom Krent David Lambert Sadek Rahman John J. Tagle

<u>Absent:</u> Karen Crusse Carlton M. Faison Barbara Fowler

Also Present:

R. Brent Savidant, Community Development Director Julie Quinlan Dufrane, Assistant City Attorney Jackie Ferencz, Planning Department Administrative Assistant Kathy L. Czarnecki, Recording Secretary

2. <u>APPROVAL OF AGENDA</u>

Resolution # PC-2019-11-079

Moved by: Tagle Support by: Apahidean

RESOLVED, To approve the Agenda as prepared.

Yes: All present (6) Absent: Crusse, Faison, Fowler

MOTION CARRIED

3. APPROVAL OF MINUTES

Resolution # PC-2019-11-080

Moved by: Lambert Support by: Hutson

RESOLVED, To approve the minutes of the November 12, 2019 Regular meeting as submitted.

Yes: All present (6) Absent: Crusse, Faison, Fowler

MOTION CARRIED

4. <u>PUBLIC COMMENT</u> – Items not on the Agenda

There was no one present who wished to speak.

SPECIAL USE APPROVAL AND PRELIMINARY SITE PLAN REVIEWS

 <u>PUBLIC HEARING - SPECIAL USE APPROVAL AND PRELIMINARY SITE PLAN</u> <u>REVIEW (File Number SU JPLN2019-0033)</u> – Proposed Bethesda Romanian Pentecostal Church Addition, North of Long Lake, East of John R, South of Tucker (2075 E Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District

Mr. Savidant asked Commissioner Apahidean to discuss his relationship with the church, noting he recused himself in previous meetings that the Board considered an application from the church.

Mr. Apahidean said he lives 800 feet from the church. He said he has no pecuniary interest in the application and feels he could render a decision on facts and findings presented. He asked the Board's preference if he should recuse himself from discussion and action on the item.

Vice Chair Krent went around the table asking each member his preference. It was the consensus of the Board that Mr. Apahidean could remain.

Mr. Savidant announced a Special Use Approval requires five (5) affirmative votes and the applicant could request a postponement until a full Board is present.

Jamal Hamood, attorney representing the church, asked to postpone the item until a full board is present.

PUBLIC HEARING OPENED

There was no one present who wished to speak.

PUBLIC HEARING CLOSED

Resolution # PC-2019-11-081

Moved by: Rahman Support by: Tagle

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval for the proposed Bethesda Romanian Pentecostal Church Addition, North side of Long Lake Road, East of John R, South of Tucker (2075 E. Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District, be postponed to the December 10, 2019 Regular meeting.

Yes: All present (6) Absent: Crusse, Faison, Fowler

MOTION CARRIED

Vice Chair Krent said the Public Hearing would continue at the December 10, 2019 Regular meeting.

 <u>PUBLIC HEARING - SPECIAL USE APPROVAL AND PRELIMINARY SITE PLAN</u> <u>REVIEW (File Number SU JPLN2019-0036)</u> – Proposed Horizon Bank Site Improvements, West side of Crooks, South of Big Beaver (2555 Crooks), Section 29, Currently Zoned O (Office) District

Mr. Savidant reviewed the proposed Horizon Bank application. He addressed the location of the drive-through, parking, landscaping improvements, lighting and elevations. Mr. Savidant said the Special Use Standards have been met and it is recommended to grant Special Use Approval and Preliminary Site Plan Approval with one condition as identified in the Planning Consultant report dated November 5, 2019.

Present were Chris Brayak of Wightman Architects and James Jaska of Horizon Bank.

Mr. Brayak addressed site improvements, specifically landscaping and lighting.

There was discussion on:

- Building occupancy; bank and other tenants.
- Ground sign; application in process.

PUBLIC HEARING OPENED

There was no one present who wished to speak. *

PUBLIC HEARING CLOSED

Resolution # PC-2019-11-082

Moved by: Hutson Support by: Rahman

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval for the proposed Horizon Bank Site Improvements, including ATM drive-through, West side of Crooks, South of Big Beaver (2555 Crooks), Section 29, Currently Zoned O (Office) District, be granted, subject to the following condition:

1. Provide lighting fixture cutsheets on plan set prior to Final Site Plan Approval.

Yes: All present (6) Absent: Crusse, Faison, Fowler

MOTION CARRIED

OTHER BUSINESS

7. <u>PUBLIC COMMENT</u> – Items on Current Agenda *

Tom Strat, 2410 Silver Pointe Drive, Waterford; addressed Agenda item #6. Mr. Strat voiced no objection to the application but noted the narrowness of the drive-through lane.

8. PLANNING COMMISSION COMMENT

There were general Planning Commission comments.

The Regular meeting of the Planning Commission adjourned at 7:23 p.m.

Respectfully submitted,

Tom Krent, Vice Chair

Kathy L. Czarnecki, Recording Secretary

C:\Users\bob\Documents\Kathy\COT Planning Commission Minutes\2019\2019 11 26 Regular Meeting_Draft.doc

DATE: December 5, 2019

TO: Planning Commission

- FROM: R. Brent Savidant, Community Development Director
- SUBJECT: <u>PUBLIC HEARING SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File</u> <u>Number SU JPLN2019-0033</u>) – Proposed Bethesda Romanian Pentecostal Church Addition, North of Long Lake, East of John R, South of Tucker (2075 E Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District

The petitioner CMA Design Services submitted the above referenced Special Use Approval and Preliminary Site Plan Approval application to construct an addition to Bethesda Romanian Pentecostal Church.

Bethesda Romanian Pentecostal Church submitted a previous Special Use application for an addition in 2017. The item was considered by the Planning Commission at four meetings and was denied approval on April 9, 2019. This application is considered a new and separate application. A public hearing was opened (and left open) on November 26, 2019 and the item was postponed by the Planning Commission at the request of the applicant.

The attached report prepared by Carlisle/Wortman Associates, Inc. (CWA), the City's Planning Consultant, summarizes the application. CWA prepared the report with input from various City departments including Planning, Engineering, Public Works and Fire. City Management supports the findings of fact contained in the report and recommends approval of the project, as noted.

Attachments:

- 1. Maps
- 2. Report prepared by Carlisle/Wortman Associates, Inc.
- 3. Public comment

G:\SPECIAL USE\SU JPLN2019-0033 BETHESDA CHURCH ADDITION\PC Memo 12 10 2019.docx

PROPOSED RESOLUTION

<u>PUBLIC HEARING - SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (File</u> <u>Number SU JPLN2019-0033)</u> – Proposed Bethesda Romanian Pentecostal Church Addition, North of Long Lake, East of John R, South of Tucker (2075 E Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District

Resolution # PC-2019-11-

Moved by: Seconded by:

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval for the proposed Bethesda Romanian Pentecostal Church Addition, North side of Long Lake Road, East of John R, South of Tucker (2075 E. Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District, be [approved] or [approved with conditions] or [denied] or [postponed].

The Planning Commission makes the following findings of fact and conclusions based on written materials, comments and testimony of the Applicant's representatives, other interested persons, professional consultants and other factual material presented to the Commission to assist with its deliberation:

- 1. The building addition **[is]** <u>or</u> **[is not]** designed in a manner that is harmonious with the character of adjacent property and the surrounding area because:
 - a. the applicant [has] or [has not] mitigated the building massing and visual impact through architectural design and landscape buffering.
 - b. Building massing is inconsistent with the character of adjacent properties and the surrounding area.
 - c. A tenet of the Master Plan is the protection of single family neighborhoods. The proposed addition [is] <u>or</u> [is not] compatible with the existing homes on Tucker.
 - d. [insert additional reasons, if any]
 - e. [insert additional reasons, if any]
- The proposed addition [does] or [does not] impact traffic on Tucker Street and [does] or [does not] significantly impact traffic entering and exiting the site because ______.
- 3. The project **[is]** <u>or</u> **[is not]** adequately served by essential public facilities and services because _____.
- 4. The addition [complies] or [does not comply] with all applicable ordinance standards because _____.

5. The proposed addition [**does**] <u>or</u> [**does not**] unreasonably impact the quality of the neighborhood on Tucker Street in comparison to the impacts associated with typical permitted uses.

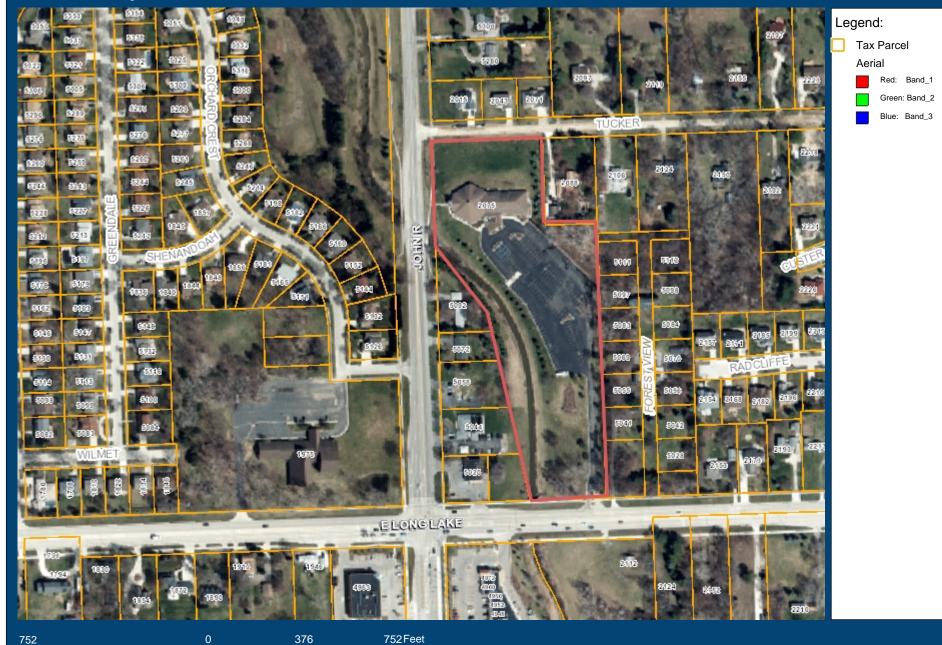
Be it finally resolved, approval shall be subject to the following conditions:

Yes: No: Absent: **MOTION CARRIED / FAILED**



Bethesda Romanian Church Addition

City of Troy Planning Department



Scale 1: 4,514

Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.

Printed: 3/7/2017



Bethesda Romanian Church Addition

City of Troy Planning Department



Scale 1: 4,514

Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.



117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

Date: November 5, 2019

Special Use and Site Plan Review For City of Troy, Michigan

Applicant:	Simon Timbuc, Pastor
Project Name:	Bethesda Romanian Pentecostal Church Addition
Plan Date:	September 19, 2019 (Stamped)
Location:	2075 E. Long Lake Road, Troy MI 48085
Zoning:	R-1C Single Family Residential
Action Requested:	Special Use and Site Plan Approval

PROJECT AND SITE DESCRIPTION

The applicant is requesting site plan approval for an addition and site improvements to an existing place of worship located on East Long Lake Road, east of John Road. The proposed two-story addition is a total of 15,780 square feet. The addition is located on the north side of the property, adjacent to Tucker Drive. The two-story building addition includes a fellowship hall, warming kitchen, chapel, restrooms, and eight (8) Sunday school classrooms. Other site improvements include additional landscaping, and a stormwater management detention pond. The addition will require the regrading of the berm that exists along Tucker.

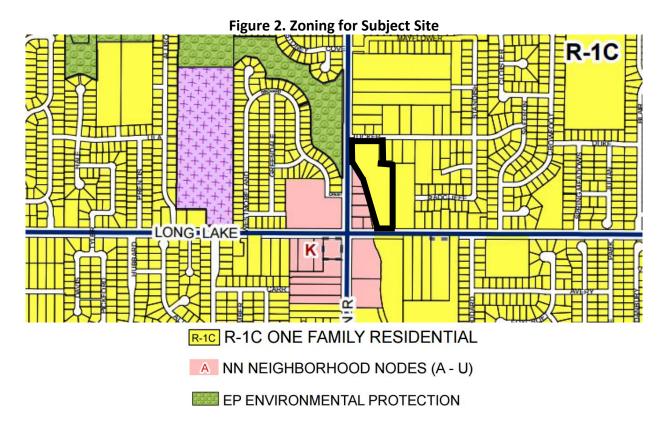
The Planning Commission denied for a building addition on April 4, 2019. The applicant has resubmitted a revised application.

The site is zoned R-1C, and the proposed additional requires a Special Use Permit.



Figure 1. Location of Subject Site

Source: Google Maps



Source: City of Troy Zoning Map

Table 1. Zoning of Adjacent Properties

	Master Plan	Zoning	<u>Use</u>
North	Single Family Residential	R-1C	Residential
South	Single Family Residential	R-1C and NN (K)	Commercial & Residential
East	Single Family Residential	R-1C	Residential
West	Single Family Residential	NN (K) and EP	Residential & Environmental
			Preservation

PHOTO OF LOCATION

The proposed location of addition.



LAST PLANNING COMMISSION REVIEW

The Planning Commission passed the following resolution on April 9, 2019:

Resolution # PC-2019-04-026 Moved by: Crusse

Support by: Krent

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval for the proposed Bethesda Romanian Pentecostal Church Addition, North side of Long Lake, East of John R, South of Tucker (2075 E. Long Lake), Section 12, Currently Zoned R-1C (One Family Residential) District, be denied.

The Planning Commission makes the following findings of fact and conclusions based on written materials, comments and testimony of the Applicant's representatives, other interested persons, professional consultants and other factual material presented to the Commission to assist with its deliberation:

- 1. The building addition is not designed in a manner that is harmonious with the character of adjacent property and the surrounding area because:
 - a. The applicant has not mitigated the building massing and visual impact through architectural design and landscape buffering.
 - b. Building massing is inconsistent with the character of adjacent properties and the surrounding area.
 - c. A tenet of the Master Plan is the protection of single family neighborhoods. The proposed addition is not compatible with the existing homes on Tucker.
- 2. The proposed addition does unreasonably impact the quality of the neighborhood on Tucker Street in comparison to the impacts associated with typical permitted uses.

Yes: Crusse, Faison, Krent, Lambert, Rahman No: Hutson, Tagle Absent: Fowler Recused: Apahidean

PLAN CHANGES

The applicant has resubmitted their application. Though it's a new application, we note the following changes to the application:

- 1. Reduced the building size from 19,167 sq/ft to 15,780 sq/ft
- 2. Reduced building footprint from 14,605 sq/ft to 12,026 sq/ft
- 3. Reduced height to 26'-8"

- 4. Excluding window bumpouts, increased setback on Tucker from 55-feet to 59-feet
- 5. Eliminated the gym use
- 6. Reduced number of classrooms
- 7. Altered first floor windows to reduce visual impact on Tucker
- 8. Eliminated all second-floor windows

LANDSCAPE, ACOUSTICS, and PHOTOMETRICS

Landscape:

The applicant proposes the following landscaping:

Туре М		Number	Height at time of planting	Maximum Height
Concolor Fir		18	10-12 feet	30-50 feet
Little Leaf Linden		6	4-5-inch diameter	50-60 feet
Green	Giant	12	9-10 feet	50-60 feet
Arborvitae				
Forsythia (shrub)		21	36 inches	8-10 feet

Acoustic Calculations:

The applicant has provided an acoustic study. The study concludes that at the Tucker Road property line the decibel level is approximately 22 decibels. That equates to a "faint" noise.

Photometric:

The applicant has provided a photometric plan that complies with ordinance requirements.

STANDARDS FOR APPROVAL

Places of Worship, and associated uses, are permitted subject to Special Use approval. For any special use, according to Section 9.02.D, the Planning Commission shall "…review the request, supplementary materials either in support or opposition thereto, as well as the Planning Department's report, at a Public Hearing established for that purpose, and shall either grant or deny the request, table action on the request, or grant the request subject to specific conditions."

Section 9.03 states that before approving any requests for Special Use Approval, the Planning Commission shall consider:

1. Compatibility with Adjacent Uses. The Special Use shall be designed and constructed in a manner harmonious with the character of adjacent property and the surrounding area. In determining whether a Special Use will be harmonious and not create a significant detrimental impact, as compared to the impacts of permitted uses.

- 2. Compatibility with the Master Plan. The proposed Special Use shall be compatible and in accordance with the goals and objectives of the City of Troy Master Plan and any associated sub-area and corridor plans.
- 3. Traffic Impact. The proposed Special Use shall be located and designed in a manner which will minimize the impact of traffic, taking into consideration: pedestrian access and safety; vehicle trip generation (i.e. volumes); types of traffic, access location, and design, circulation and parking design; street and bridge capacity and, traffic operations at nearby intersections and access points. Efforts shall be made to ensure that multiple transportation modes are safely and effectively accommodated in an effort to provide alternate modes of access and alleviate vehicular traffic congestion.
- 4. Impact on Public Services. The proposed Special Use shall be adequately served by essential public facilities and services, such as: streets, pedestrian or bicycle facilities, police and fire protection, drainage systems, refuse disposal, water and sewage facilities, and schools. Such services shall be provided and accommodated without an unreasonable public burden.
- 5. Compliance with Zoning Ordinance Standards. The proposed Special Use shall be designed, constructed, operated and maintained to meet the stated intent of the zoning districts and shall comply with all applicable ordinance standards.
- 6. Impact on the Overall Environment. The proposed Special Use shall not unreasonably impact the quality of natural features and the environment in comparison to the impacts associated with typical permitted uses.
- 7. Special Use Approval Specific Requirements. The general standards and requirements of this Section are basic to all uses authorized by Special Use Approval. The specific and detailed requirements relating to particular uses and area requirements must be also satisfied for those uses.

RECOMMENDATION

The Planning Commission shall determine if the applicant has provided sufficient evidence to meet the special use standards.

If the Planning Commissions finds the special use standards have been met, the Planning Commission may approve the special use and preliminary site plan.

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP

BETHESDA ROMANIAN PENTECOSTAL CHURCH 2075 E LONG LAKE RD, TROY, MI 48085



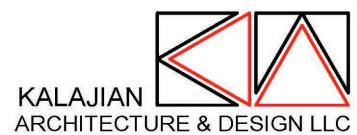
(RENDERING SHOWN AT 10 YEAR GROWTH)



(RENDERING SHOWN AT 10 YEAR GROWTH)



BUILDER: CMA DESIGN SERVICES 8183 RHODE DDR. SUITE B SHELBY TWP., MI 48317



ARCHITECT: KALAJIAN ARCHITECTURE & DESIGN 7871 AUSTIN DR., TROY, MI 48083

ORIGINAL PROPOSED BUILDING APRIL 2019

NEW PROPOSED BUILDING





NWNFR:

BETHESDA ROMANIAN PENTECOSTAL CHURCH 2075 E LONG LAKE RD. TROY, MI 48085

INDEX:

TITLE SITE OVERVIEW/LANDSCA NOWAK & FRAUS NOWAK & FRAUS NOWAK & FRAUS NOWAK & FRAUS FEMA FIRM MAP FEMA FIRM MAP FEMA CLOSE UP FLOOR PLAN SECOND FLOOR PLAN ELEVATIONS ACOUSTICS SITE ISOMETRIC PHOTOMETRIC

-PLEASE BE AWARE THAT THE KITCHEN SHOWN ON THE PROPOSED ADDITION IS SOLELY FOR THE PURPOSE OF WARMING ALREADY PREPARED FOOD -DUMPSTER LOCATION/ TRASH PICKUP THE CHURCH WILL NOT HAVE ANY TRASH PICK UP SCHEDULED ALONG TUCKER DRIVE. AND WILL UTILIZE THE SAME TRASH SERVICE THAT IS CURRENTLY IN PLACE FOR THE EXISTING BUILDING. THE EXISTING DUMPSTER SURROUND IS LOCATED ON THE SOUTHWEST SIDE OF THE CHURCH'S PARKING LOT AND ACCESSED THROUGH THEIR LONG LAKE RD ENTRANCE -ALTERNATE LOCATIONS FOR ADDITION: -THE REMANING SITE HAS MANY RESTRICTIONS SUCH AS PARKING, EASEMENTS AND WETLANDS LOCATIONS. AN ADDITION ON THE SOUTH SIDE OF THE EXISTING CHURCH WOULD REQUIRE LOST PARKING SPACES TO BE RE-LOCATED TO THE NORTH SIDE OF THE CHURCH -ANYWHERE ELSE THE ADDITION IS PLACED WITHIN THE SITE WOULD OCCUR WITHIN THE FLOOD PLAN AS SHOWN WITHIN THE FLOOD PLAIN MAP PROVIDED. THIS WOULD NOT MAKE IT FEASIBLE WITHOUT MITIGATION WHICH WOULD BE IMPRACTICAL WITH THIS SITE SINCE MOST OF IT IS WITHIN THE FLOOD PLAIN ZONE OTHER THAN THE EXISTING AND PROPOSED BUILDING LOCATION -THE SCHEDULED CHURCH TIMES ARE AS FOLLOWS: -SUNDAY 9:00 A.M. AND 5:00 P.M. -TUESDAY: 630 P.M. -WEDNESDAY: 7:00 P.M. -THURSDAY: 5:30 P.M. AND 7:00 P.M. -FRIDAY: 6:00 P.M. (BAND & CHOIR PRACTICE) AND 6:30 P.M. (BAND & CHOIR PRACTICE) 8:00P.M (YOUTH GROUP) -SATURDAY: 10:30 A.M. AND 6:00 P.M. (MENS CHOIR) -A FULL SCHEDULE INCLUDING SPECIAL EVENTS CAN BE SEEN BY CLICKING ON THE PROVIDED LINK TO THE CHURCH WEBSITE CALANDER: HTTP://WWW.BETEZDA.COM/CALANDER/ -LANDSCAPING MAINTENANCE -ALL LANDSCAPED AREAS SHALL BE AUTOMATICALLY IRRIGATED SEE PG. C-1 -ALL LIGHTING ON THE SITE SHALL BE SHIELDED AND NOT ENCROACH UPON ABUTTING PROPERTIES OR RIGHT-OF-WAYS -LANDSCAPING TO BE INSTALLED AT THE START OF THE PROJECT TO ACT AS A VISUAL BUFFER BETWEEN NEIGHBORS AND ACTIVE SITE -SITE ACCESS -SEE PAGE C-1 TO VIEW SUGGESTED CONSTRUCTION DRIVEWAYS -NOISE -PLEASE SEE ACOUSTIC STUDY ON PAGE A-4 -SIZE OF THE BUILDING -THE OVERALL SQFT. WAS REDUCED FROM 19.200 SQFT (FIRST SUBMISSION) TO 12.026 SQFT. PLEASE SEE PRIMARY PROJECT CHANGES AS WELL AS PAGE A-3 -THE BUILDING HEIGHT WAS REDUCED FROM THE ORIGINAL 39'-O" TO A HEIGHT OF 28'-O" PLEASE SEE PRIMARY PROJECT CHANGES AS WELL AS PAGE A-3 -WINDOWS -WINDOWS WERE KEPT TO A MINIMUM ON THE NORTH SIDE OF THE BUILDING WITH WESTWARD

EXPOSURE(NOT FACING TUCKER) -PROPERTY VALUE -N/A

BUILDING INFORMATION:

BUILDING CODE: MECHANICAL CODE: PLUMBING CODE: ELECTRICAL CODE ENERGY CODE: FIRE CODE: BARRIER FREE:

USE GROUP:

CONSTRUCTION TYPE: **BUILDING AREA:**

SPRINKLERED: RISK CATEGORY: OCCUPANCY:

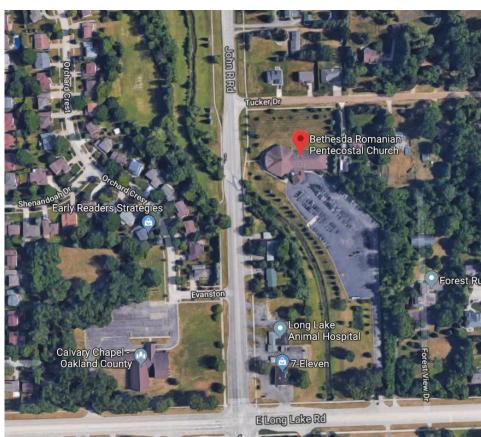
ASSEMBLY:

CLASSROOM: FOUNDATION SOIL BEARING: ROOF LIVE LOAD: ROOF DEAD LOAD: GROUND SNOW LOAD: FROST DEPTH: WIND LOAD: SEISMIC LOAD:

	PAGE NUMBER
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	A-4
	A-5
	E-1

PREVIOUS PLANNING COMMISION COMMENTS

-SMELLS/ODORS COMING FROM THE KITCHEN



PROJECT LOCATION:

BETHESDA ROMANIAN PENTECOSTAL CHURCH 2075 E LONG LAKE RD. TROY, MI 48085

GENERAL STATEMENTS/NOTES:

-LANDSCAPING MAINTENANCE - SEE ATTACHED STATEMENT -CLASSROOM USE- SEE ATTACHED STATEMENT -CONSTRUCTION TRAFFIC- SEE PAGE C-1 -MEETING WITH NEIGHBORS ON / -CONSTRUCTION PHASE 1- PLANTING TO CONCEAL SITE WORK -PARKING

PRIMARY PROJECT CHANGES

-FOOTPRINT:

FIRST SUBMISSION- 19,200 SQFT APRIL 2019 SUBMISSION- 14,605 SQFT. NEW SUBMISSION- 12,026 SQFT. DIFFRENCE BETWEEN SUBMISSIONS- 2,579 SQFT REDUCTION 37.4 % FROM THE FIRST SUBMISSION

-HEIGHT: FIRST SUBMISSION- 39'-O" 2019 SUBMISSION- 30'-0" NEW SUBMISSION- 26'-8" -VISIBILITY FROM BLDG TO NEIGHBORS -REMOVED BASKETBALL COURT -REDUCED CLASSROOMS

PREVIOUS SUBMISSIONS

PLANNING	COMMISSION	SUBMISSION
PLANNING	COMMISSION	SUBMISSION

2017.04.25 2017.07.25 2018.10.09 2019.02.26 2019.

2015 MICHIGAN BUILDING CODE 2015 MICHIGAN MECHANICAL CODE 2015 MICHIGAN PLUMBING CODE 2017 NATIONAL ELECTRIC CODE (2017 NEC) W/ PART 8 AMENDMENTS ASHRAE 90.1 2013 PART 10A MICHIGAN UNIFORM ENERGY 2015 INTERNATIONAL FIRE CODE P.A. 1 OF 1966 AS AMENDED CHAPTER 11 OF THE MICHIGAN BUILDING CODE, ICC/ANSI A117.1-3009 STANDARD AS REFRENCED FROM CHAPTER 11 A-3 ASSEMBLY USES INTENDED FOR WORSHIP FIRST FLOOR- 12,026 SQFT SECOND FLOOR- 3,754 SQFT YES 111 PER TABLE 1004.2 MICHIGAN BUILDING CODE 2015 (FLOOR AREA IN SQ.FT. PER PERSON) (MAXIMUM FLOOR AREA ALLOWANCE PER OCCUPANT) WITHOUT FIXED SEATS 7 NET STANDING SPACE 5 NET UNCONCENTRATED (TABLES AND CHAIRS) 15 NET 20 NET

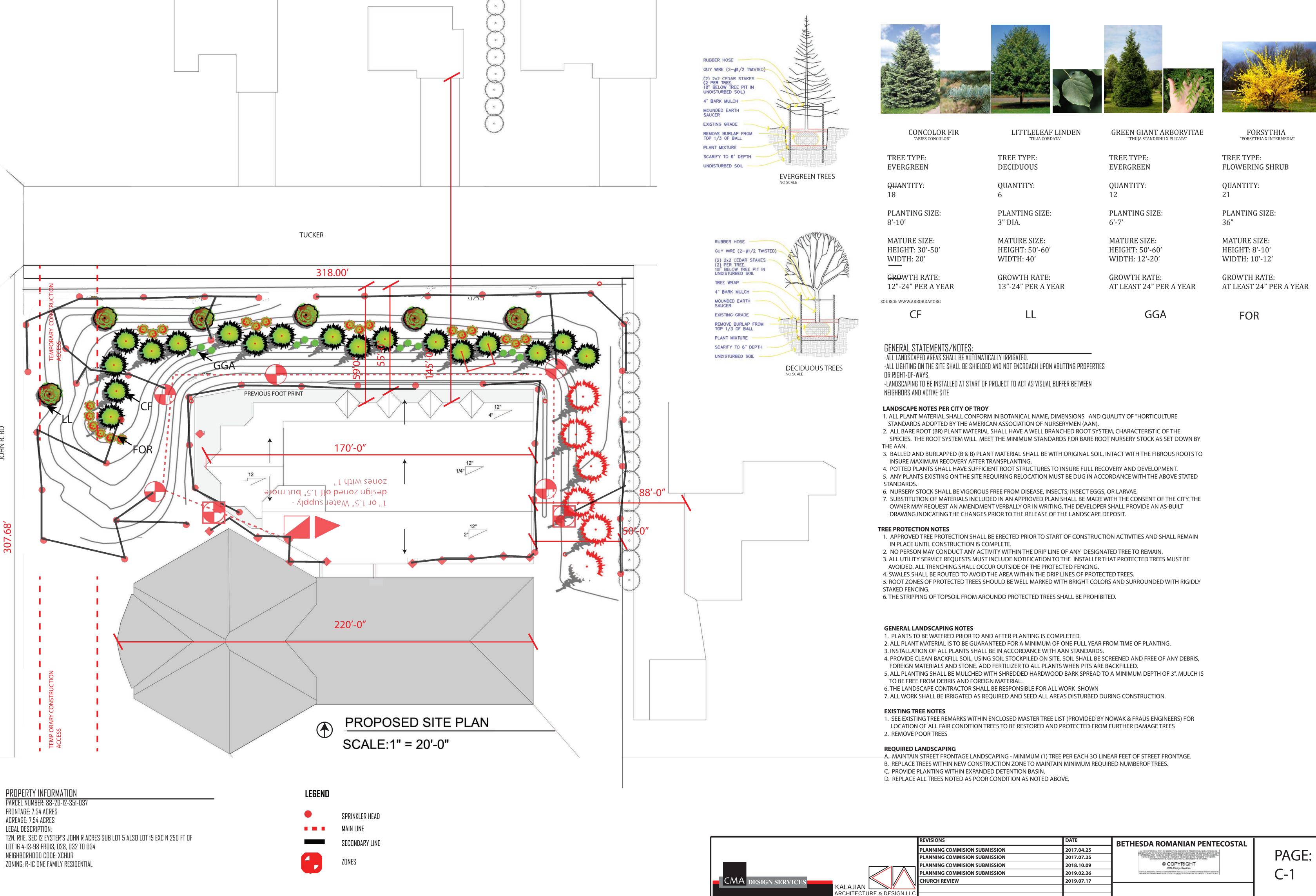
2,000 PSF ASSUMED

42 IN. 115 MPH Ss= 0112 S1= 0.043

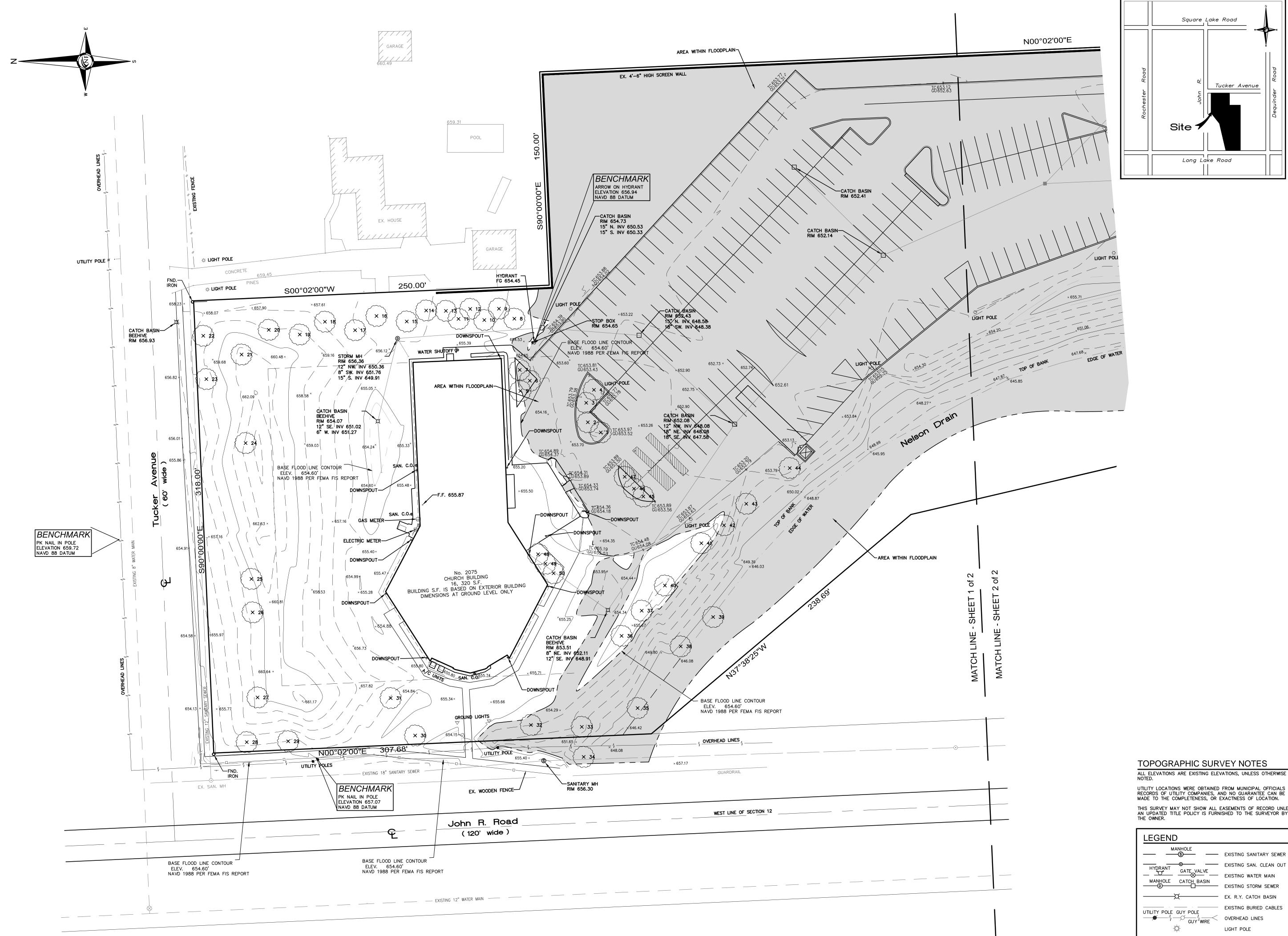
© COPYRIGHT **CMA Design Services**

NTRACTORS SHALL VERIFY AND COORDINATE ALL DIMENSIONS ON THESE DRAWINGS, AS WELL AS REVIE

EXTERIOR BUILDING ELEVATIONS. SECTION AND DETAILS BEFORE COMME OR CONFLICT OCCURS BETWEEN THESE PLANS, BUILDING ELEVATIONS, SE

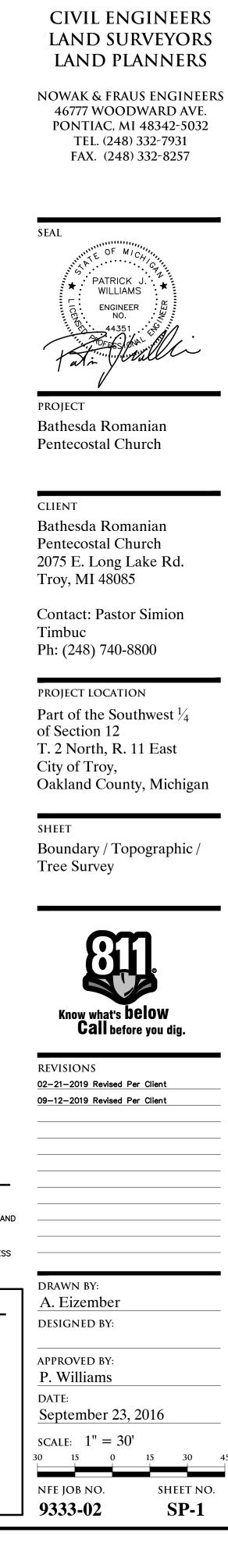


ONS	DATE	BETHESDA ROMANIAN PENTECOSTAL
IING COMMISION SUBMISSION	2017.04.25	ALL CONTRACTORS SHALL YEAP AND COORDINATE ALL DIRENSIONS ON THESE DRIVINGS, AS VELL AS FRANK AND COORDINATE PLAN SHITL HOTTROP BILLINGS BEATTON AND FEALS BEFORE CONTRACTOR WITH THE VORK
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CH REVIEW	2019.07.17	

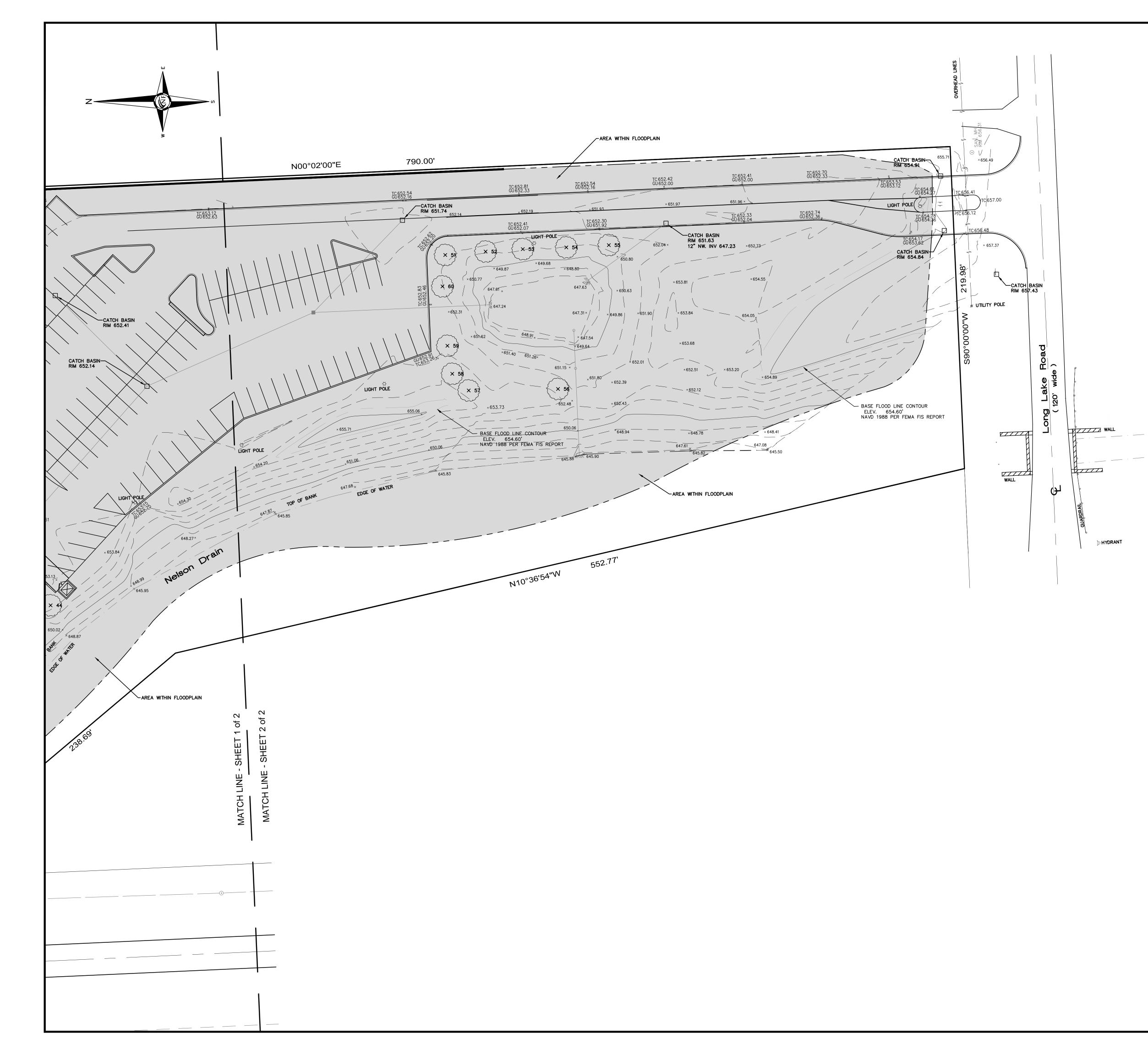


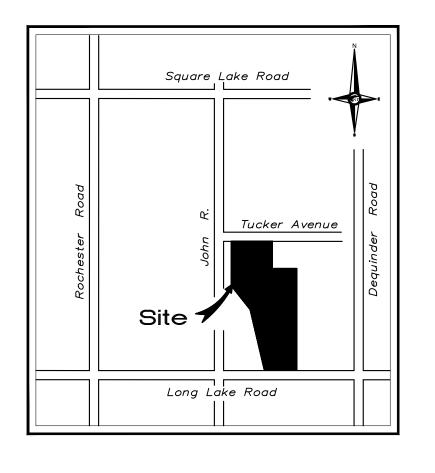
UTILITY LOCATIONS WERE OBTAINED FROM MUNICIPAL OFFICIALS AND RECORDS OF UTILITY COMPANIES, AND NO GUARANTEE CAN BE MADE TO THE COMPLETENESS, OR EXACTNESS OF LOCATION. THIS SURVEY MAY NOT SHOW ALL EASEMENTS OF RECORD UNLESS AN UPDATED TITLE POLICY IS FURNISHED TO THE SURVEYOR BY THE OWNER.

MANHOLE EXISTING SANITARY SEWER HYDRANT GATE VALVE GATE VALVE EXISTING WATER MAIN MANHOLE CATCH BASIN EXISTING STORM SEWER EXISTING BURIED CABLES
HYDRANT GATE VALVE EXISTING WATER MAIN MANHOLE CATCH BASIN EXISTING STORM SEWER
MANHOLE CATCH BASIN MANHOLE CATCH BASIN EXISTING WATER MAIN EXISTING STORM SEWER EX. R.Y. CATCH BASIN
EXISTING STORM SEWER
UTILITY POLE GUY POLE
$- \int \int$
LIGHT POLE
I SIGN
EXISTING GAS MAIN



ENGINEERS







PROJECT Bathesda Romanian Pentecostal Church

aria

CLIENT

Bathesda Romanian Pentecostal Church 2075 E. Long Lake Rd. Troy, MI 48085

Contact: Pastor Simion Timbuc Ph: (248) 740-8800

PROJECT LOCATION

Part of the Southwest ¹/₄ of Section 12 T. 2 North, R. 11 East City of Troy, Oakland County, Michigan

SHEET

Boundary / Topographic / Tree Survey



Know what's **below Call** before you dig.

REVISIONS	
02-21-2019 Revised Per Client	
09–12–2019 Revised Per Client	
DRAWN BY:	
A. Eizember	
DESIGNED BY:	
APPROVED BY:	
P. Williams	
DATE:	
September 23, 2016	
SCALE: $1'' = 30'$	
30 15 0 15 30	

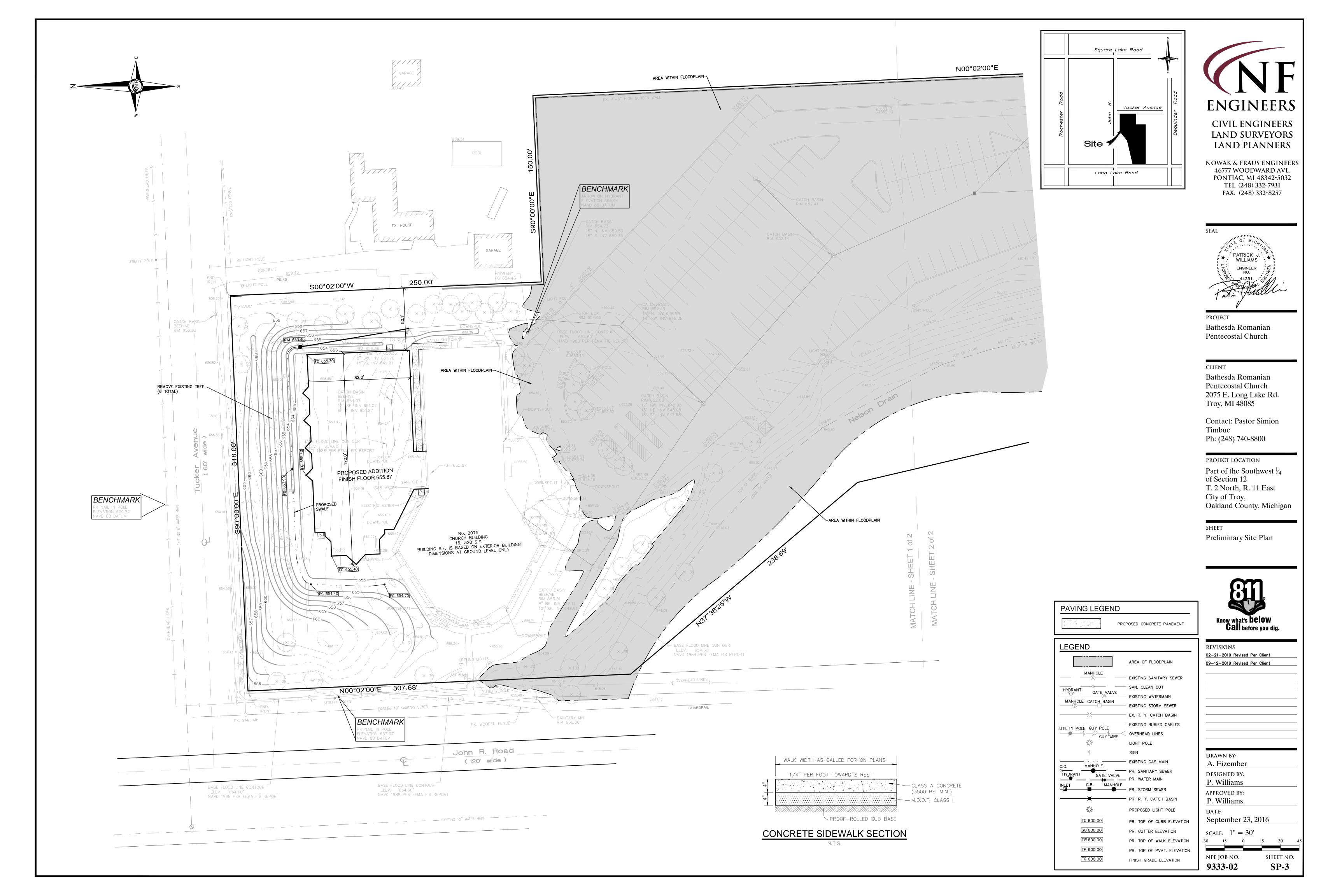
NFE JOB NO. **9333-02** SHEET NO.

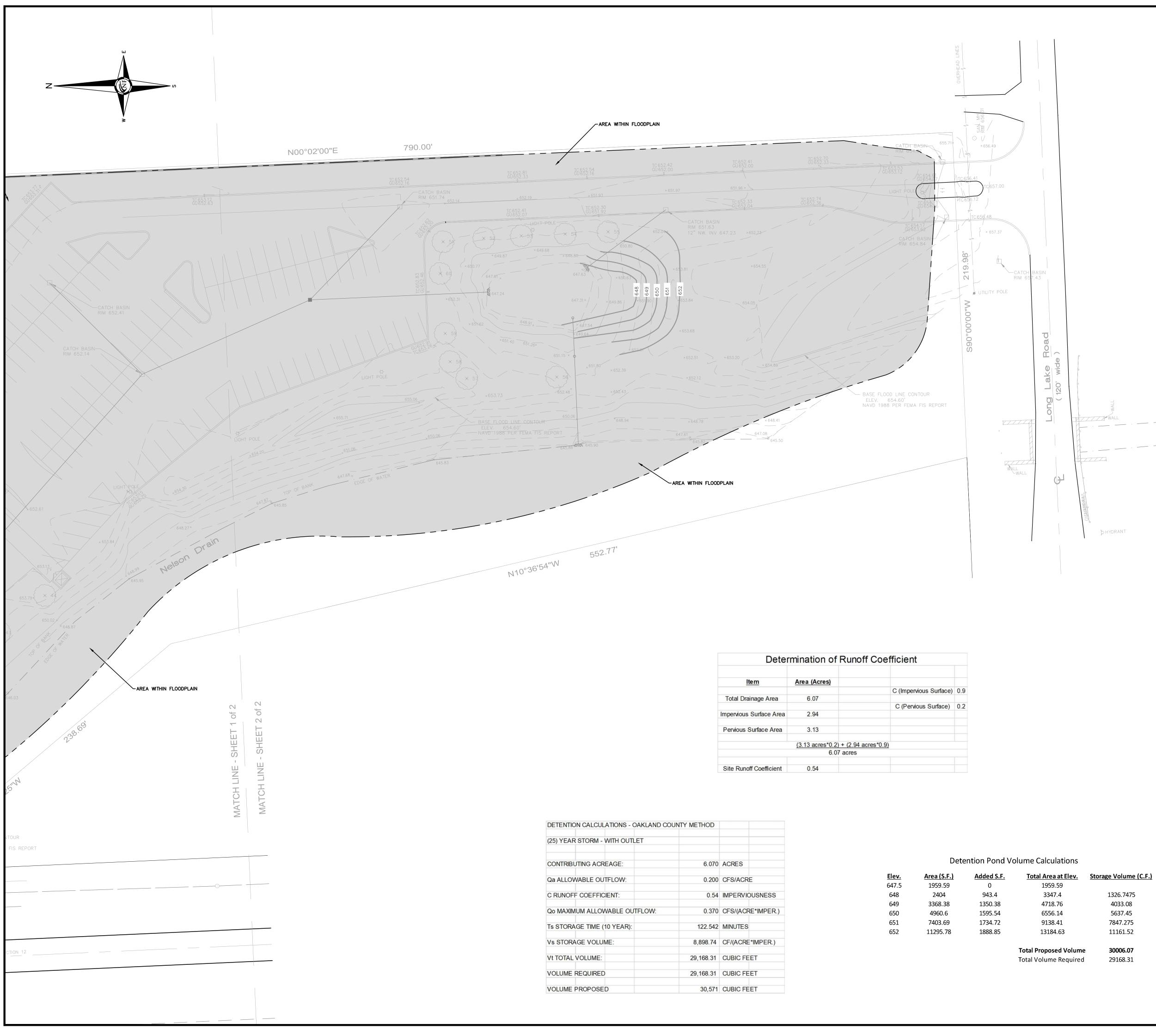
SP-2

TOPOGRAPHIC SURVEY NOTES ALL ELEVATIONS ARE EXISTING ELEVATIONS, UNLESS OTHERWISE NOTED.

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LEGEND	
MANHOLE	EXISTING SANITARY SEWER
	EXISTING SAN. CLEAN OUT
MANHOLE CATCH BASIN	EXISTING WATER MAIN
	EXISTING STORM SEWER
X	EX. R.Y. CATCH BASIN
UTILITY POLE GUY POLE	EXISTING BURIED CABLES
	OVERHEAD LINES
ф.	LIGHT POLE
4	SIGN
· · · ·	EXISTING GAS MAIN



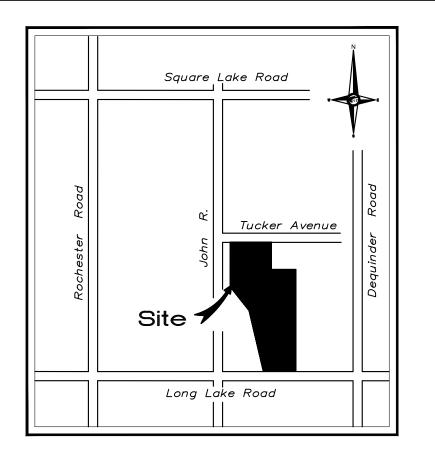


Deter	mination of	Runoff Coefficient	
ltem	Area (Acres)		
		C (Impervious Surface)	0.9
Total Drainage Area	6.07		
		C (Pervious Surface)	0.2
Impervious Surface Area	2.94		
Pervious Surface Area	3.13		
	(3.13 acres*0.2)	+ (2.94 acres*0.9)	
	6.0	7 acres	
Site Runoff Coefficient	0.54		

DETENTION CALCULATIONS - OAKLAND CO	DUNTY METHOD

(25) YEAR STORM - WITH OUTLET		
CONTRIBUTING ACREAGE:	6.070	ACRES
Qa ALLOWABLE OUTFLOW:	0.200	CFS/ACRE
C RUNOFF COEFFICIENT:	0.54	IMPERVIOUSNESS
Qo MAXIMUM ALLOWABLE OUTFLOW:	0.370	CFS/(ACRE*IMPER.)
Ts STORAGE TIME (10 YEAR):	122.542	MINUTES
Vs STORAGE VOLUME:	8,898.74	CF/(ACRE*IMPER.)
Vt TOTAL VOLUME:	29,168.31	CUBIC FEET
VOLUME REQUIRED	29,168.31	CUBIC FEET
VOLUME PROPOSED	30,571	CUBIC FEET

ev.	<u>Area (S.F.)</u>	Added S.F.	<u>Total Area at Ele</u>
17.5	1959.59	0	1959.59
48	2404	943.4	3347.4
49	3368.38	1350.38	4718.76
50	4960.6	1595.54	6556.14
51	7403.69	1734.72	9138.41
52	11295.78	1888.85	13184.63





Bathesda Romanian Pentecostal Church

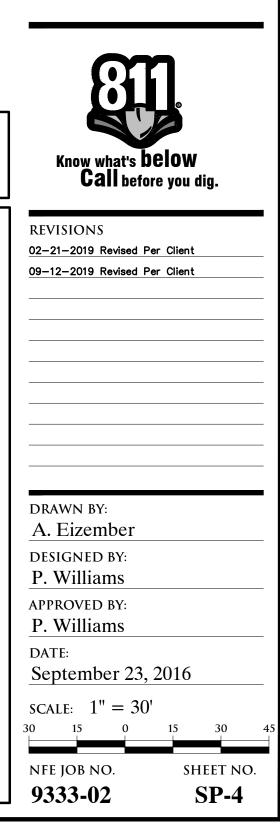
CLIENT

Bathesda Romanian Pentecostal Church 2075 E. Long Lake Rd. Troy, MI 48085

Contact: Pastor Simion Timbuc Ph: (248) 740-8800

PROJECT LOCATION Part of the Southwest $\frac{1}{4}$ of Section 12 T. 2 North, R. 11 East City of Troy, Oakland County, Michigan

SHEET Preliminary Site Plan



PAVING LEGEND	
	POSED CONCRETE PAVEMENT
LEGEND	
	AREA OF FLOODPLAIN
MANHOLE	EXISTING SANITARY SEWER
HYDRANT	SAN. CLEAN OUT
	EXISTING WATERMAIN
MANHOLE CATCH BASIN	EXISTING STORM SEWER
X	EX. R. Y. CATCH BASIN
UTILITY POLE GUY POLE	EXISTING BURIED CABLES
	OVERHEAD LINES
**	LIGHT POLE
q	SIGN
 C.O. MANHOLE	EXISTING GAS MAIN
HYDRANT GATE VALVE	PR. SANITARY SEWER
INLET C.B. MANHOLE	PR. WATER MAIN
	PR. STORM SEWER
*	PR. R. Y. CATCH BASIN
ж.	PROPOSED LIGHT POLE
TC 600.00	PR. TOP OF CURB ELEVATION
GU 600.00	PR. GUTTER ELEVATION
TW 600.00	PR. TOP OF WALK ELEVATION
TP 600.00	PR. TOP OF PVMT. ELEVATION
FG 600.00	FINISH GRADE ELEVATION

CLEARING	Þ HYDRANT

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all the areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on the FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Michigan State Plane South zone 6401 (FIPSZONE 2113). The horizontal datum was NAD83. Differences in datum, spheroid, projection or state plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services

NOAA, N/NGS12 National Geodetic Survey

SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282

(301) 713-3242

To obtain current elevation, description, and/or location information about the bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit their website at http://www.ngs.noaa.gov.

Base Map information shown on this FIRM was derived from the Oakland County GIS Department from photography dated September 2002 or later.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

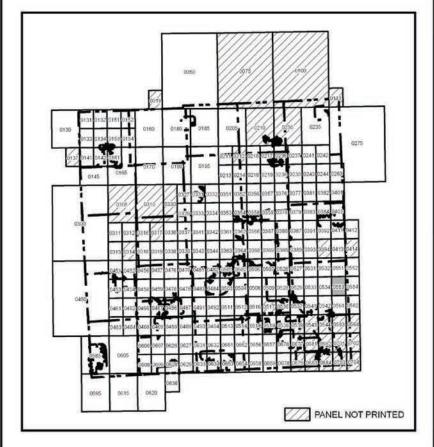
Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and their website at http://msc.fema.gov/.

If you have **questions about this map** or **questions** concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip/.

The profile base lines depicted on this map represent the hydraulic modelin baselines that match the flood profiles in the FIS report. As a result if improved topographic data, the profile base line in some cases may deviate significantly from the channel centerline or appear outside the SFHA.

PANEL INDEX



83° 07' 30.0"

42° 35' 37.5'

400000 FT

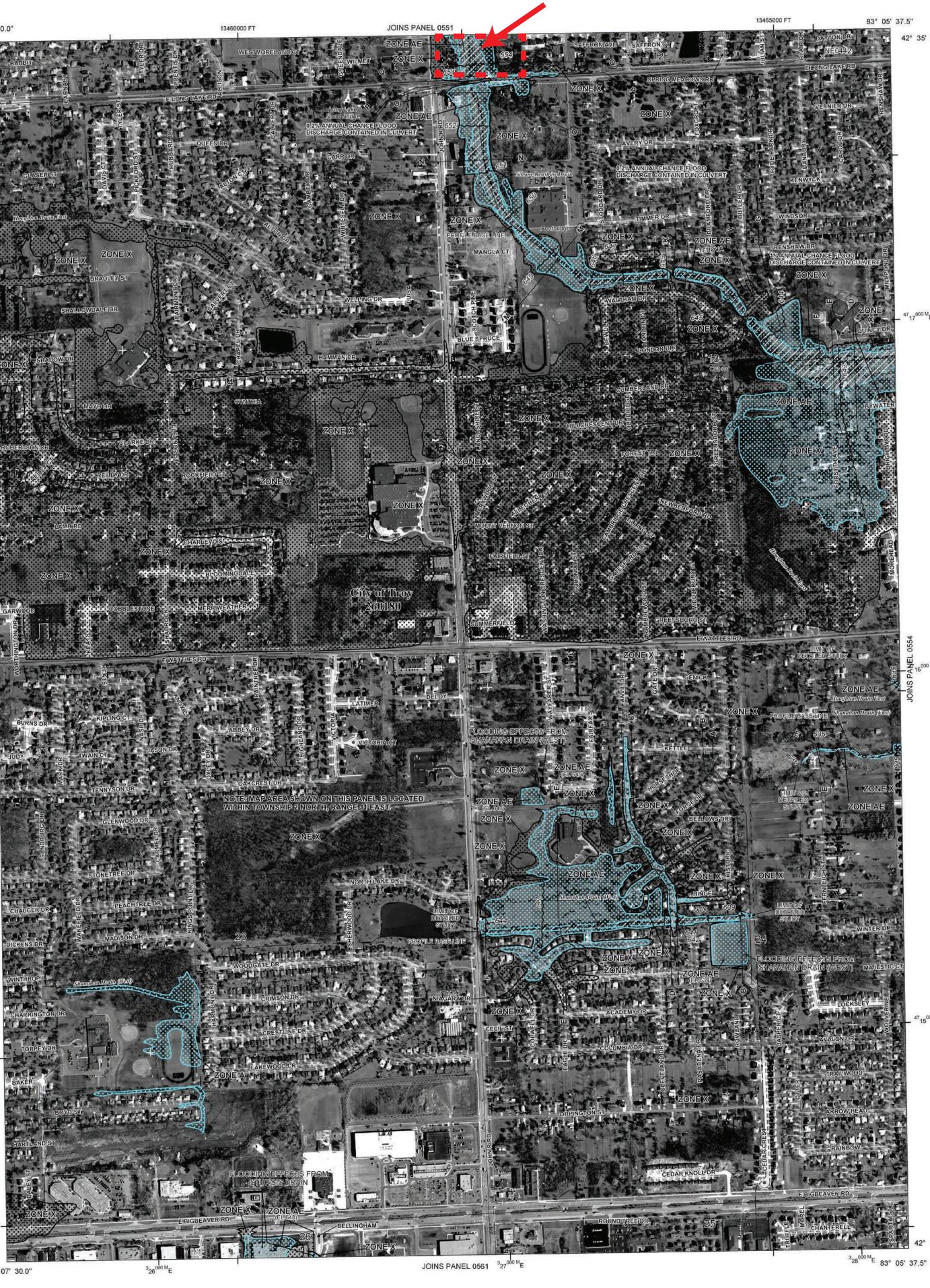
390000 F1

42° 33' 45.0"

395000 FT

83° 07' 30.0"

PROJECT LOCATION





33' 45.0"

INUNDATION	DOD HAZARD AREAS SUBJECT TO BY THE 1% ANNUAL CHANCE FLOOD
that has a 1% chance of being Flood Hazard Area is the area sul of Special Flood Hazard include	0 year flood), also known as the base flood, is the flood equaled or exceeded in any given year. The Special oject to flooding by the 1% annual chance flood. Areas Zones A, AE, AH, AO, AR, A99, V, and VE. The Base surface elevation of the 1% annual chance flood.
LONER	od Elevations determined. Elevations determined.
Elevations de	s of 1 to 3 feet (usually areas of ponding); Base Flood etermined. s of 1 to 3 feet (usually sheet flow on sloping terrain);
	ths determined. For areas of alluvial fan flooding, velocities
chance flood	ial flood hazard formerly protected from the 1% annual event by a flood control system that was subsequently Zone AR indicates that the former flood control system is
being restor greater floor	ed to provide protection from the 1% annual chance or I.
flood protect determined.	rotected from 1% annual chance flood by a Federal tion system under construction; no Base Flood Elevations
Elevations d	I zone with velocity hazard (wave action); no Base Flood etermined. I zone with velocity hazard (wave action); Base Flood Elevations
determined.	
The floodway is the channel of a	REAS IN ZONE AE stream plus any adjacent floodplain areas that must be
kept free of encroachment so the substantial increases in flood heig	at the 1% annual chance flood can be carried without hts.
ZONE X Areas of 0.2	D AREAS % annual chance flood; areas of 1% annual chance flood
with average 1 square mil	e depths of less than 1 foot or with drainage areas less than e; and areas protected by levees from 1% annual chance
flood.	5
	nined to be outside of the 0.2% annual chance floodplain.
	ch flood hazards are undetermined, but possible.
	RRIER RESOURCES SYSTEM (CBRS) AREAS
	PROTECTED AREAS (OPAs) Ily located within or adjacent to Special Flood Hazard Areas.
	1% annual chance floodplain boundary 0.2% annual chance floodplain boundary
	Floodway boundary
	Zone D boundary CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513 (EL 10)	Base Flood Elevation line and value; elevation in feet* Base Flood Elevation value where uniform within zone;
(EL 10) *Referenced to the North Americ	elevation in feet*
	Cross section line
(23) (23) 97° 07' 30" , 32° 22' 30"	Transect line Geographic coordinates referenced to the North American
⁴⁷ 10 ^{000m} N	Datum of 1983 (NAD 83), Western Hemisphere 1000-meter Universal Transverse Mercator grid values, zone 17
4700000 FT	5000-foot grid tick: Michigan State Plane South Coordinate System, 6401 zone (FIPSZONE 2113), Lambert Conformal
HE0181×	Conic projection Bench mark (see explanation in Notes to Users section of this FIRM panel)
• M1.5	River Mile
	MAP REPOSITORY
EFFECTIVE January 16, 2009 – to change incorporate previously issued For community map revision Map History table located in I To determine if flood insuran	listing of Map Repositories on Map Index FECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 29, 2006 E DATE(S) OF REVISION(S) TO THIS PANEL Base Flood Elevations, and Special Flood Hazard Areas and to
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NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all the areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevationstable in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary if Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on the FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Michigan State Plane South zone 6401 (FIPSZONE 2113). The horizontal datum was NAD83. Differences in datum, spheroid, projection or state plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12

National Geodetic Survey SSMC-3, #9202

1315 East-West Highway Silver Spring, Maryland 20910-3282

(301) 713-3242

To obtain current elevation, description, and/or location information about the bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit their website at http://www.ngs.noaa.gov.

Base Map information shown on this FIRM was derived from the Oakland County GIS Department from photography dated September 2002 or later.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

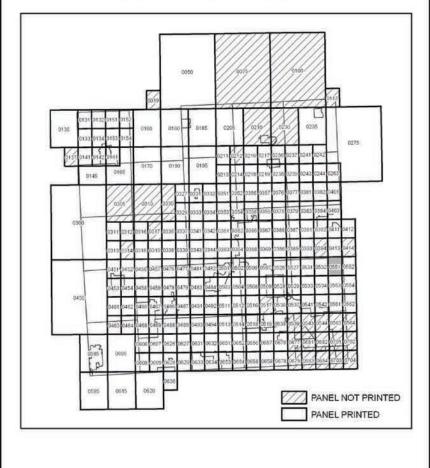
Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and their website at http://msc.fema.gov/.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip/.

The profile base lines depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result if improved topographic data, the **profile base line** in some cases may deviate significantly from the channel centerline or appear outside the SFHA.

PANEL INDEX

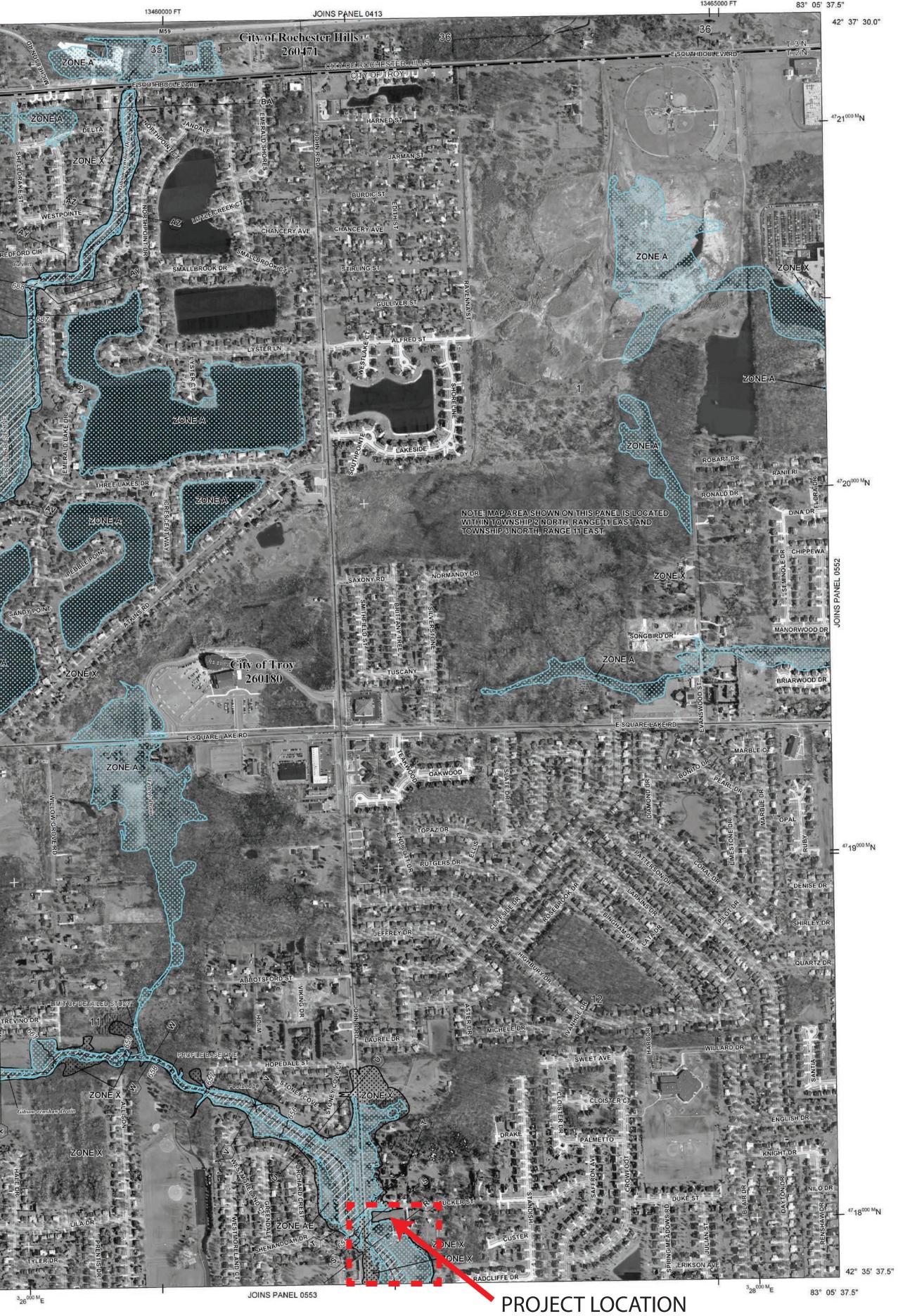


83° 07' 30.0"





42° 35' 37.5" 83° 07' 30.0"



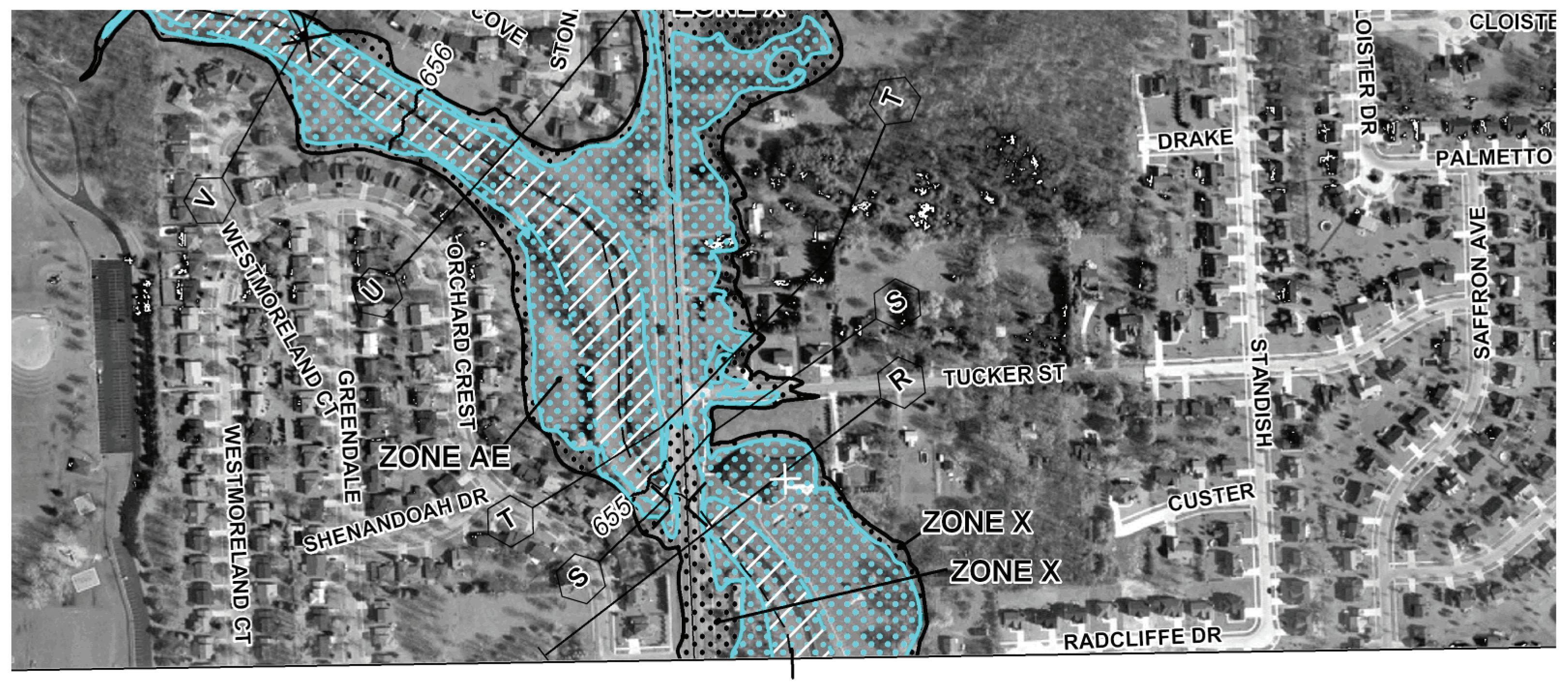


	LEGEND
5' 37.5" 42° 37' 30.0"	SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD The 1% annual chance flood (100 year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood. ZONE A No Base Flood Elevations determined.
- ⁴⁷ 21 ^{000 M} N	ZONE AE Base Flood Elevations determined. ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined. Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined. ZONE AO Area of special flood hazard formerly protected from the 1% annual chance flood event by a flood control system that was subsequently decertified. Zone AP indicates that the former flood control control control system is
	decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood. ZONE A99 Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined. ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined. ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined. ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined. ZONE X FLOODWAY AREAS IN ZONE AE The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights. ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood. ZONE
⁴⁷ 20 ^{000 M} N	COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS OTHERWISE PROTECTED AREAS (OPAs) CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas. 1% annual chance floodplain boundary 0.2% annual chance floodplain boundary 0.2% annual chance floodplain boundary 0.2% annual chance floodplain boundary CBRS and OPA boundary CBRS a
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= ⁴⁷ 19 ^{000 M} N	MAP SCALE 1" = 500' 250 0 500 1000 FEET FEET 150 0 150 300 METERS
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4718 ^{000 M} N 42° 35' 37.5"	Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community. MAP NUMBER 26125C0551F EFFECTIVE DATE
	SEPTEMBER 29, 2006 Federal Emergency Management Agency

EVISIONS DATI **BETHESDA ROMANIAN PENTECOSTAL** PLANNING COMMISION SUBMISSION 2017.04.25 PAGE: LANNING COMMISION SUBMISSION 2017.07.25 © COPYRIGHT LANNING COMMISION SUBMISSION 2018.10.09 CMA Design Services F-2 LANNING COMMISION SUBMISSION 2019.02.26 THE DRAWINGS, DESCRE, DETAILS, AND PLANS IN THIS SET ARE THE PROPERTY OF DIAL Design Services AND ARE NOT TO Describerings. THE SET PLANS, ARE DESIGNATED ONLY FOR THE PROPERTY AND ANY FURTHER IDENTIFICATION OF

FEMA FIRM MAP

Federal Emergency Management Agency



JOINS PANEL 0553



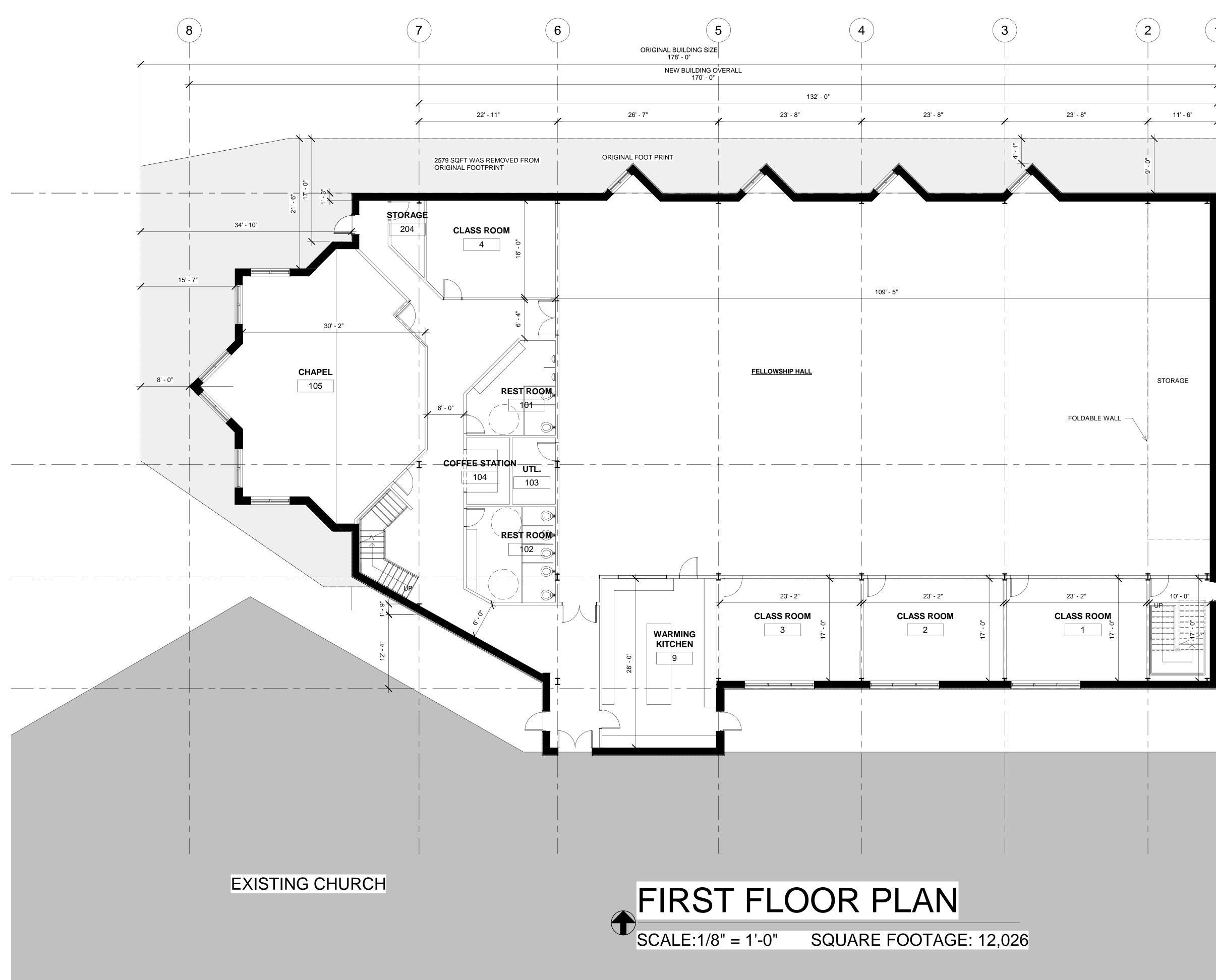
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 Fax: (586) 726-3609

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GENERAL STATEMENTS/NOTES:

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-ALL LANDSCAPED AREAS SHALL BE AUTOMATICALLY IRRIGATED. -ALL LIGHTING ON THE SITE SHALL BE SHIELDED AND NOT ENCROACH UPON ABUTTING PROPERTIES OR RIGHT-OF-WAYS.

PROPERTY INFORMATION

(E)

(B)

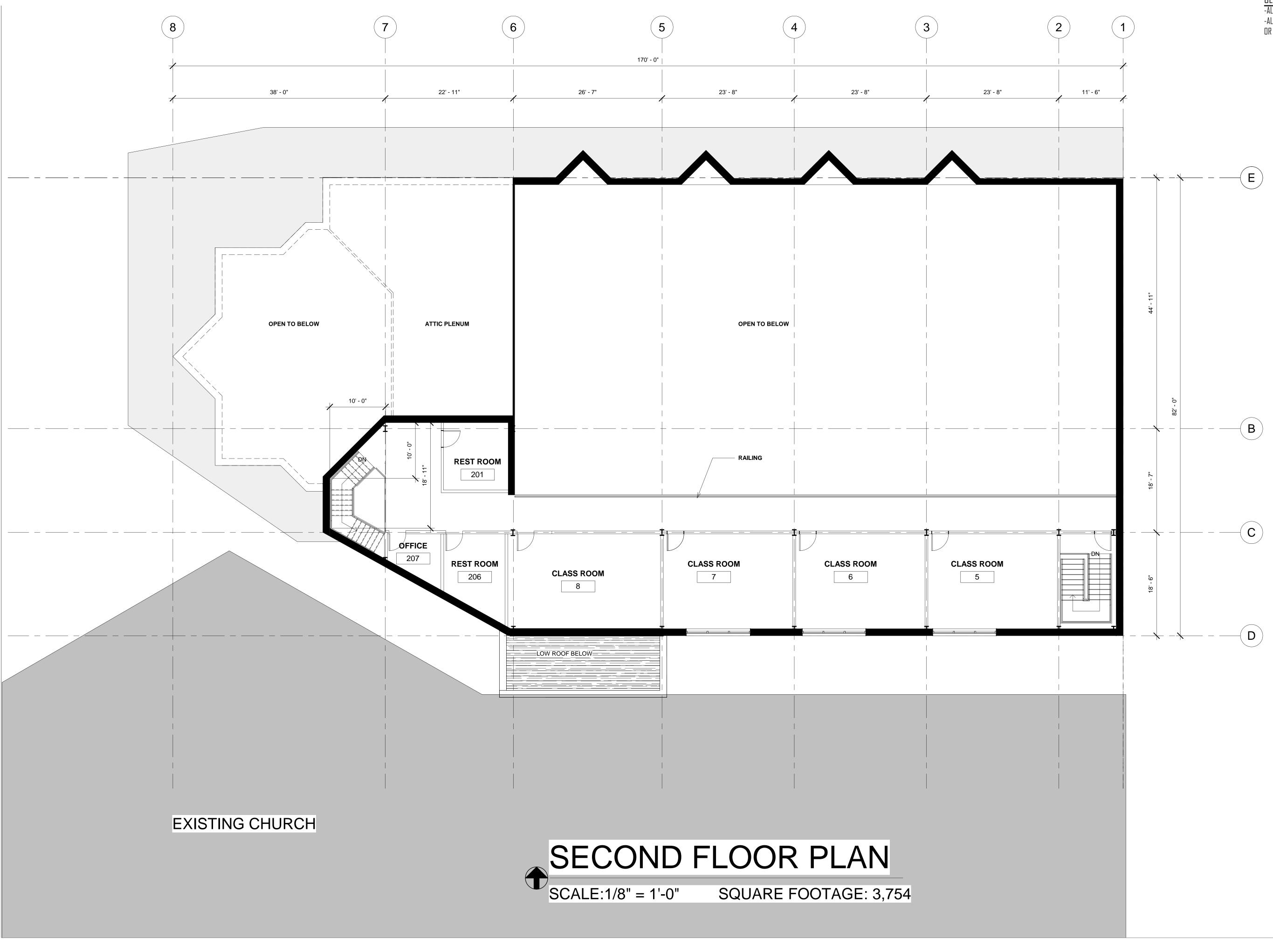
(C)

-(D)

PARCEL NUMBER: 88-20-12-351-037 FRONTAGE: 7.54 ACRES ACREAGE: 7.54 ACRES LEGAL DESCRIPTION: T2N, R11E, SEC 12 EYSTER'S JOHN R ACRES SUB LOT 5 ALSO LOT 15 EXC N 250 FT OF LOT 16 4-13-98 FRD13, 028, 032 TO 034 NEIGHBORHOOD CODE: XCHUR ZONING: R-IC ONE FAMILY RESIDENTIAL

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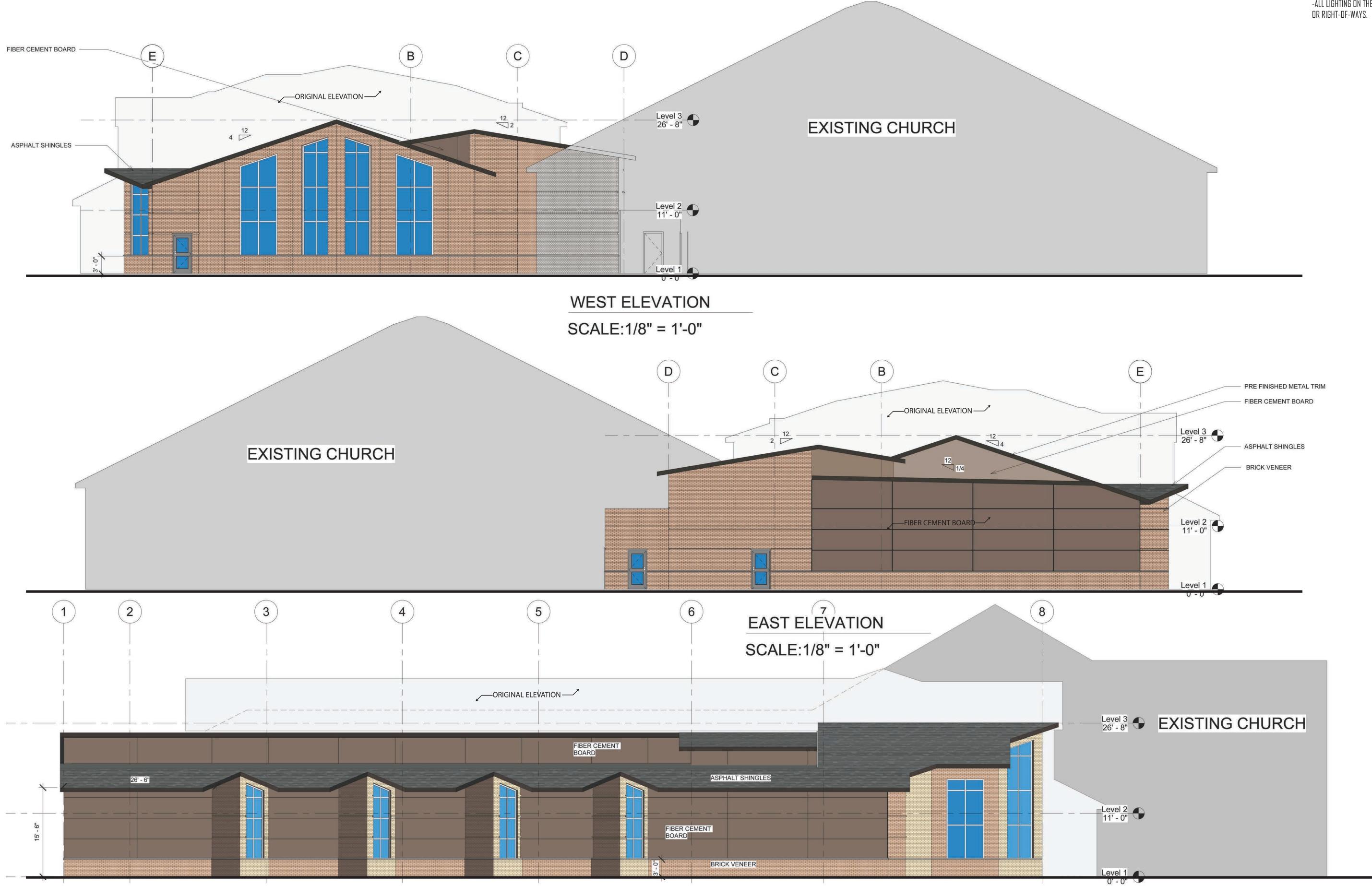




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HURCH REVIEW	2019.07.17		
		SECOND FLOOR PLAN	



NORTH ELEVATION SCALE:1/8" = 1'-0"



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	2019.07.17	ELEVATIONS	

EQUATIONS

TL = SPLs – SPLr + 10Log S/A TL = Transmission Loss

SPLs = The average sound pressure level in the source room SPLr =The average sound pressure level in the receiving room

- S = The surface area of the partition
- A = The absorption, in sabins, in the receive room

$A = S \alpha$

A = Total absorption, Sabins S= Surface Area, square feet or square meters α = Absorption Coefficient

Common Variables

S= 110'-0" x 12'-0" = 1320 sqft. SPLs = 95 dB the sound of a full orchestra

SPLr- 50 dB the average sound of a suburban area

Transmission Loss Through the Wall

 $A = S \alpha$

Using the coefficient of: Gypsum board $\frac{1}{2}$ ", nailed to 2x4's, 15 in. o.c.

Actual Wall:

5/8" Gypsum board, screwed to 2x6's 18 ga. Metal studs, batt insulation, Dens Glass, vapor barrier 2" air gap, Brick veneer. STC value of 56 R-Value 22.2

250Hz

 $A = S \alpha$ =1320 sqft x .08 = 105.6 sabins 500Hz A = S a = 1320 sqft x .05 =66 sabins . 1000- 4000 Hz $A = S \alpha$ = 1320 sqft x .03= 39.6 sabins Best case scenario using 1000-4000 Hz TL = SPLs - SPLr + 10Log S/A = 95dB – 50db +10log 1320sqft/39.6 sabins =45 + 15.23 Loss of 60.23 dB Worst case scenario using 250 Hz TL = SPLs - SPLr + 10Log S/A =95dB – 50 dB + 10log 1320sqft/105.6 sabins =45+10.97 =55.97 dB Inverse Square Law According to the inverse-square law, the intensity ratio for doubling of distance is $2^2 = 4$, and the corresponding decibel reduction is 10 log 4, or 6 dB Starting dB 95dB -55.97dB= 39.03dB 2² = 4 39.03dB -6dB=33.03 4²= 16

4²= 16 33.03dB -6dB =27.03dB

Even coming out of the building, the average background noise of a suburban area would be 50 resulting in not being able to hear the noise coming from the church. At 16'-D" you would not be able to hear the church in a rural area

50'-0"

8

dB

21.97

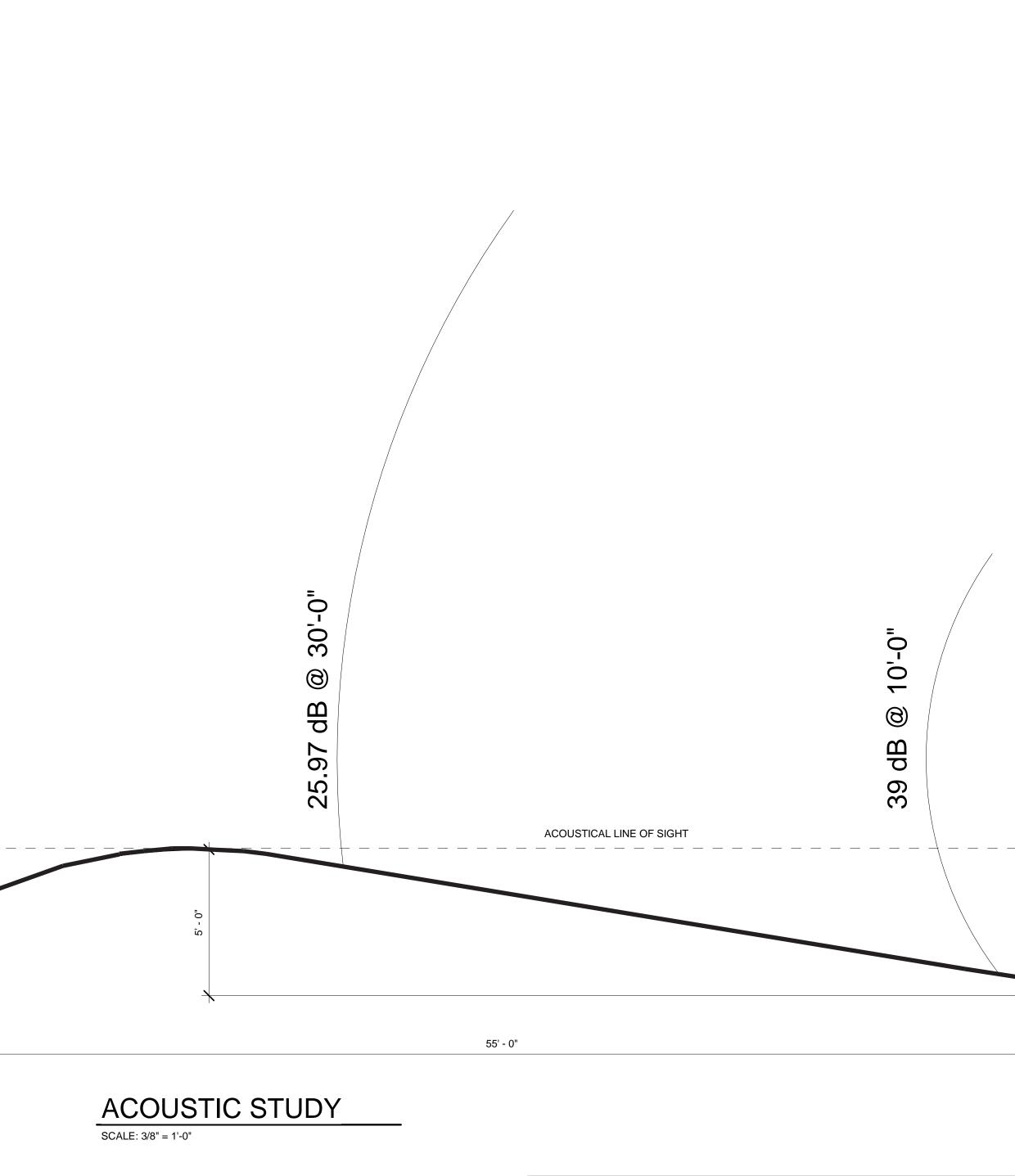
Resources: Source: Mechanical and Electrical Equipment for Buildings.

	APPARENT LOUDNESS DEAFENINING DEAFENINING VERY LOUD VERY LOUD	EXAMPLES JET AIRCRAFT THRESHOLD OF FEELING THUNDER SUBWAY TRAIN	
*	VERY LOUD VERY LOUD VERY LOUD VERY LOUD LOUD	NDISY INDUSTRIAL PLANT BAND LOUD STREET NDISES VACUUM CLEANER AVERAGE STREET NDISE	95 dB 90 dB 80 dB 70 dB
*	LOUD MDDERATE MDDERATE FAINT FAINT FAINT VERY FAINT	AVERAGE OFFICE 2-PERSON CONVERSATION PRIVATE OFFICE BEDROOM NDISE AT SIDEWALK RUSTLING LEAVES NORMAL BREATHING	60 dB 50 dB 40 dB 30 dB 21.97 dB 20 dB 10 dB

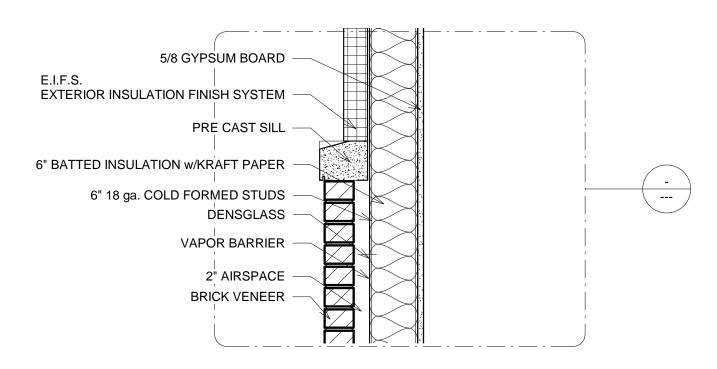
*DEPICTS SOUND FROM ACOUSTIC STUDY

SOURCE: "SOUND CONTROL CONSTRUCTION"

2ND EDITION-PRINCIPALS AND PERFORMANCE BY UNITED STATES GYPSUM







Wall Assembly Analysis Brick

STC = 56 R-Value = 22.2

2″ airspace Vapor Barrier

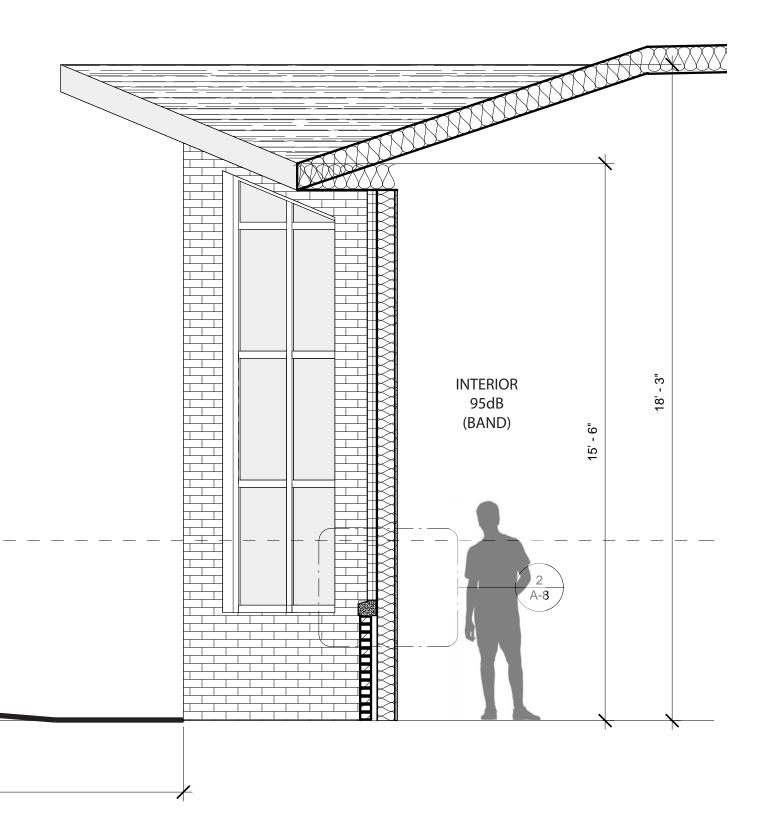
Densglass

6" batted insulation w/Kraft paper

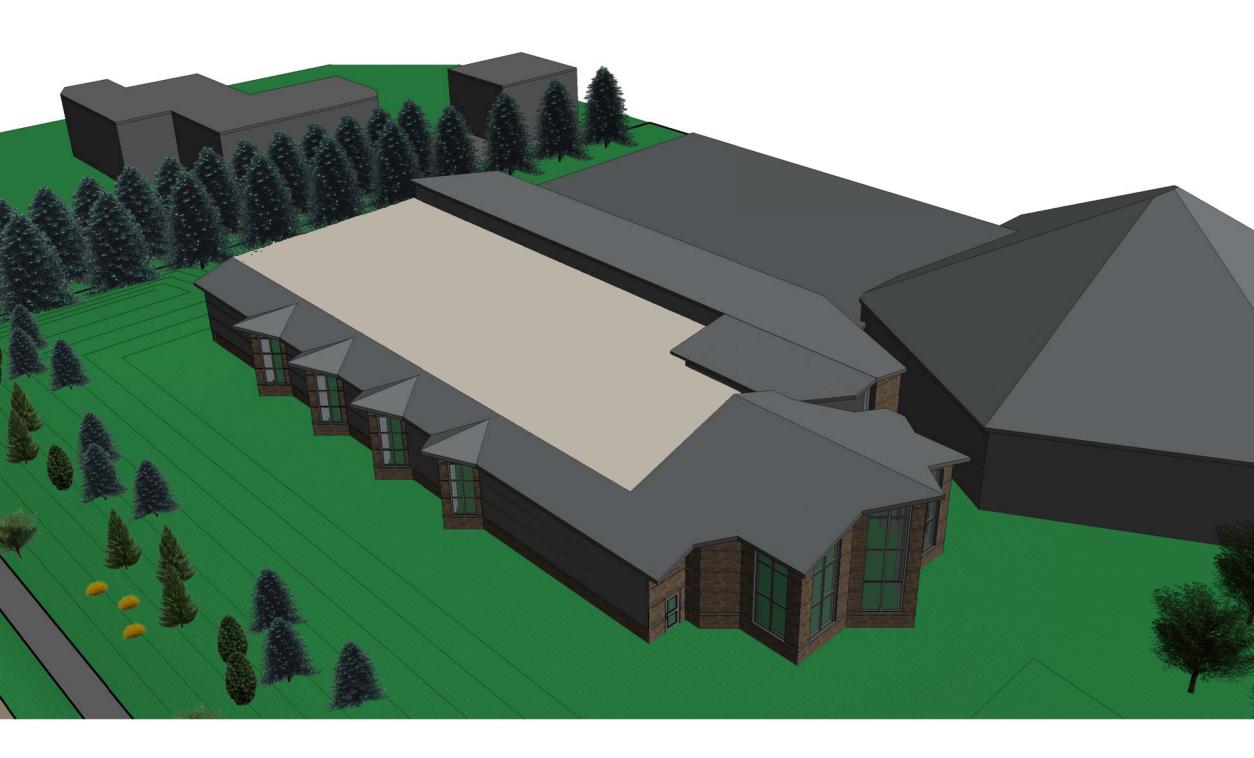
6" 18ga. Cold Form Studs

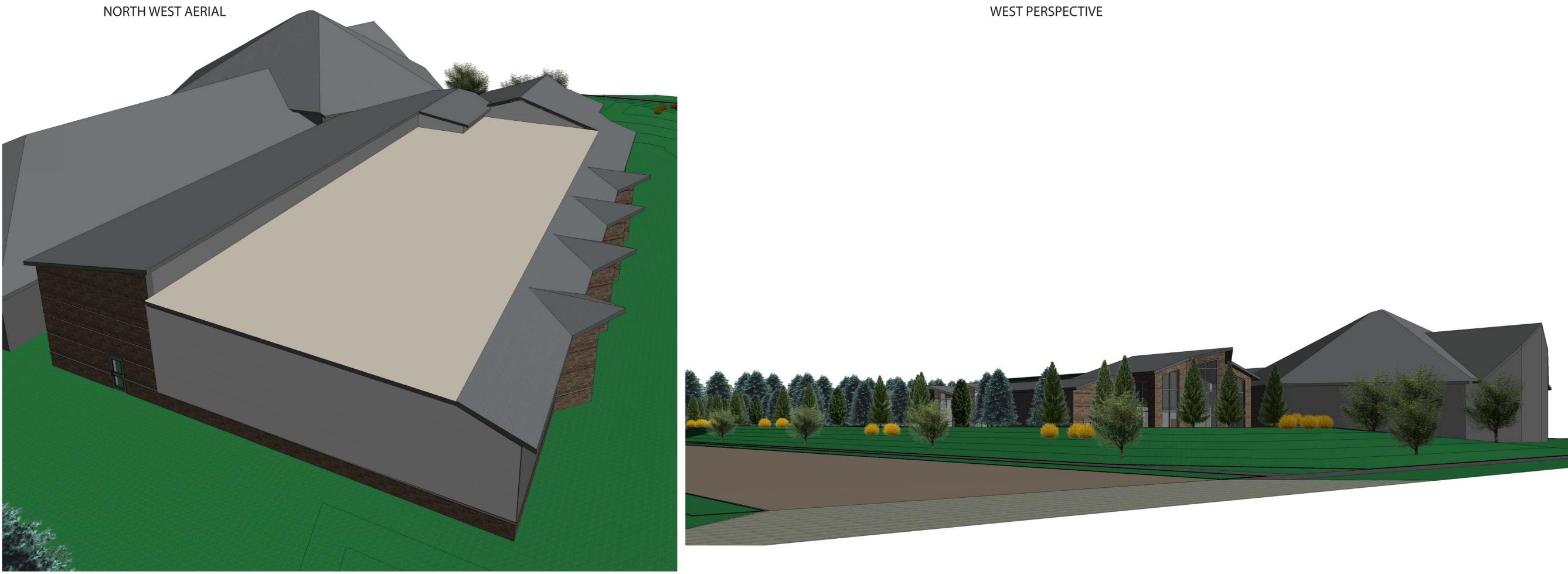
5/8" Drywall Paint

Note: Information based on data provided from Owens Corning, NAIMA, MCAA, Architectrual Acoustics by David Egan



VISIONS	DATE	BETHESDA ROMANIAN PENTECOSTAL	
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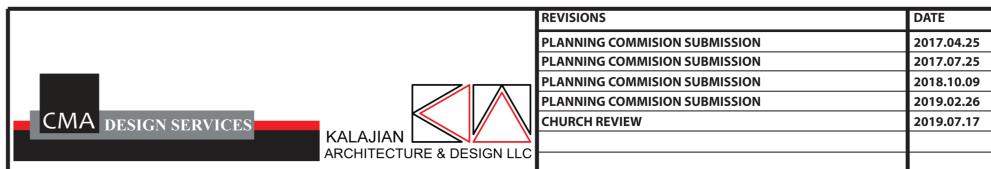


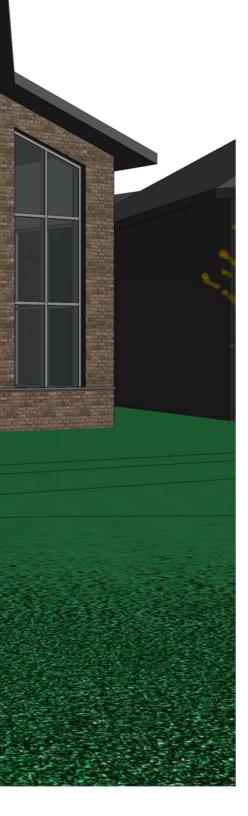


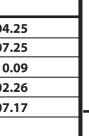
NORTH EAST AERIAL



PERSPECTIVE FROM JON R. & TUCKER

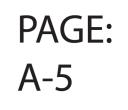






BETHESDA ROMANIAN PENTECOSTAL

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ISOMETRICS



d"series

pecification



Back Box (BBW, ELCW

-1	Catalog
	Number

Notes	
Туре	
	Type

Introduction

The D-Series Wall luminaire is a stylish, fully integrated LED solution for building-mount applications. It features a sleek, modern design

and is carefully engineered to provide long-lasting, energy-efficient lighting with a variety of optical and control options for customized performance.

With an expected service life of over 20 years of nighttime use and up to 74% in energy savings over comparable 250W metal halide luminaires, the D-Series Wall is a reliable, low-maintenance lighting solution that produces sites that are exceptionally illuminated.

DSXW1 LED							
Series	LEDs	Drive Current	Color tem perature	Distribution	Voitage	Mounting	Control Options
DSXW1 LED	10C 10LEDs (ane engine) 20C 20LEDs (two engines)	350 350 mÅ 530 530 mÅ 700 700 mÅ 1000 1000 mÅ (1Å) 1	30K 3000K 40K 4000K 50K 5000K AMBPC Amber phosphor converted	125 Type I Shart 12M Type I Medium 135 Type II Shart 13M Type II Medium 14M Type IV Medium 14M Farward Throw Medium ASYDF Asymmetric diffuse	MW0LT2 1203 2083 2403 2773 34734 48034	Shipped included (blank) Surface mounting bracket BBW Surface- mounted back box (for conduit entry) ¹	Shipped installed PE Photoeleckric cell, button type * DMG 0-10V dimming driver (no controls; wirespulled outside foture)) PIR 180° motion/ambient light sensor; <15° might ** PIR 180° motion/ambient light sensor; <-15° might ** PIRH 180° motion/ambient light sensor; 15-30 might ** PIRH 180° motion/ambient sensor, 8-15° mounting height, ambient sensor enabled at 16° ** PIRHFG3V Motion/ambient sensor, 15-30° mounting height, ambient sensor enabled at 16° ** PIRHTFG3V Group bettery backup (includes external component enclosure), non CEC compilant *
itherOption Shipped inst	< 0.000 MIN	Shipped se par	1	(mpshod) D. Dark bronze	DSSXD	Sandstone	DWHGXD Texared white
		terrent spikes DBLX and DNAX guard DWH	D Natural aluminum	DDBTXD DBLBXD DNATXD	Textured dark bronze Textured black Textured natural alumin	DSSTXD Textured sandstone	
Chebrood an DSXWHSU DSXWB3W U DSXWB3W U DSXWTWG U	cessories notahippant anglaratioly Haze sideshield (one per light engine) Bird-determit spikes Wire guard accessory Vandal guard accesory Vandal guard accesory	2 MVOLT driver 3 Single fuze (S 4 Only available 5 Back box ship 6 Photocontrol 7 Reference M 8 Cold weather voltage optio 9 Not available	operates on any line vol- F) requires 120, 277 or 38 with 200, 700mA or 10 is installed on foture. Cai (FE) requires 120, 208, 2 stion Sensor table on page (200) nated. Not compan- ies. Emergency compone with ELCW.	tible with conduit entry applica	DF) requires . PIRH. e ordered as Notavailable tions. Notava	an accessory. with motion/ambient ligh silable with BBW mountin	
	IDNIA HTING.		• Conyers, Georgia Brands Lighting, Inc. J	30012 • Phone: 800.2793 Ill rights reserved.	8041 • www	w lithe nin.com	DSXW1-LED Rev. 3/13/18

Schedule										
Symbol	Label	QTY	Catalog Number	Description	Lamp	Number Lamps	Lumens per Lamp	LLF	Wattage	Mounting Height
$\langle \Box$	Α	4	DSXW1 LED 10C 530 40K T3M MVOLT	DSXW1 LED WITH (1) 10 LED LIGHT ENGINES, TYPE T3M OPTIC, 4000K, @ 530mA.	LED	1	2159	0.9	19.1	9'-0"

Statistics							
Description	Symbol	Avg	Мах	Min	Max/Min	Avg/Min	Avg/Max
ENTRANCE 1	ж	1.5 fc	3.0 fc	0.3 fc	10.0:1	5.0:1	0.5:1
ENTRANCE 2	Ж	1.4 fc	3.2 fc	0.3 fc	10.7:1	4.7:1	0.4:1
ENTRANCE 3	ж	1.8 fc	3.2 fc	1.1 fc	2.9:1	1.6:1	0.6:1
ENTRANCE 4	Ж	1.5 fc	2.8 fc	0.8 fc	3.5:1	1.9:1	0.5:1

Mounting Height Note

MOUNTING HEIGHT IS MEASURED FROM GRADE TO FACE OF FIXTURE. POLE HEIGHT SHOULD BE CALCULATED AS THE MOUNTING HEIGHT LESS BASE HEIGHT.

Drawing Note

THIS DRAWING WAS GENERATED FROM AN ELECTRONIC IMAGE FOR ESTIMATION PURPOSE ONLY. LAYOUT TO BE VERIFIED IN FIELD BY OTHERS.

General Note

1. SEE SCHEDULE FOR LUMINAIRE MOUNTING HEIGHT. 2. CALCULATIONS ARE SHOWN IN FOOTCANDLES AT: 0' - 0"

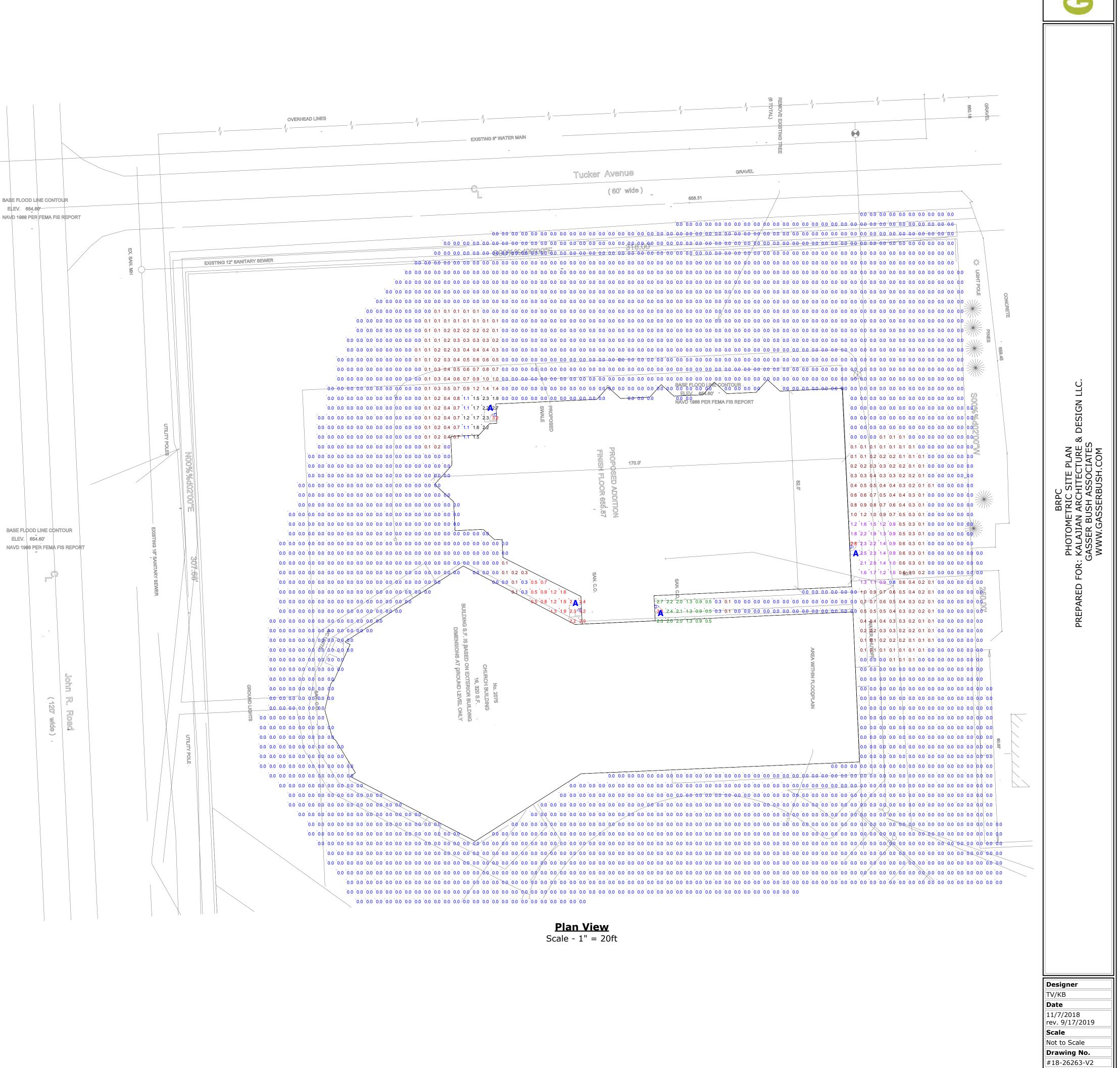
THE ENGINEER AND/OR ARCHITECT MUST DETERMINE APPLICABILITY OF THE LAYOUT TO EXISTING / FUTURE FIELD CONDITIONS. THIS LIGHTING LAYOUT REPRESENTS ILLUMINATION LEVELS CALCULATED FROM LABORATORY DATA TAKEN UNDER CONTROLLED CONDITIONS IN ACCORDANCE WITH ILLUMINATING ENGINEERING SOCIETY APPROVED METHODS. ACTUAL PERFORMANCE OF ANY MANUFACTURER'S LUMINAIRE MAY VARY DUE TO VARIATION IN ELECTRICAL VOLTAGE, TOLERANCE IN LAMPS, AND OTHER VARIABLE FIELD CONDITIONS. MOUNTING HEIGHTS INDICATED ARE FROM GRADE AND/OR FLOOR UP.

THESE LIGHTING CALCULATIONS ARE NOT A SUBSTITUTE FOR INDEPENDENT ENGINEERING ANALYSIS OF LIGHTING SYSTEM SUITABILITY AND SAFETY. THE ENGINEER AND/OR ARCHITECT IS RESPONSIBLE TO REVIEW FOR MICHIGAN ENERGY CODE AND LIGHTING QUALITY COMPLIANCE.

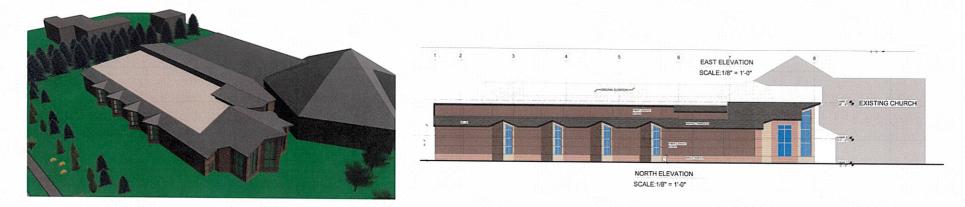
UNLESS EXEMPT, PROJECT MUST COMPLY WITH LIGHTING CONTROLS REQUIRMENTS DEFINED IN ASHRAE 90.1 2013. FOR SPECIFIC INFORMATION CONTACT GBA CONTROLS GROUP AT ASG@GASSERBUSH.COM OR 734-266-6705.

Ordering Note

FOR INQUIRIES CONTACT GASSER BUSH AT QUOTES@GASSERBUSH.COM OR 734-266-6705.



1 of 1

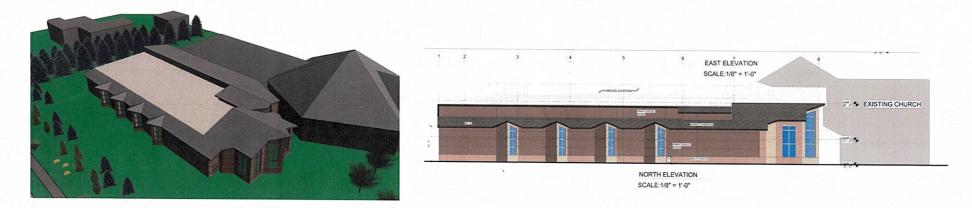


Petition to OPPOSE Bethesda Romanian Pentecostal Church Addition (Dated September 19, 2019) Please sign below if you would OPPOSE this type of developmental in your residential neighborhood

Name	Address	Signature	Troy Home Owner
PRONEER KADALA	2150 Chaps DV	total	Yes
Srinivas Alapati	2118 Chaps dr.	A	Yes
Marcelo Bertocchi	2086 Chaps Dr.		Renting
Matteo DeSano	SO12 JOHN RRd	Also da	Rent
Randall Stevens	3092 John RRd	Randgeer. In	Yes
Ken Chapping	2097 TUCKER	KEN ANORRONI	YRG
Siniter Mishra	2023 Chaps Dr	Sut	omen
Wiktfal and	1124 TUCKER. DR.	MIKE LANHAMSR	YES
Bokai Jin	2043 Tucker Dr	ben	YES
ALEXANDER TABOR	5280 JOHN R.	akjonden Jabon	YES
Damiel Sig L Bei	5041 Forent View	Barr	Yes

Name	Address	Signature	Home Owner
Faroole Salem	2015 Tuckey	Martith Solem	7.65
Julian calen	1014 TOCKEN	Juling solar	Yes
JUDI MILIDRAG	5300 John R	al milie	yes
BRIAN D. MILIDRAG	5300 JOHN R. RD	Balan	YES
MONICA HAUSNER	2071 Theker pr.	Monaca Hansner	Yes
CHRISTOPHER HAUSNER	2071 TUCKER Dr	abut Hour	Yes
leavin Alhaidan	2088 Jucker Dr.	C Z	Xee
Live Hashim	2088 Jucked Dr.	tate	Vee
Thomas Scheyer	2248 Tucker Dr	All	Yes
Tracy Schener	2248 Tucker Dr.	Tracy Schem	Yes
Jean Maikrzek	2322 Tucker Dr.	Yon Mailzek	Web
Rick Churay	2338 Tucker	Robert R Cheng.	Pes
JAMES White	2322 TUEKer	Agroan White	705
Doreen Tash	1325 Jucker	Dorean Mask	Ves
KARAN MARCON	2317 Juchter	if for al	45
VIMEE WATSON	2262 TUCKER Dr	1=hutto	VES
Michael Ve66. an	2155 TUCKEY DI	M Cageon	1725
Brund Veccipi	2119 TUCKEYDr	1 Veggian	205
Jianne: Wu	5111 Forest View Dr. Troy MI 48085	End im	Yes
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Showing Abaharmana	3003 Forent Vire De Troly	YEORS AND	Yes
A & Chemi	5069 Forest view Dr Troy	San dutt	Yes
Yaging How	5055 forese new or Troy	Key AN	tes
Sulva Hermiz	5028 Forest View Dr. Troy	Malary	Yes
PATRICK SMITH	2234 TUCKER	Jell i h/V	Ver
SANDRA ANDREONI	2097 TUCKER	In Graremie	YES
ELAINEM, WOIP	2150 JUCKER	& Oceine m. w. copp	lyes

Petition to OPPOSE Bethesda Romanian Pentecostal Church Addition (Dated September 19, 2019)



Petition to OPPOSE Bethesda Romanian Pentecostal Church Addition (Dated September 19, 2019) Please sign below if you would OPPOSE this type of developmental in your residential neighborhood

Name	Address	Signature	Troy Home Owner
Kathryn Mouton	3431 Totbill Drive, 48084	tothe Und	Yes
Brandon Preblich	4628 Argyle Dr 48085	hey hund	Yes
/ Cichaed Tellenth	4056 Middlebury 4808	5 Indillet	YOS
RiTA Mecoli	5088 Shady Creek 48085	Rota Meroli	Ves
Richard PongM	5088 Shady Crede 48085	RANA	MES
Rachana Malleuroud	2039 Chaps Dr.	Albaurua -	Yes -
Yeelin Tan	2055 Chars Dr	-	Tes
Saudha Ramslingam	2119 chaps dr	Localla	yes
SRÍNÍVAS. TALLURI	2182 chars po	T. Solum	yes
SIVO. Konthounen	2166 Chapson	Kipnismi	yes.

From:	Mary Stockdale
To:	<u>Planning</u>
Subject:	rezoning of #88-20-03-278-027
Date:	Monday, October 7, 2019 2:33:04 PM

Attention to the Planning Commission,

Since we live on De Etta, we want to bring to your attention the difficulty we have at leaving the street. We do have a street light but it seems its for the church's discretion when it works. It is a nightmare to try to turn left off the street, either due to the church having services or when they let out, heavy traffic, or just the oddity of the traffic flow. I'm surprised at the number of cars I have seen STOP at the yellow flashing light. Since we are a dead end street, we are forced to deal with this headache. Now you want to rezone for an office district. We don't know what added grief that's going to add to this congestion but do want you to know that we don't want any outlet onto De Etta.

Thank you

Dennis and Mary Stockdale 964 De Etta

From:	Monica Hausner
To:	Planning
Subject:	Fwd: Please Read - Opposition to Bethesda Development
Date:	Monday, November 11, 2019 3:16:18 PM
Attachments:	image.png
	image.png
	image.png
	Bethesda Church Opposition Petition November 2019.pdf

------ Forwarded message ------From: **Monica Hausner** <<u>mhausner2@gmail.com</u>> Date: Mon, Nov 11, 2019 at 3:08 PM Subject: Please Read - Opposition to Bethesda Development To: Monica Hausner <<u>mhausner2@gmail.com</u>>

Dear Troy Planning Commission,

I am writing to express my opposition to the September 19, 2019 Bethesda Romanian proposed expansion in our residential neighborhood for the following reasons:

1) NOT Compatible with Adjacent Uses

The proposed addition is still <u>NOT constructed in a manner that is</u> <u>harmonious with the character of the adjacent property</u> and the surrounding areas.

• Massive size (15,789 square feet & 170 feet long) compared to the residential homes. Residents could not build this type of structure.

 Very minimal reduction (2,600 sq. ft.) in size from April 2019 submission

• The reduction in classrooms was simply replaced by a chapel

• The fellowship hall is just another name for the gym—it is the same size (109 ft. x 63 ft.)

• Building will directly face the Tucker residents' front door

• **Building looks and feels like a School** - Parishioners emailed me and said that the church wants to open a school at this building in the future

• Low quality materials - "Pre-manufactured" construction using cement board and faux brick

• **Setback Concerns** - The homes on the same side of the proposed addition will have a significant larger setback

2) Noise, Lights and Hours of Operation Concerns

- Residents surrounding the church complained about the noise
- Activities continue until 11pm or later on the weekends
- Residents on Forest View complained about the light pollution and lights on late at night

3) **Long History of Parking/Safety Concerns – How can the church be** allowed to expand?

Recent photos from November 3, 2019



4) NOT Compatible with the Master Plan

• **Loss of green space** – The master plan speaks to preservation of green space.

5) Negative Impact on the Overall Environment & Neighborhood

• The addition will negatively affect the quality and natural features of **Tucker** (i.e. reduction in green space) and significantly change the look and feel of the residential neighborhood.

• Proposed **tree plantings will take 10 years to mature, if they even survive**. The church has a history of over 20 years of not maintaining their property/landscaping (see pictures below).

There has been no maintenance of the trees for the past 20 years, yet the applicant provides beautiful "drawings" of the landscaping. Below are examples of the <u>current landscaping</u>.





6) Concerns with the Church Renting the Facility

The information below is from the Bethesda website: <u>https://www.betezda.com/calendar/</u>

"<u>To reserve any church facilities</u> please email us at: betezdarpc@gmail.com

Please specify room(s) requested: Sanctuary Fellowship Hall Upstairs Room(s) Other

Date and Time

Thank you for reserving in advance, and please allow 24-48 hours for confirmation and approval."

Also, you can see the church already has a "Fellowship Hall" listed. Why is a second Fellowship Hall needed? 7) Residents surrounding the church **OPPOSE** the development as represented by the red stars below. Also, please see the attached petition with <u>47</u> signatures of the area residents opposing the development.



8) **The Planning Commission should align with the newly Elected Mayor and City Council**

Below are the Mayor's and City Council's top goals:

- Ethan Baker "<u>Preserve the character of our neighborhoods</u> by balancing green space with smart, reasonable development"
- Edna Abrahim "Ensure common sense development that <u>preserves</u> <u>our neighborhood's character</u>"
- Theresa Brooks "Work to protect green spaces and respect our residents"
- Ann Erickson Gault "Work to preserve our existing neighborhoods,

support only developments that <u>fit within the character of those</u> <u>neighborhoods</u>"

Thank you for your support.

Kind regards, Monica Hausner 2071 Tucker Dr.

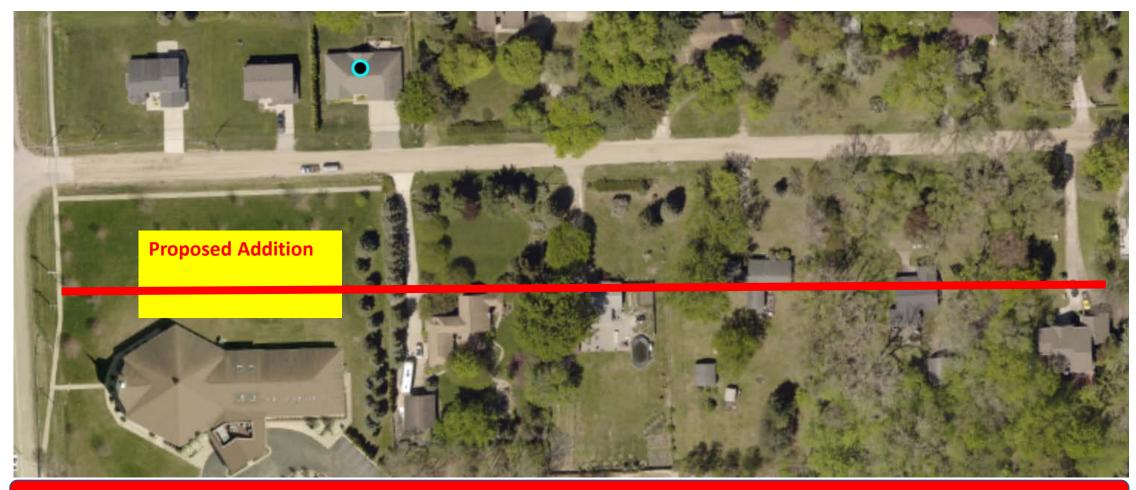
Residents Opposition to Bethesda Proposed Addition

November 26, 2019

Addition is <u>Not Compatible</u> with the Existing Homes & Master Plan

- Massive size (15,789 square feet & 170 feet long) compared to the residential homes. Residents could not build this type of structure.
 - Very minimal reduction (2,600 sq. ft.) in size from April 2019 submission
 - Reduction in classrooms was simply replaced by a chapel
 - Fellowship hall is just another name for the gym—same size (109 ft. x 63 ft.)
- Building looks and feels like a School Parishioners informed me that the church wants to open a school at this building
- Low quality materials "Pre-manufactured" construction using cement board and faux brick
- Significant loss of green space The tenant of the Master Plan is the protection of single-family neighborhoods

Addition Does Not Match the Setback of Existing Homes - <u>Not Harmonious with Neighborhood</u>



BUILDING MASS IS INCONSISTENT WITH THE ADJACENT PROPERTIES

Red stars represent surrounding Residents that signed a petition <u>OPPOSING</u> addition



History of Parking/Safety Concerns Parking in Fire Lanes & Non-Designated Areas





Recent Parking/Safety Concerns

November 3, 2019





November 24, 2019









PARKING IN THE FIRE LANE

Renting Concerns Church already has a Fellowship Hall (Per Website)

Below information is from the church's website:

- To reserve any church facilities please email us at: betezdarpc@gmail.com
- Please specify room(s) requested:
 Sanctuary
 Fellowship Hall
 Upstairs Room(s)
 Other

Date and Time

Thank you for reserving in advance, and please allow 24-48 hours for confirmation and approval.

NO MAINTENANCE ON TREES FOR THE PAST 20 YEARS

Current Tree in Front of Church

New Spring 2019 Planting



Weeds already growing with the new plantings

Concerns with Current Noise, Lights & Hours of Operation

- Residents surrounding the church complained about the current noise late into the night and the light pollution
- On weekends, activities can continue until 11pm or later

Planning Commission Needs to Align with Newly Elected Mayor & City Council's Top Goals

- Ethan Baker "Preserve the character of our neighborhoods by balancing green space with smart, reasonable development"
- Edna Abrahim "Ensure common sense development that preserves our neighborhood's character"
- Theresa Brooks "Work to protect green spaces and respect our residents"
- Ann Erickson Gault "Work to <u>preserve our existing neighborhoods</u>, support only developments that fit within the character of those <u>neighborhoods</u>"

(Source: Troy Times – October 24, 2019 & October 31, 2019)

OPPOSE for the following reasons:

- Building massing is inconsistent with the character of adjacent properties
- A tenet of the Master Plan is the protection of single-family neighborhoods
- Proposed addition is not compatible with the existing homes on Tucker
- Proposed addition does unreasonably impact the quality of the neighborhood on Tucker



2075 E. Long Lake Rd., Troy, MI 48085 248-7507407

November 25, 2019

To the City of Troy, Planning Commission,

Attention: R. Brent Savidant Community Development Director

As members of the Bethesda RP Church, we were taken aback by your presentation of an email that was purported to have been submitted anonymously by someone who claimed to be a member of the church. That letter, although it was submitted anonymously, was presented at the last meeting, on the overhead screen, to create the impression that members of our church do not support the project at issue.

As a result, the church leadership took a brief survey of our membership and confirmed, what we already knew, that the project at issue has overwhelming support from our membership. This support is evidenced by the signatures on the attached petition. More than half of those signatures are from Troy residents who would like to see this project move forward.

We ask that this letter and the attached petition be added to the file and taken into consideration at the public hearing scheduled for November 26th, 2019.

Respectfully submitted.

Bethesda Church Administrative Board

Name	Address (Street, City, Zip Code, State)	Signature
Somiamin BARAIAC	3395 Kilmer MOY mi 48083	Budn'-Berner
ARPAD G. MADO	304 BELJIAVENOV. TROY Mi. 48085	annas a luciolo-
Adina Baraiac	3395 Kilmer TROY MI 48083	aan
Lacramioara Siladni	2027 Waterfall Dr Troy Mi 48083	Frilaphin
Loredana Pop J	3105 Talbot Dr. Troy MI 48083	Anda/19
DAN CIMPAN	16680 AtCREST DR, TROY, Mi, 48083	Domlongan
CORHEL DRÁGOI	1923 Kirktaik den Trey, MI 48083	Ant
ADRIANA DEAGOI	1923 Kirkton Dr. MI 48083	AD 5'
TOHM TRIPORT	1380 MUNERTOIT TROY MI48082	John Sugar
ATTA TRIBON	1880 Milverton dr. TRoy Mi 48083	Ben Stron
PETRU LUPAS	2191 TUCKER de TROY MI 48085	Peter Bepe
CORNERIA LUBAS	2197 TUCKES AT TROY live 48085	C Super
DOINA IANEHIS	2197 BURDIC DR. TRay L1: 48085	Boins your
VICTOR JANCHIS	2197 BURDIC DI. TROY Mi 48085	Fonchis
The Reades	I THE HIBER	
CECILIA VARVARA	73 Forthton dr. TROY Mi 48084	Cecilia Kowan
DANIEL GEORGIU	INT GLASER DR. TROY, Mi 48085	
AMELIA GEORGIU	1117 GLADER DR, TROY, MI 48085	Amelice Georgia
Sherghina Stanciu	111 ×1 Mahan Ave. Hazel Park Mi 48030	Shevyline Stanner
Gabriel Ardelean	AON Butternut Hill Tray Hi a8098	gr

Name	Address (Street, City, Zip Code, State)	Signature
Laura Ardetean	alog Butternut Holl Troy M. A8098	Land
DAN Signani	3027 WATER MAN Proy Mr 68083	A Ceda.
AANIEC TA?	850 Barilane TROY. 171- 48084 C	(loover)
DORINA SILAGH .!	3027 WATERTALL DT., TROY, MIN 8083	the incost
GAVRIL SILAGHI	SO27 WATERALL Dr. Roy M1548083	Goerly
MARIAVA LASC	5690 Houghten Dr. Troy Mi-48098	Chicleiona Lose
LIVIL ARTIC	4059 Gaitesford Circle br M, 48085	Jes f
Luciant Camelin Petertor		the p
DUMITRU MANCI	2391 CASTLETON TROY MI 48083	Alleka
OLIMPIA XADLE	2391 CALLOON TDOG \$14.4803	G D
DANIEL MULRZA	2218 TUCKER TROY/Mi 48085	Tays
LIGH MULZA	2218 Jucker Tray ME 480851	Alit
ELIZABETH MADO	304 BELHAVEN DM TROY Mr. 48085	Elijabeth alloado
PETRY POP	3105 TALBOT AV TROY MI 48083	they total
Elisabeta Pop	3105 TALBOT AR TROY MI 48083	Clisabetta Top
MARIA MORIARY	1781 Castleton DR TRoy Mi 42083	like
Victor MORARY	1781 CoistReton DR TRoy Mi 48083	Affect & p
Mariana Galan	3435 KILMER TROY MI 48083	Judio de
Vasile Galan	3435 KILMER TROY MI 48083	to V
		/

Name	Address (Street, City, Zip Code, State)	Signature
GABRIEL IOACHIMCIUE	1110 GLASER DRIVE, TROY 48085	Joch
JOHD SERBAN	2727 CHESTERFIELDER TROV	FREM
LIDIA MARC	1807 NORTH LAKE DR, TROY F	Schare
DOREL MARC	1807 NORTH LAKE DR TROY, MI	store
ADICION FARIENS	r c t	
Yuto Fakashimu	2414 John Read Troy MI	12
Stefania Craseuc	41561 AEQUIXIBRE RS TROY 48085	Chuelles.
IDAN + Tripon	1880 Millionly Dr Ting 48083 (Ange to
Claudin Dumitrescu	6352 Solomon Ave. TROY. Mi 48085	· Ander '
FLORIN FILIMON	2840 CLAYTON DO TROY ME 48083	Pla
GABRIEL ROS	-100-BL	
MARCELA MUSCAS	1662 MILVERTON TROJ MI 48083	
NICOLAE MUSCAS	1662 MiLVERTON TROJ MI 48083	Nicolae Herras
Teader Schudb	2743 Dover Dr. Troy ULL'4POR3	French
Silvia Such	2743 DOUDY DETROY MY 4POP3	Awala
Simina Tyiran	3465 Femleigh Dr. DoymME 48083	A
Daniel P. Tyiran	3465 Fernleigh Dr., Troy, MI 48083	Dalti
ROMED PELLE	2257 GOLFVIEW DR, APT. 201 TROY MI 48084	Romen felle
RODICA PELLE	2257 Gell View, APT- 201 TROY MI 48084	Podice felle
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Name	Address (Street, City, Zip Code, State)	Signațure	
TEOFIL BARJUCA	2240 Custer Dr. Troy, M1 48085	Sug 1 2	
MARIA COBMA	GLGE BROCKTON AVE "HILDBOT!	An	
GHEORGHE ROJU	LO LO GABRIEL AR TROY MI 48083	Douce	
Renei Drasovean	953 Mucr Str. Troy Hi 42284	Laid	.^ .
CRAINICE ANU DARIUS	2239 LONGFELLOW AVE ON; N93377	1 to a second se	HEET
Cheorghe Ardeleau		The produces	
Cezica Gradiany	2315 Cumberland Dr. Troy MI4808		
Dorin. Grodligues	2315 Cumberland Dr. TROG. MIN. 48085	- longue los	
Florica Ardeleon	2751 Confinental & Troy M 4808:	Talefier	
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Irene Ianos	809 Hartland Dr Troy MI 48083		
Cornelia Rasco/	793 Hartland Dr Troy MI 48083	C. Ravel	
Nicolai Rascol	793 Harfland Dr Troy MI 48083	N. Karreron	
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Julien Cobut	2350 E. NEWARK RD LAPPER Mi	D-Mahuto
Michede Cohof	2350 E. NEWARK R. LAPEBR ME 48446	What see
Gabriela Cohut	12130 Jode Point, Sterling Hats Mi 48312	Cohut
Nick Cohut	12130 Jode Point, stertina Hats, Mi 48312	duce
Daniel Marcy	2812 Hartline, Rochester Hus 48209	Auc
Ruxanda Peret	2962 Wallmidge Rd Roduester And GRO	2 Minut
OUMPIA CORNEANU	3450 Brookland St. Shaby Tup. M. 48317	
Daniela Tecar	3926 Oak Knoll Rd. Waterford Mi 48328	Agny
GEORGE STRAGOS	14514 ASHTON DR. SHELRY YE315	Torage
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Esther Chirodea	1801 N Boohester Rd	
DANIEL CHIRODEN	1801 N ROCHESTER RA	
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Priscilla Chrita	620 E_ Rowland Ave Madison-Hight 3007 Belinda Dr Steeling Heights	Parton
Jonathan Tida	3007 Belinela Dr Sterling Heights	

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Sanda Panfis	6556 Petito et Washington Mi	Sanda Pent
Lucian Pantis	6556 Petilo et Washington Mi	Lucian Pantis
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Andre Petreca	1700 S Coats a dx Edd mi 49371	Andrei Planes
PAVEL PETREA	1700 SCOATSRD AXFORD MI 48371	F. cuil
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Anca Balint	1830 HCKail Rd Leonard 41 48367	Anas-
Ida Miletic-	Madison Heght 301101 MI. 48967	Sm
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GABY BUNA	Sterling HS G8310	Gourf Frees
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Robert Ianchis	Dail Dai Dai Candria Mi 18 xe 3	J Balen
Ramona Manya		Robert US
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Claudia Lypus	3550 E Clarkston Rd Oahland 482	
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DATE: December 6, 2019

- TO: Planning Commission
- FROM: R. Brent Savidant, Community Development Director
- SUBJECT: <u>PUBLIC HEARING SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (SP</u> <u>JPLN2019-0037</u>) – Proposed Bostick 801, LLC Redevelopment, East side of Crooks, south of Big Beaver (801 W. Big Beaver), Section 28, Currently Zoned BB (Big Beaver Road) District

The petitioner Bostick 801, LLC submitted the above referenced Special Use and Preliminary Site Plan application for the proposed mixed use project including a 6-story, 140 room Hyatt Place Hotel, 232-seat (8,538 square feet) Ford's Garage restaurant and 5-level parking deck. The 4.22-acre site is currently zoned BB.

The attached report prepared by Carlisle/Wortman Associates, Inc. (CWA), the City's Planning Consultant, summarizes the project. CWA prepared the report with input from various City departments including Planning, Engineering, Public Works and Fire. City Management supports the findings of fact contained in the report and the recommendations included therein.

Attachments:

- 1. Maps
- 2. Report prepared by Carlisle/Wortman Associates, Inc.
- 3. Traffic Impact Assessment, prepared by ROWE PSC, dated September 24, 2019.
- 4. Shared Parking Analysis, prepared by ROWE PSC, dated October 14, 2019.
- 5. Memorandum prepared by OHM, dated October 23, 2019.
- 6. Email from Ron Wilson (with attachments).

G:\SPECIAL USE\SU JPLN2019-0037 (SP) BOSTICK 801 LLC REDEVELOPMENT\PC Memo 2019 12 10.docx

PROPOSED RESOLUTION

<u>PUBLIC HEARING – SPECIAL USE AND PRELIMINARY SITE PLAN REVIEW (SP</u> <u>JPLN2019-0037)</u> – Proposed Bostick 801, LLC Redevelopment, East side of Crooks, south of Big Beaver (801 W. Big Beaver), Section 28, Currently Zoned BB (Big Beaver Road) District

Resolution # PC-2019-12-

Moved by: Seconded by:

RESOLVED, That Special Use Approval and Preliminary Site Plan Approval, pursuant to Articles 8 and 9 of the Zoning Ordinance, as requested for the proposed mixed use Bostick 801, LLC Redevelopment, including hotel, restaurant and parking structure, located on the east side of Crooks, south of Big Beaver (801 W. Big Beaver), Section 28, Currently Zoned BB (Big Beaver) District, be granted, subject to applicant the following:

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) or
(denied, for the following reasons:) or
(postponed, for the following reasons:)

Yes: No:

MOTION CARRIED/FAILED

G:\SPECIAL USE\SU JPLN2019-0008 COURTYARD HOTEL\Proposed PC Resolution 11 12 2019.docc



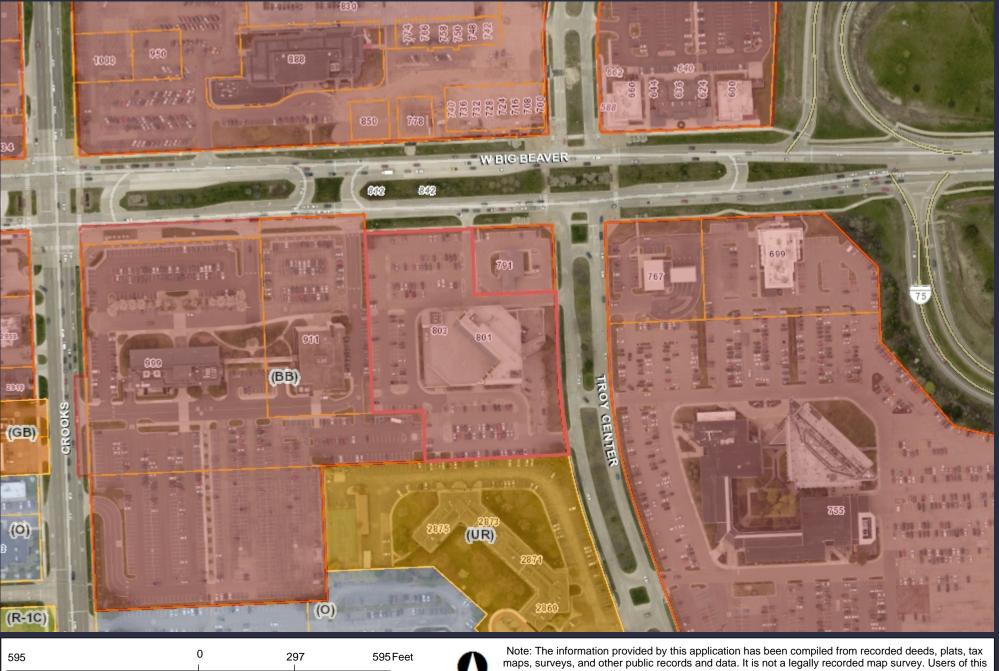
GIS Online



Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.



GIS Online



data are hereby notified that the source information represented should be consulted for verification.



117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

Date: December 5, 2019

Special Use and Preliminary Site Plan Review For City of Troy, Michigan

Applicant:	801 Bostic LLC
Project Name:	Hyatt Place-Big Beaver
Plan Date:	October 14, 2019
Location:	Big Beaver
Zoning:	BB, Big Beaver Form-Based District
Action Requested:	Preliminary Site Plan and Special Use Approval

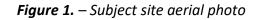
PROJECT AND SITE DESCRIPTION

801 Big Beaver is currently improved with a 5-story office building (Huntington Bank building). The applicant is proposing the construction of the following additional buildings on the site:

- 6-story, 72-foot tall, 140 room Hyatt Place Hotel
- 232-seat, 8,538 sq/ft restaurant
- 5-level, 361-space parking structure

The 361-space parking structure will serve all uses on site. The hotel and restaurant will be connected. The hotel/restaurant will be located on the existing surface parking area just west of the existing office building. The parking structure will be located on the existing surface parking area just south of the existing office building. The parking structure and restaurant are permitted uses in the BB, Big Beaver form-base district. The hotel is a special use in the BB, Big Beaver form-base district.

Hyatt Place Hotel SU and SPR December 5, 2019





Size of subject property: 4.22 acres

Current use of subject property:

Office (bank with drive-through on corner is not part of this site)

Proposed use of subject site:

- 6-story, 72-foot, 140 room Hyatt Place Hotel
- 232-seat, 8,538 sq/ft restaurant
- 5-level, 361-space parking structure

Zoning:

The property is zoned BB, Big Beaver form-base district

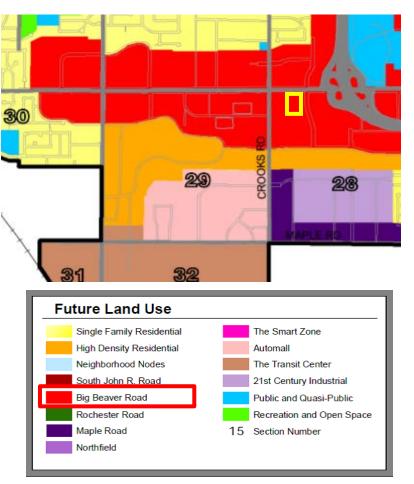
Surrounding Property Details:

Direction	Zoning	Use		
North	BB, Big Beaver	Retail and Hotel		
South	UR, Urban Residential	Multiple Family Residential		
East	BB, Big Beaver	Office		
West	BB, Big Beaver	Office		

MASTER PLAN

The site is located within the area designated as Big Beaver in the Master The Big Beaver designation Plan. responds to the recommendations set forth in the Big Beaver Corridor Study, which promotes flexibility with land use relationships including higher density, vertically integrated mixed-use commercial, office, and residential towers. The Big Beaver Corridor Study and Master Plan promote redevelopment with a greater mix of land uses, particularly new residencies, but also encourages the use of prominent ground floor retail, restaurants, and cafes allowing visual interest and activity for visitors and residents.

Additional hotel uses along and peripheral to Big Beaver promote the goals of the Big Beaver Corridor Study and Master Plan including transforming Big Beaver into a destination or "people



place" characterized by round-the-clock activity and an exciting nightlife, promote redevelopment opportunities along the corridor, maintain and improve existing businesses along Big Beaver, and transform the corridor into a pedestrian-friendly environment.

Specifically, page 48 of the Big Beaver Corridor Study calls this site as a mixed use; retail/office/residential use. Adding a compatible and vibrant mix of uses including structed parking to the existing mixed-use fabric is consistent with the Master Plan.

NATURAL RESOURCES

Woodlands: The applicant has identified a total of 38 regulated trees on the site. Of the trees surveyed, 3 are a landmark tree and 35 are woodland trees. The applicant is preserving 24 of the 38 regulated trees.

Replacement Details				
Protected Tree	Inches Removed	Replacement Required		
Landmark	32 inches	32 inches		
Woodland	108 inches	54 inches		
Preservation/Mitigation	Inches Preserved	Credit		
Landmark	16 inches	32 inches		
Woodland	226 inches	452 inches		
Protected Replacement Required	86 Inches			
Preservation Credit	484 Inches			
Total	+ 398-inch credit			

Items to be Addressed: None

BUILDING LOCATION AND SITE ARRANGEMENT

The proposed restaurant/hotel building is situated adjacent to Big Beaver. The restaurant use of the building will be the portion that fronts on Big Beaver. A majority of the first floor of the hotel is at-grade parking. The remaining portion first floor portion is lobby and storage.

The 5-level, 359-space parking structure is located on the rear portion of the site along the southern property line. Access to the site will provided with one curb cut on Big Beaver Road and two curb cuts on Troy Center Drive. The site includes pedestrian connections between the parking structure and all other uses on site.

Items to be Addressed: None

AREA, WIDTH, HEIGHT, SETBACKS

BB, Big Beaver form-base district bulk requirements are set forth in Article 5.

Table 1. – Site requirements and proposed dimensions

	Required / Allowed	Provided	Compliance		
Front (Big Beaver)	10-foot build-to-line, up to 30-feet with PC approval	11.9 to 21.3 feet	Complies with PC approval		
Front (Troy Center Drive)	10-foot build-to-line, up to 30-feet with PC approval	10.7 10 18.5 feet	Complies with PC approval		
Side	0 foot	6.9 feet	Complies		
Rear	40 foot minimum	45 feet	Complies		
Required Open Space	15 percent	24 percent	Complies		
Building Height	Unlimited	6-story, 72-foot	Needs Special Use Approval		
Parking (Big Beaver)	Not located in front yard and screened	Existing parking	Existing parking		
Parking (Troy)	Not located in front yard and screened	Existing parking	Existing parking		

Items to be Addressed: None

PARKING

Parking Calculations:

The applicant has completed a parking study for the site. Uses on site used to determine parking calculations include:

- Existing 122,386 sq/ft office (77,874 leasable)
- New 140 room hotel
- New 232 seat restaurant

The parking study notes that shared parking for this site is appropriate due to the varied proposed uses. The study concludes that the overall peak demand on site is 2 p.m on a weekday, which requires 449 parking spaces. The applicant has indicated plans that show 359 spaces and 361 spaces. The applicant should clarify the number of spaces in the parking structure. The site is served with 359/361 deck parking spaces and 106 at grade spaces, which total 465/467 spaces total on site.

Use	Weekday	Weekend
	(peak)	(peak)
Restaurant	76	99
Hotel	114	154
Office	259	0
Total	449	253

The City's engineering consultant OHM has reviewed the parking study and does not recommend approval. They note two items:

- 1. The analysis method must be revised. While Troy allows for the use of ULI / ITE methodologies for shared parking, the actual parking rates used MUST reflect city zoning ordinance required rates.
- 2. The analysis states an assumption that the 2,949 SF of conference space will be used by non-guests during the day and by hotel guests at night. The peak period for weekday parking is 2pm which is a time when the conference facilities could well be used based on this assumption. OHM believes that the conference facilities be considered in the parking analysis.

Items to be Addressed: 1). Clarify number of parking spaces in structure; and 2). Address OHM's parking study review

TRAFFIC

The applicant completed a traffic impact assessment (TIA) for the proposed 140 room hotel and 232-seat restaurant. The traffic study only accounts for traffic increase due to the hotel and restaurant. It does not account for existing traffic of the existing office building.

The traffic study concludes that the hotel and restaurant will add 71 cars during the AM peak hour and 138 cars during the PM peak hour:

	AM Peak Hour		PN	/I Peak Ho	Week Day		
	In	Out	Total	In	Out	Total	
Restaurant	3	3	6	40	19	59	633
Hotel	38	27	65	40	39	79	1,154
Total Trips	41	30	71	80	58	138	1,787

The trip generation will be distributed as follows:

<u>AM Peak Hour</u> 20% from and 34% to the south 5% from and 63% to the east 75% from and 3% to the west <u>PM Peak Hour</u> 26% from and 26% to the south 12% from and 66% to the east 62% from and 8% to the west

The traffic study concludes that no additional traffic improvements are necessary.

The City's engineering consultant OHM has reviewed the parking study and does not recommend approval. They note four items:

- 1. The architectural drawing attached to the TIA and showing the parking calculations differs from the preliminary site plan and the provided parking.
- 2. The figures showing the site generated traffic volumes appear to be incomplete. There is no traffic shown going through the I-75 interchange.
- 3. The background traffic shows a total for the site driveways of 19 during the AM peak, with 17 entering and 2 exiting. During the PM peak the total is 51, with 17 entering and 34 exiting. According to ITE trip generation calculations the existing office building on site would be expected to generate approximately 76 entering and 13 exiting for a total of 89 during the AM peak. The total is 89 during the PM peak, with 14 exiting and 75 exiting. OHM questions what the building occupancy was at the time of the counts. If substantially unoccupied, the background traffic should be adjusted to reflect the trip generation of a reasonably occupied site.
- 4. There is a large disparity in the collected turning volumes at Troy Center Drive and Big Beaver when compared to the MDOT study. The counts in 2017 were more than double the counts collected in 2019. These recent counts were likely impacted by Big Beaver and I-75 construction and should not be relied on. The study should instead use the 2017 counts taken for Big Beaver at Town Center and at the crossovers flanking this intersection. Regarding the site driveway volumes, reference the concern noted above.

Items to be Addressed: Address OHM's traffic study review

ACCESS AND CIRCULATION

Vehicular access:

The site is currently served with three curb cuts; one on Big Beaver Road and two curb cuts on Troy Center Drive. The only change to access is that the applicant is altering the location of one of the curb cuts on Troy Center Drive.

The City's engineering consultant OHM has reviewed the site plan for circulation and has made noted recommendations.

Pedestrian access:

The site includes direct pedestrian connection from Big Beaver and internal pedestrian connections including between the parking structure and all other uses on site. The applicant is providing a brick-paver walkway from the structure to the hotel. The brick-paver walkway should continue across the hotel's front to the restaurant entrance.

Items to be Addressed: 1). Address OHM's circulation review; and 2). Continue brick-paver across the hotel's front to the restaurant entrance.

LIGHTING

The applicant has provided a lighting (photometric) plan and lighting fixture details. The applicant is proposing a four (4) pole lights, six (6) bollard lights, twenty-nine (29) parking structure down lighting, and thirty-four (34) building lights. It appears that the lighting levels slightly exceed the maximum levels along the western property line.

The photometric plan did not indicate any lighting of the parking structure. It is presumed that the garage will be lit, even if just internally. Lighting should be shown to determine light impact, particularly on the upper levels as they will be seen from the adjacent apartment building to the south.

Items to be Addressed: 1). *Reduce lighting levels along western property line; and 2). Indicate lighting and photometrics for the parking structure.*

LANDSCAPING

The application includes a landscape plan and calculations.

	<u>Required:</u>	Provided:	Compliance:
Street Trees: The Ordinance requires that	Big Beaver Road	Big Beaver: 8 trees	Compliant but see
the greenbelt shall be landscaped with a	243 LF = 8 trees		note below
minimum of one (1) deciduous tree for		Troy Center: 12	
every thirty (30) lineal feet, or fraction	Troy Center Drive	trees	
thereof, of frontage abutting a public road	372 LF = 12 trees		
right-of-way.			
Site landscaping: A minimum of twenty	15%	12.5% landscaping.	Appears complaint
percent (15%) of the site area shall be		Hardscape	with hardscape
comprised of hardscape and landscape		percentage not	percentage
material.		provided	provided.
Parking Lot Landscaping: 1 tree for every	82 spaces =	10 trees	Compliant
8 parking spaces. Trees may be located	10 trees		
adjacent to parking lot with planning			
commission approval.			

While the applicant is compliant with landscaping along Troy Center Drive, the applicant is not proposing any landscaping between the 5-story parking structure and Troy Center Drive. The applicant should provide some landscaping to soften the front façade of the parking structure from Troy Center Drive.

Items to be Addressed: 1). Provide hardscape calculation; and 2). Provide landscaping to soften the front façade of the parking structure from Troy Center Drive.

FLOOR PLANS AND ELEVATIONS

The applicant has submitted floor plans and elevations. Materials include a mix of "wall plank" panels, and fiber cement board. As set forth in Section 8.06.B. Development shall incorporate the following recognized best architectural building design practices:

- 1) Foster a lasting impact on the community through the provision of high-quality design, construction, and detailing.
- 2) Provide high quality, durable materials, such as but not limited to stone, brick, glass, and metal. E.I.F.S. or material equivalent shall only be used as an accent material.
- 3) Develop buildings with creativity that includes balanced compositions and forms.
- 4) Design roofs that are appropriate to the architectural style of the building and create an appropriate visual exterior mass of the building given the context of the site.
- 5) For commercial buildings, incorporate clearly defined, highly visible customer entrances using features such as canopies, porticos, arcades, arches, wing walls, ground plane elements, and/or landscape planters.
- 6) Include community amenities that add value to the development such as patio/ seating areas, water features, artwork or sculpture, clock towers, pedestrian plazas with park benches or other features located in areas accessible to the public.

<u>Hotel</u>: The primary building material is brick with metal panels on the upper floors. Colors appears to be a mixture of browns, tans, greys, and white.

<u>Restaurant:</u> The primary building material is brick with metal panels on the second floor. The brick and metal material and color match the hotel.

<u>Parking Structure:</u> The primary material is concrete with brick accents and other architectural details. All four sides of the structure will be highly visible including the southern elevation from the apartment site.

The existing 5-story office building on site is highlighted with white paneling. The applicant's architect should describe how the hotel/restaurant and parking structure materials and architecture complement the existing office building and any other surrounding buildings.

The applicant has been asked to bring to the meeting the building materials samples, a color rendering, and a 3-D model to evaluate the overall architecture program. The Planning Commission should discuss the materials and elevations based upon the additional information provided at the meeting.

Items to be Addressed: Make any changes to building material use and elevations based upon direction from the Planning Commission.

STANDARDS FOR APPROVAL

Hotels are permitted subject to Special Use approval. For any Special Use, according to Section 9.02.D, the Planning Commission shall "…review the request, supplementary materials either in support or opposition thereto, as well as the Planning Department's report, at a Public Hearing established for that purpose, and shall either grant or deny the request, table action on the request, or grant the request subject to specific conditions."

Section 9.03 states that before approving any requests for Special Use approval, the Planning Commission shall consider:

- 1. Compatibility with Adjacent Uses. The Special Use shall be designed and constructed in a manner harmonious with the character of adjacent property and the surrounding area. In determining whether a Special Use will be harmonious and not create a significant detrimental impact, as compared to the impacts of permitted uses.
- 2. Compatibility with the Master Plan. The proposed Special Use shall be compatible and in accordance with the goals and objectives of the City of Troy Master Plan and any associated sub-area and corridor plans.
- 3. Traffic Impact. The proposed Special Use shall be located and designed in a manner which will minimize the impact of traffic, taking into consideration: pedestrian access and safety; vehicle trip generation (i.e. volumes); types of traffic, access location, and design, circulation and parking design; street and bridge capacity and, traffic operations at nearby intersections and access points. Efforts shall be made to ensure that multiple transportation modes are safely and effectively accommodated in an effort to provide alternate modes of access and alleviate vehicular traffic congestion.
- 4. Impact on Public Services. The proposed Special Use shall be adequately served by essential public facilities and services, such as: streets, pedestrian or bicycle facilities, police and fire protection, drainage systems, refuse disposal, water and sewage facilities, and schools. Such services shall be provided and accommodated without an unreasonable public burden.
- 5. Compliance with Zoning Ordinance Standards. The proposed Special Use shall be designed, constructed, operated and maintained to meet the stated intent of the zoning districts and shall comply with all applicable ordinance standards.
- 6. Impact on the Overall Environment. The proposed Special Use shall not unreasonably impact the quality of natural features and the environment in comparison to the impacts associated with typical permitted uses.
- 7. Special Use Approval Specific Requirements. The general standards and requirements of this Section are basic to all uses authorized by Special Use Approval. The specific and detailed

requirements relating to particular uses and area requirements must be also satisfied for those uses.

The Planning Commission desires to review each lodging use on a case-by-case basis to ensure the architectural and material quality is consistent with the stated intention of the Master Plan and Zoning Ordinance; that such use does not oversaturate the market especially considering the difficulty in retrofitting hotels for future uses; and ensure that a hotel development does potentially negatively impact adjacent properties.

The applicant should describe the hotel market conditions that may impact the hotel to ensure that they market is not over saturated.

Provided that the applicant can prove to the satisfaction of the Planning Commission that the hotel market is not over saturated, conceptually we find the Special Use Standards. However as noted in OHM's review there are outstanding issues with regards to parking and traffic that must be addressed prior to a final recommendation.

RECOMMENDATION

Overall we support the development of this site and find it to be a significant investment on Big Beaver. However, there are some noted site plan issues, specifically with regards to parking and traffic, that must be addressed prior to approval. We recommend the Planning Commission hold a public hearing but postpone action to allow the applicant to address noted items.

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP



Large Firm Resources. Personal Attention. "

Memorandum

To:	David Hunter, PE, PS – PEA, Inc.
From:	Michael J. Labadie, PE and Jill M. Bauer, PE, PTOE
Date:	September 24, 2019
RE:	Traffic Impact Assessment for Hotel and Restaurant Addition to Big Beaver Business Center

ROWE Professional Services Company has completed a traffic impact assessment (TIA) related to the proposed hotel and quality restaurant in the existing Big Beaver Business Center at 801/803 Big Beaver Road. This development is located in the southwest quadrant of the Big Beaver Road/Troy Center Drive intersection in Troy, MI. The current site plan (included in the materials attached to this report) indicates a 140-room hotel and a 7,550-square-foot (SF) quality restaurant. This TIA is intended to determine if any improvements would be necessary to mitigate traffic impacts related to the proposed development on the adjacent road network. It has been completed in accordance with the requirements specified by the city's engineering consultant. For the analysis, traffic models were provided by the Michigan Department of Transportation (MDOT) that represented both existing conditions and the new interchange layout that will be built at Big Beaver Road and I-75. These traffic models were revised to account for the addition of the hotel and restaurant. In the MDOT study, all traffic volumes were forecasted by MDOT for the year 2040.

TRAFFIC IMPACT ASSESSMENT

Traffic Counts

Turning movement counts were collected by MDOT at the following intersections along Big Beaver Road:

- Big Beaver Road and westbound to eastbound crossover (west of Crooks)
- Big Beaver Road and Crooks Road
- Big Beaver Road and eastbound to westbound crossover (east of Crooks)
- Big Beaver Road and westbound to eastbound crossover (west of Troy Center Drive)
- Big Beaver Road and Troy Center Drive
- Big Beaver Road and Wilshire Drive
- Big Beaver Road and eastbound to westbound crossover (east of Wilshire Drive)
- Big Beaver Road and southbound I-75 ramps
- Big Beaver Road and northbound I-75 ramps
- Big Beaver Road and westbound to eastbound crossover (west of Livernois Road)
- Big Beaver Road and Livernois Road
- Big Beaver Road and eastbound to westbound crossover (east of Livernois Road)

MDOT then used this data to project 2040 traffic volumes. MDOT provided the Synchro Models and 2040 traffic projections from the I-75 Modernization Project for use in this traffic study. These models were then revised to account for the addition of the hotel and restaurant. Additional turning movement counts were collected, via Traffic Data Collection (TDC), during the weekday AM (7 a.m. to 9 a.m.) and PM (4 p.m. to 6 p.m.) peak periods on August 14, 2019 at the intersections of:

- Big Beaver Road and Site Driveway 1
- Troy Center Drive and Site Driveway 2
- Troy Center Drive and Site Driveway 3
- Troy Center Drive and PNC Site Driveway

The existing peak hour traffic volumes are shown in Figure 1 attached to this memo.

Background Traffic Scenario

MDOT provided traffic data projected to the year 2040 for intersections on Big Beaver Road listed above. Based on a review of the traffic volumes provided by MDOT, a background growth rate of 0.2 percent was utilized on all traffic counts taken by TDC. The projection of this data to 2040 was considered adequate to account for future development that may take place between the present and 2040, so no other proposed developments were included in the background traffic.

The background traffic volumes are shown in Figure 2 attached to this memorandum.

Trip Generation

Using the information and methodologies specified in the latest version of *Trip Generation* (10^{th} Edition) published by the Institute of Transportation Engineers (ITE), ROWE forecast the weekday AM and PM peak hour trips associated with the proposed commercial development. The results of the trip generation forecasts for the proposed hotel (140 rooms) and the proposed quality restaurant (7,550 SF) are provided below in Table 2.

	Land		AM	Peak H	lour	PN	I Peak H	Hour	Week
Land Use	Use Code	Units	In	Out	Total	In	Out	Total	Day
Quality Restaurant	931	SF	3	3	6	40	19	59	633
Hotel	310	Rooms	38	27	65	40	39	79	1,154
Total	-	-	41	30	71	80	58	138	1,787

 Table 2

 Trip Generation for Proposed Commercial Development

Trip Distribution

The existing traffic volumes were used to develop a trip distribution model for the AM and PM peak hours for the new traffic that will be generated by the proposed development. The existing traffic patterns indicate the following probable distribution for the completion of the hotel (140 rooms) and the proposed quality restaurant (7,550 SF):

AM Peak Hour

20% from and 34% to the south 5% from and 63% to the east 75% from and 3% to the west

PM Peak Hour

26% from and 26% to the south 12% from and 66% to the east 62% from and 8% to the west

The site generated vehicle trip assignments for the site are shown in Figure 3 attached to this memo. The background traffic volumes were combined with the site generated traffic volumes to obtain the total future traffic volumes, which are shown in Figure 4 attached to this memo.

Level of Service Analysis

A level of service (LOS) analysis for existing, background (no build), and total future (build) conditions for the AM and PM peak hours was performed for the intersections of:

- Big Beaver Road and Site Driveway 1
- Big Beaver Road and Troy Center Drive
- Troy Center Drive and Site Driveway 2
- Troy Center Drive and Site Driveway 3 •
- Troy Center Drive and PNC Site Driveway
- Eastbound Big Beaver Road and westbound to eastbound crossover/Kelly Services Drive (West • of Troy Center Drive)
- Westbound Big Beaver Road and eastbound to westbound crossover (east of Troy Center Drive) •
- All signalized and stop controlled intersections in the I-75/Big Beaver Road interchange

According to the most recent edition of the *Highway Capacity Manual (6th Edition)*, LOS is a qualitative measure describing operational conditions of a traffic stream or intersection. LOS ranges from A to F, with LOS A being the best and LOS D generally being considered acceptable. Table 3 presents the criteria for defining the various levels of service for signalized and unsignalized intersections.

LOS Criteria					
LOS	Average Stopped Delay/Vehicle (seconds)				
LUS	Signalized Intersection Unsignalized Intersection				
А	≤ 10	≤ 10			
В	$> 10 \text{ and } \le 20$	$> 10 \text{ and } \le 15$			
С	$> 20 \text{ and } \le 35$	> 15 and ≤ 25			
D	$> 35 \text{ and } \le 55$	> 25 and ≤ 35			
Е	> 55 and ≤ 80	$>$ 35 and \leq 50			
F	> 80	> 50			

Table 3
LOS Criteria

Note: LOS D is considered acceptable in urban/suburban areas.

The results of the LOS analyses for the intersection listed above are summarized in the tables below.

Unsignalized Intersection of Big Beaver Road and Site Driveway 1

The intersection of Big Beaver Road and Site Driveway 1 consists of a stop controlled northbound (NB) approach with one incoming and one outgoing lane. Big Beaver Road consists of three eastbound (EB) lanes, with the rightmost lane supporting both through and right turn movements. The results of the LOS analysis for the unsignalized intersection of Big Beaver Road and Site Driveway 1 indicate that, under existing conditions, all approaches to the intersection operate at an LOS A with no delay during the AM peak hour and at an LOS F or A during the PM peak hour. These conditions are similar for the increase in background traffic in 2040. The LOS F occurs on the approach exiting the site. This analysis does not account for the signal 300 feet upstream that creates gaps for vehicles to utilize. The delays experienced by motorists may be lower than calculated.

With the addition of site generated traffic, the northbound site driveway approach at the intersection would operate at an LOS D or better during the AM peak hour and at an LOS F during the PM peak hour. The delays experienced by motorists may be lower than calculated due to the provision of gaps created by the upstream signalized intersection.

The operational results for the intersection of Big Beaver Road and Site Driveway 1 are presented in Tables 4 and 5. **Table 4**

AM Peak Hour LOS Analysis for Big Beaver Road and Site Driveway 1						
Approach Existing Background Future						
Eastbound Big Beaver Road	A (0.0)	A (0.0)	A (0.0)			
Northbound Site Driveway 1	A (0.0)	A (0.0)	D (34.2)			
Overall	A (0.0)	A (0.0)	A (0.1)			

(XX.X) Average seconds of delay per vehicle.

PM Peak Hour LOS Analysis for Big Beaver Road and Site Driveway 1							
Approach Existing Background Future							
Eastbound Big Beaver Road	A (0.0)	A (0.0)	A (0.0)				
Northbound Site Driveway 1	F (87.6)	F (105.9)	F (161.1)				
Overall	A (0.2)	A (0.2)	A (0.8)				

Table 5

(XX.X) Average seconds of delay per vehicle.

Signalized Intersection of Big Beaver Road and Troy Center Drive

The results of the LOS analysis for the signalized intersection of Big Beaver Road and Troy Center Drive indicate that, under existing conditions, all approaches to the intersection operate at an LOS D or better during both the AM and PM peak hours. With the increase in background traffic, the eastbound Big Beaver Road approach to the intersection operates at an LOS B or better during the AM and PM peak hours; however, the northbound Troy Center Drive approach operates at an LOS E. This analysis does not account for gaps in traffic from upstream signals, which could allow vehicles to turn right on red, reducing the delay.

With the addition of site generated traffic, all approaches to the intersection would operate with minimal increases to delay and a similar LOS at all approaches during both the AM and PM peak hours when compared to background conditions. Therefore, the traffic generated by the proposed development would have a minimal impact on the operation of this intersection.

The operational results for the intersection of Big Beaver Road and Troy Center Drive are presented in Tables 6 and 7.

Table 6 AM Peak Hour							
LOS Analysis for Big Beaver Road and Troy Center Drive							
Approach Existing Background Future							
Eastbound Big Beaver Road	A (0.5)	A (4.4)	A (4.5)				
Northbound Troy Center Drive	D (52.5)	E (57.4)	E (57.5)				
Overall	A (3.2)	A (7.1)	A (7.4)				

(XX.X) Average seconds of delay per vehicle.

PM Peak Hour							
LOS Analysis for Big Beaver Road and Troy Center Drive							
Approach Existing Background Future							
Eastbound Big Beaver Road	B (10.0)	B (22.3)	C (27.7)				
Northbound Troy Center Drive	D (52.0)	E (63.1)	E (63.0)				
Overall	B (13.4)	C (25.6)	C (30.8)				

Table 7

(XX.X) Average seconds of delay per vehicle.

Unsignalized Intersection of Troy Center Drive and Site Driveway 2

The intersection of Troy Center Drive and Site Driveway 2 consists of a stop controlled eastbound approach with one incoming and one outgoing lane. Troy Center Drive consists of two southbound (SB) lanes, with the rightmost lane supporting both through and right turn movements. The results of the LOS analysis for the unsignalized intersection of Troy Center Drive and Site Driveway 2 indicate that, under existing conditions, all approaches to the intersection operate at an LOS A or better during both the AM and PM peak hours. With the increase in background traffic, all approaches to the intersection operate at an LOS A or better during the AM and PM peak hours.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS A or better during the AM and PM peak hours. Therefore, the traffic generated by the proposed development would have a negligible impact on the operation of this intersection.

The operational results for the intersection of Troy Center Drive and Site Driveway 2 are presented in Tables 8 and 9.

AM Peak Hour						
LOS Analysis for Troy Center Drive and Site Driveway 2 Approach Existing Background Future						
A (9.0)	A (9.1)	A (9.2)				
A (0.0)	A (0.0)	A (0.0)				
A (0.0)	A (0.0)	A (0.4)				
	er Drive and Existing A (9.0) A (0.0)	Existing Background A (9.0) A (9.1) A (0.0) A (0.0) A (0.0) A (0.0)				

Table 8

(XX.X) Average seconds of delay per vehicle.

Table 9 **PM Peak Hour**

LOS marysis for froy Center Drive and She Driveway 2					
Approach	Existing	Background	Future		
Eastbound Site Driveway 2	A (8.9)	A (9.0)	A (9.3)		
Southbound Troy Center Drive	A (0.0)	A (0.0)	A (0.0)		
Overall	A (0.4)	A (0.4)	A (0.9)		

(XX.X) Average seconds of delay per vehicle.

Unsignalized Intersection of Troy Center Drive and Site Driveway 3

The intersection of Troy Center Drive and Site Driveway 3 consists of a stop controlled eastbound approach with one incoming and one outgoing lane. Troy Center Drive consists of two southbound lanes, with the rightmost lane supporting both through and right turn movements. The results of the LOS analysis for the unsignalized intersection of Troy Center Drive and Site Driveway 3 indicate that, under existing conditions, all approaches to the intersection operate at an LOS A or better during the AM and PM peak hours. With

the increase in background traffic, all approaches to the intersection operate at an LOS A or better during the AM and PM peak hours.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS A or better during the AM and PM peak hours. Therefore, the traffic generated by the proposed development would have a negligible impact on the operation of this intersection.

The operational results for the intersection of Troy Center Drive and Site Driveway 3 are presented in Tables 10 and 11.

Table 10 AM Peak Hour LOS Analysis for Troy Center Drive and Site Driveway 3						
Approach Existing Background Futur						
Eastbound Site Driveway 3	A (8.7)	A (8.8)	A (8.8)			
Southbound Troy Center Drive	A (0.0)	A (0.0)	A (0.0)			
Overall	A (0.1)	A (0.1)	A (0.1)			

(XX.X) Average seconds of delay per vehicle.

Table 11							
PM Peak Hour							
LOS Analysis for Troy Cen	LOS Analysis for Troy Center Drive and Site Driveway 3						
Approach Existing Background Future							
Eastbound Site Driveway 3	A (8.8)	A (8.9)	A (9.1)				
Southbound Troy Center Drive	A (0.0)	A (0.0)	A (0.0)				
Overall	A (0.8)	A (0.8)	A (1.6)				

(XX.X) Average seconds of delay per vehicle.

Unsignalized Intersection of Troy Center Drive and PNC Site Driveway

The intersection of Troy Center Drive and PNC Site Driveway consists of stop controlled eastbound and westbound (WB) approaches with one eastbound lane that can make a left turn or through movement and one eastbound lane that can make a right turn. Troy Center Drive consists of two northbound lanes, with the rightmost lane supporting both through and right turn movements. The results of the LOS analysis for the unsignalized intersection of Troy Center Drive and PNC Site Driveway indicate that, under existing conditions, all approaches to the intersection operate at an LOS B or better during the AM and PM peak hours. With the small increase in background traffic, all approaches to the intersection operate at an LOS B or better during the AM and PM peak hours.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS B or better during the AM and PM peak hours. Therefore, the traffic generated by the proposed development would have a negligible impact on the operation of this intersection.

The operational results for the intersection of Troy Center Drive and PNC Site Driveway are presented in Tables 12 and 13.

AM Peak Hour LOS Analysis for Troy Center Drive and PNC Site Driveway							
Approach Existing Background Future							
Eastbound PNC Site Driveway	B (10.5)	B (10.7)	B (10.7)				
Westbound PNC Site Driveway	A (8.7)	A (8.7)	A (8.7)				
Northbound Troy Center Drive	A (0.0)	A (0.0)	A (0.0)				
Overall	A (4.0)	A (4.0)	A (3.9) ¹				

Table 12

(XX.X) Average seconds of delay per vehicle.

¹Delay decreases due to rounding in HCM methodology

Table 13
PM Peak Hour
LOS Analysis for Troy Center Drive and PNC Site Driveway

Approach	Future				
Eastbound PNC Site Driveway	B (10.2)	B (10.3)	B (10.7)		
Westbound PNC Site Driveway	A (9.5)	A (9.6)	A (9.8)		
Northbound Troy Center Drive	A (0.0)	A (0.0)	A (0.0)		
Overall	A (5.2)	A (5.3)	A (5.0) ¹		

(XX.X) Average seconds of delay per vehicle.

¹Delay decreases due to rounding in HCM methodology

Signalized Intersection of EB Big Beaver Road and WB to EB Crossover/Kelly Services Drive

The results of the LOS analysis for the signalized intersection of eastbound Big Beaver Road and westbound to eastbound crossover/Kelly Services Drive indicate that, under existing conditions, all approaches to the intersection operate at an LOS E or better during the AM and PM peak hours. With the increase in background traffic, all approaches to the intersection operate at an LOS E or better during the AM peak hour and at an LOS F in the PM peak hour. The LOS F on the northbound and southbound approaches in the PM hour may be caused by volume balancing completed by MDOT in the provided analysis files. A large number of vehicles exit the site the in PM, but no vehicles exit the site in the AM, which is not typical for an office building. It appears likely that traffic volume data was not collected and input into the MDOT analysis files, but rather that this intersection was used as a volume balancing node. Notwithstanding, with an actuated signal, signal timings and delays will vary throughout the peak hour, and delays experienced by motorists may be less than predicted by the model.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS E or better during the AM peak hour and at an LOS F in the PM peak hour, with a very minimal increase in delays. Therefore, the traffic generated by the proposed development would not have a noticeable impact on the operation of this intersection.

The operational results for the intersection of eastbound Big Beaver Road and westbound to eastbound crossover/Kelly Services Drive are presented in Tables 14 and 15.

Table 14AM Peak HourLOS Analysis for EB Big Beaver Road and WB to EB Crossover/Kelly Services Drive

Approach	Existing	Background	Future
Eastbound Big Beaver Road	B (10.3)	B (14.3)	B (14.6)
Northbound Kelly Services Drive	A (0.0)	A (0.0)	A (0.0)
Southbound WB to EB Crossover	E (66.4)	E (78.4)	$E(78.2)^{1}$
Overall	C (24.7)	C (30.7)	C (30.8)

(XX.X) Average seconds of delay per vehicle.

¹Delay decreases due to rounding in HCM methodology.

Table 15PM Peak HourLOS Analysis for EB Big Beaver Road and WB to EB Crossover/Kelly Services Drive

Approach	Existing	Background	Future
Eastbound Big Beaver Road	A (9.5)	B (10.8)	B (11.6)
Northbound Kelly Services Drive	E (75.7)	F (131.6)	F (131.6)
Southbound WB to EB Crossover	E (65.7)	F (122.4)	F(130.0)
Overall	C (24.9)	D (40.2)	D (41.7)

(XX.X) Average seconds of delay per vehicle.

Signalized Intersection of WB Big Beaver Road and EB to WB Crossover (East of Troy Center Drive) The results of the LOS analysis for the signalized intersection of westbound Big Beaver Road and eastbound to westbound crossover indicate that, under existing conditions, all approaches to the intersection operate at an LOS C or better during the AM peak hour and at an LOS E or higher during the PM peak hour. With the increase in background traffic all approaches to the intersection operate at an LOS E or better during the AM and PM peak hours. This analysis does not account for gaps in traffic from upstream signals, which could allow vehicles to turn right on red, reducing the delay.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS E or better during the AM and PM peak hours. Therefore, the traffic generated by the proposed development would have a negligible impact on the operation of this intersection.

The operational results for the intersection of westbound Big Beaver Road and eastbound to westbound crossover are presented in Tables 16 and 17.

LOS Analysis for WB Big Beaver Road and EB to WB Crossover						
Approach Existing Background Future						
Westbound Big Beaver Road	A (8.5)	A $(5.9)^1$	A (5.9)			
Northbound EB to WB Crossover	C (34.2)	E (73.2)	$E(72.8)^{1}$			
Overall	A (9.2)	A (7.5) ¹	A (7.6)			

Table 16 AM Peak Hour LOS Analysis for WB Big Beaver Road and EB to WB Crossover

(XX.X) Average seconds of delay per vehicle.

¹Delay decreases due to actuated signal timing.

LOS Analysis for WB Big Beaver Road and EB to WB Crossover						
Approach	pproach Existing Background Futu					
Westbound Big Beaver Road	A (6.1)	A (5.6)	A (5.7)			
Northbound EB to WB Crossover	E (58.3)	E (59.7)	$E(59.3)^{1}$			
Overall	B (12.9)	B (12.2)	B (12.3)			

Table 17
PM Peak Hour
LOS Analysis for WB Big Beaver Road and EB to WB Crossover

(XX.X) Average seconds of delay per vehicle.

¹Delay decreases due to actuated signal timing.

Existing Interchange of Big Beaver Road and I-75

The results of the LOS analysis for the existing interchange of Big Beaver Road and I-75 indicate that, under existing conditions, all approaches to the intersection operate at an LOS D or better during the AM and PM peak hours, with the exception of the westbound Big Beaver Road and southbound I-75 off-ramp, which operates at an LOS E in the AM peak hour.

The operational results for the existing interchange of Big Beaver Road and I-75 are presented in Table 18.

LOS Analysis for Existing Big Beaver Road and I-75 Interchange					
Intersection	Approach	Traffic Control	AM Peak	PM Peak	
ED Dia Daavan	Eastbound Big Beaver Road	Free	A (0.0)	A (0.2)	
EB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (0.2)	A (0.5)	
Koau aliu ND 1-75	NB I-75 Off-Ramp	Stop Sign	D (34.2)	C (24.9)	
	Eastbound Big Beaver Road	Signal	A (4.1)	B (15.8)	
EB Big Beaver Road and SB I-75	SB I-75 On-Ramp	Free	A (0.5)	A (0.5)	
Road and SB I-75	SB I-75 Off-Ramp	Signal	D (41.2)	D (54.2)	
WD Dig Deeven	Westbound Big Beaver Road	Signal	A (5.3)	A (1.4)	
WB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (0.2)	A (0.1)	
Koau aliu ND 1-73	NB I-75 Off-Ramp	Signal	D (42.9)	D (47.6)	
WD Dia Deeven	Westbound Big Beaver Road	Signal	B (18.9)	A (8.1)	
WB Big Beaver Road and SB I-75	SB I-75 On-Ramp	Free	A (0.1)	A (0.1)	
Kuau aliu SD I-73	SB I-75 Off-Ramp	Signal	E (62.1)	D (39.4)	

 Table 18

 LOS Analysis for Existing Big Beaver Road and I-75 Interchang

(XX.X) Average seconds of delay per vehicle.

Proposed Interchange of Big Beaver Road and I-75

MDOT is currently in the design phase for the reconstruction of the interchange of Big Beaver Road and I-75. The existing cloverleaf interchange will be removed, and a Diverging Diamond Interchange (DDI) will be built. The results of the LOS analysis for the proposed interchange of Big Beaver Road and I-75 indicate that, under background conditions, all approaches to the new intersections associated with the construction of a DDI operate at an LOS D or better during the AM and PM peak hours, with the exception of westbound Big Beaver Road and southbound I-75 which operates at an LOS E in the PM peak hour. These delays are considered acceptable for a major urban freeway interchange.

With the addition of site generated traffic, all approaches to the intersection would operate at an LOS D or better during the AM and PM peak hours, with the exception of westbound Big Beaver Road and southbound I-75 which operates at an LOS E in the PM peak hour. Therefore, the traffic generated by the proposed development would have a negligible impact on the operation of this intersection.

The operational results for the proposed interchange of Big Beaver Road and I-75 are presented in Tables 19 and 20.

LOS Analysis for Proposed Big Beaver Road and 1-75 Interchange				
Intersection	Approach	Traffic Control	Background	Future
ED Dig Deeven Dood	Eastbound Big Beaver Road	Signal	C (26.8)	C (27.0)
EB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (8.0)	A (8.0)
allu IND I-73	NB I-75 Off-Ramp	Signal	C (25.2)	C (25.2)
ED Die Deeren Deed	Eastbound Big Beaver Road	Signal	D (49.6)	D (50.2)
EB Big Beaver Road	SB I-75 On-Ramp	Free	A (0.0)	A (0.0)
and SB I-75	SB I-75 Off-Ramp	Signal	B (13.8)	B (13.8)
WD Die Deeren Deed	Westbound Big Beaver Road	Signal	B (19.6)	B (19.6)
WB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (0.0)	A (0.0)
and INB I-75	NB I-75 Off-Ramp	Signal	C (29.2)	C (29.2)
	Westbound Big Beaver Road	Signal	D (42.2)	D (42.3)
WB Big Beaver Road	SB I-75 On-Ramp	Free	A (7.5)	A (7.5)
and SB I-75	SB I-75 Off-Ramp	Signal	D (51.4)	D (51.4)

Table 19
AM Peak Hour
LOS Analysis for Proposed Big Beaver Road and I-75 Interchange

Table 20	
PM Peak Hour	
LOS Analysis for Proposed Big Beaver Road and I-75 Interchange	

Intersection	Approach	Traffic Control	Background	Future
ED Dig Deeven Dood	Eastbound Big Beaver Road	Signal	C (34.8)	D (36.0)
EB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (9.5)	A (9.5)
allu IND 1-73	NB I-75 Off-Ramp	Signal	C (31.7)	C (31.7)
ED Dig Deeven Dood	Eastbound Big Beaver Road	Signal	C (34.8)	C (34.8)
EB Big Beaver Road and SB I-75	SB I-75 On-Ramp	Free	A (0.0)	A (0.0)
allu SD 1-73	SB I-75 Off-Ramp	Signal	C (25.9)	C (25.9)
WD Die Deeren Deed	Westbound Big Beaver Road	Signal	D (47.6)	D (49.9)
WB Big Beaver Road and NB I-75	NB I-75 On-Ramp	Free	A (0.0)	A (0.0)
allu IND 1-73	NB I-75 Off-Ramp	Signal	B (17.7)	B (17.7)
WD Die Deeren Deed	Westbound Big Beaver Road	Signal	E (74.4)	E (75.5)
WB Big Beaver Road and SB I-75	SB I-75 On-Ramp	Free	A (7.5)	A (7.5)
allu SD I-73	SB I-75 Off-Ramp	Signal	C (27.2)	C (27.2)

Conclusions and Recommendations for the Traffic Impact Assessment

The proposed project consists of a 140-room hotel and a 7,550-square-foot quality restaurant. The proposed development will have access to Troy Center Drive via two existing approaches and access to Big Beaver Road via one existing approach. The proposed development is forecast to generate 71 trips during the AM peak hour (41 inbound and 30 outbound from the site) and 138 trips during the PM peak hour (80 inbound and 58 outbound from the site).

An operational analysis was performed for existing, background and total future conditions for the intersections of:

- Big Beaver Road and Site Driveway 1
- Big Beaver Road and Troy Center Drive
- Troy Center Drive and Site Driveway 2
- Troy Center Drive and Site Driveway 3
- Troy Center Drive and PNC Site Driveway
- Eastbound Big Beaver Road and westbound to eastbound crossover/Kelly Services Drive (west of Troy Center Drive)
- Westbound Big Beaver Road and eastbound to westbound crossover (east of Troy Center Drive)
- All signalized and stop controlled intersections in the I-75/Big Beaver Road interchange

The operational analysis indicated that most approaches of the intersections studied would operate at acceptable levels during both the AM and PM peak hours. The northbound approach of the site driveway exiting onto Big Beaver Road, the northbound approach of Troy Center Drive turning onto Big Beaver Road, and the northbound approach to the eastbound to westbound crossover experience larger delays and an LOS E or F in existing, background, and future traffic scenarios. This analysis does not account for the close proximity of the upstream signals that can create gaps in traffic flow for turning vehicles to utilize, thereby decreasing the delay experienced by vehicles. All delays experienced by motorists are considered acceptable.

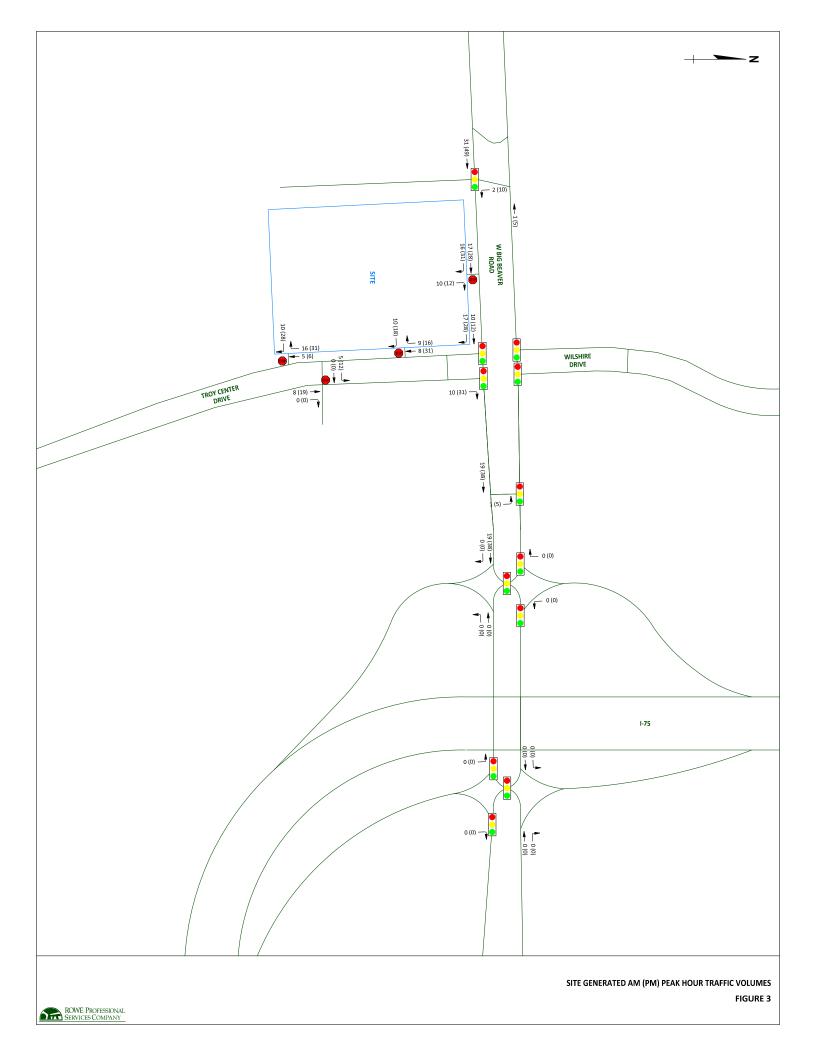
Attachments

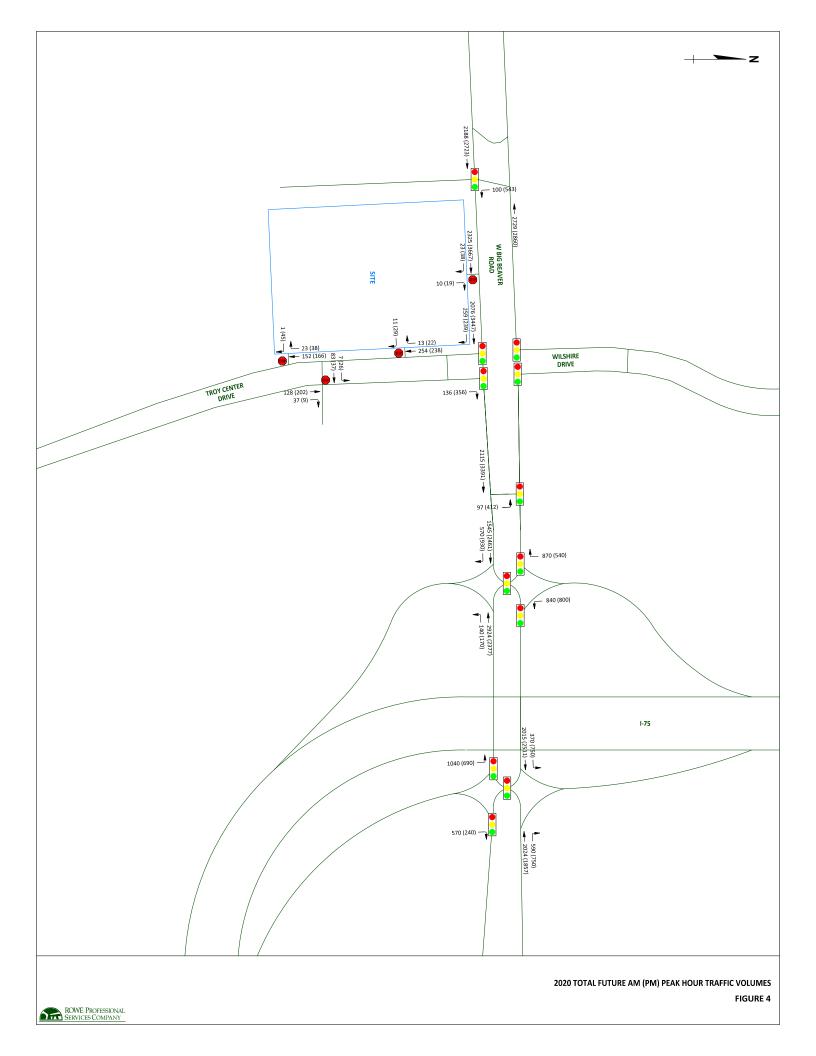
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REPORT FIGURES









TRAFFIC COUNTS



www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4SY NE File Name : TMC_1 EB Big Beaver & Site Dw_8-14-19 Site Code : TMC_1 Start Date : 8/14/2019 Page No : 1

4 Hour traffic study was conducted during typical weekday (Wednesday) from 7:00 AM - 9:00 AM morning & 4:00 PM - 6:00 PM afternoon peak hours, while school was not in session.

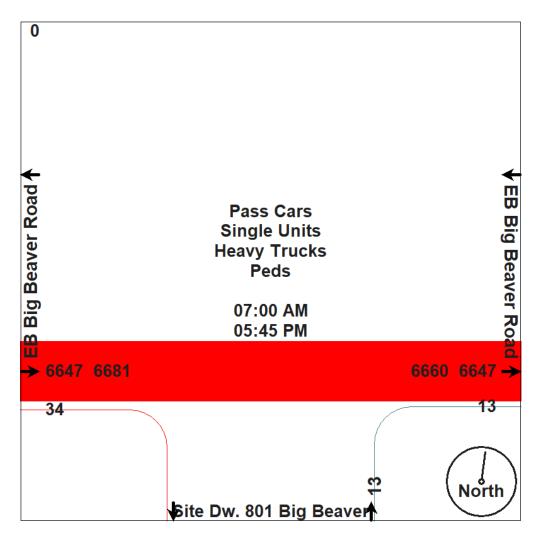
			Gro	ups Printed-	Pass Car	s - Single	Units - H	eavy Trucks	s - Peds				
	E		aver Road			e Dw. 801	Big Beav				aver Road		
		Westb				North				Eastb			
Start Time	Thru	Left		App. Total	Right	Left		App. Total	Right	Thru		pp. Total	Int. Total
07:00 AM	0	0	0	0	1	0	0	1	4	279	0	283	284
07:15 AM	0	0	0	0	0	0	0	0	1	315	0	316	316
07:30 AM	0	0	0	0	0	0	1	1	0	343	0	343	344
07:45 AM	0	0	0	0	0	0	0	0	0	345	0	345	345
Total	0	0	0	0	1	0	1	2	5	1282	0	1287	1289
08:00 AM	0	0	0	0	0	0	0	0	2	363	0	365	365
08:15 AM	0	0	0	0	0	0	0	0	4	396	0	400	400
08:30 AM	0	0	0	0	0	0	0	0	0	333	0	333	333
08:45 AM	0	0	0	0	0	0	2	2	1	410	0	411	413
Total	0	0	0	0	0	0	2	2	7	1502	0	1509	1511
*** BREAK ***													
04:00 PM	0	0	0	0	0	0	0	0	7	459	0	466	466
04:15 PM	0	0	0	0	3	0	0	3	3	481	0	484	487
04:30 PM	0	0	0	0	2	0	0	2	2	492	0	494	496
04:45 PM	0	0	0	0	0	0	1	1	3	464	0	467	468
Total	0	0	0	0	5	0	1	6	15	1896	0	1911	1917
05:00 PM	0	0	0	0	3	0	0	3	1	530	0	531	534
05:15 PM	0	0	0	0	3	0	0	3	2	489	0	491	494
05:30 PM	0	0	0	0	1	0	0	1	1	496	0	497	498
05:45 PM	0	0	0	0	0	0	1	1	3	452	0	455	456
Total	0	0	0	0	7	0	1	8	7	1967	0	1974	1982
Grand Total	0	0	0	0	13	0	5	18	34	6647	0	6681	6699
Apprch %	0	0	0		72.2	0	27.8		0.5	99.5	0		
Total %	0	0	0	0	0.2	0	0.1	0.3	0.5	99.2	0	99.7	
Pass Cars	0	0	0	0	13	0	0	13	34	6538	0	6572	6585
% Pass Cars	0	0	0	0	100	0	0	72.2	100	98.4	0	98.4	98.3
Single Units	0	0	0	0	0	0	0	0	0	81	0	81	81
% Single Units	0	0	0	0	0	0	0	0	0	1.2	0	1.2	1.2
Heavy Trucks	0	0	0	0	0	0	0	0	0	28	0	28	28
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0.4	0	0.4	0.4
Peds	0	0	0	0	0	0	5	5	0	0	0	0	5
% Peds	0	0	0	0	0	0	100	27.8	0	0	0	0	0.1

TDC Traffic Comments: Non-signalized "T" intersection. Video VCU camera was located within NE intersection quadrant. Note: Peds. are excluded from peak hour reports. Traffic study was performed for Troy Center Drive Traffic Impact Study for ROWE Professional Services Company.



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Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4SY NE File Name : TMC_1 EB Big Beaver & Site Dw_8-14-19 Site Code : TMC_1 Start Date : 8/14/2019 Page No : 2



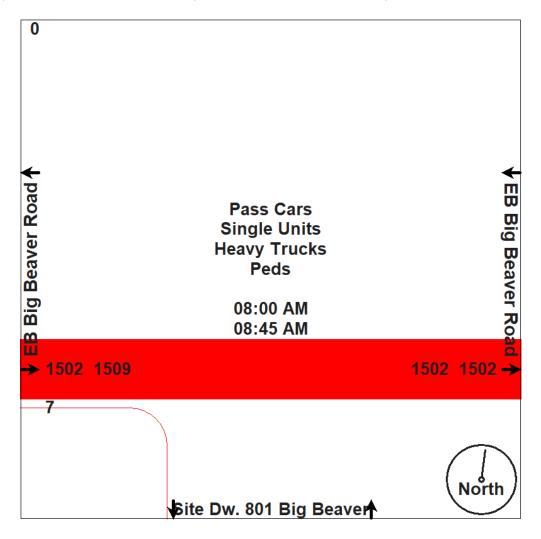


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ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4SY NE File Name : TMC_1 EB Big Beaver & Site Dw_8-14-19 Site Code : TMC_1 Start Date : 8/14/2019 Page No : 3

		Big Beaver F Westbound		Site D	w. 801 Big E Northbound		EB E	Big Beaver I		
								Eastbound]
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total
Peak Hour Analysis From	m 07:00 AM to	o 12:30 PM	- Peak 1 of 1							
Peak Hour for Entire Inte	ersection Beg	ins at 08:00	AM							
08:00 AM	0	0	0	0	0	0	2	363	365	365
08:15 AM	0	0	0	0	0	0	4	396	400	400
08:30 AM	0	0	0	0	0	0	0	333	333	333
08:45 AM	0	0	0	0	0	0	1	410	411	411
Total Volume	0	0	0	0	0	0	7	1502	1509	1509
% App. Total	0	0		0	0		0.5	99.5		
PHF	.000	.000	.000	.000	.000	.000	.438	.916	.918	.918
Pass Cars	0	0	0	0	0	0	7	1472	1479	1479
% Pass Cars	0	0	0	0	0	0	100	98.0	98.0	98.0
Single Units	0	0	0	0	0	0	0	24	24	24
% Single Units	0	0	0	0	0	0	0	1.6	1.6	1.6
Heavy Trucks	0	0	0	0	0	0	0	6	6	6
% Heavy Trucks	0	0	0	0	0	0	0	0.4	0.4	0.4
Peds	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0



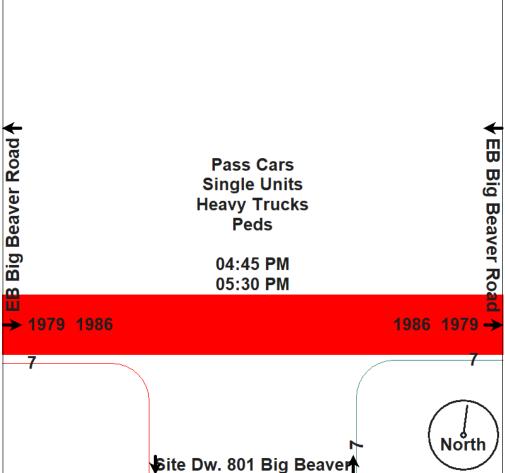


www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: OWE Professional Services Compan

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4SY NE File Name : TMC_1 EB Big Beaver & Site Dw_8-14-19 Site Code : TMC_1 Start Date : 8/14/2019 Page No : 4

	V	g Beaver Ro Vestbound		N	Northbound Eastbound			Northbound Eastbound				
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	Int. Total		
eak Hour Analysis From '												
eak Hour for Entire Inters	ection Begin	s at 04:45 P	M						1			
04:45 PM	0	0	0	0	0	0	3	464	467	467		
05:00 PM	0	0	0	3	0	3	1	530	531	534		
05:15 PM	0	0	0	3	0	3	2	489	491	494		
05:30 PM	0	0	0	1	0	1	1	496	497	498		
Total Volume	0	0	0	7	0	7	7	1979	1986	1993		
% App. Total	0	0		100	0		0.4	99.6				
PHF	.000	.000	.000	.583	.000	.583	.583	.933	.935	.933		
Pass Cars	0	0	0	7	0	7	7	1957	1964	1971		
% Pass Cars	0	0	0	100	0	100	100	98.9	98.9	98.9		
Single Units	0	0	0	0	0	0	0	15	15	15		
% Single Units	0	0	0	0	0	0	0	0.8	0.8	0.8		
Heavy Trucks	0	0	0	0	0	0	0	7	7	7		
% Heavy Trucks	0	0	0	0	0	0	0	0.4	0.4	0.4		
Peds	0	0	0	0	0	0	0	0	0	0		
% Peds	0	0	0	0	0	0	0	0	0	0		
									_			
	0											





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Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4SY NE File Name : TMC_1 EB Big Beaver & Site Dw_8-14-19 Site Code : TMC_1 Start Date : 8/14/2019 Page No : 5

Aerial Photo





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Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 6H3 SE File Name : TMC_2 SB Troy Center & North Site Dw_8-14-19 Site Code : TMC_2 Start Date : 8/14/2019 Page No : 1

4 Hour traffic study was conducted during typical weekday (Thursday) from 7:00 AM - 9:00 AM morning & 4:00 PM - 6:00 PM afternoon peak hours, while school was not in session.

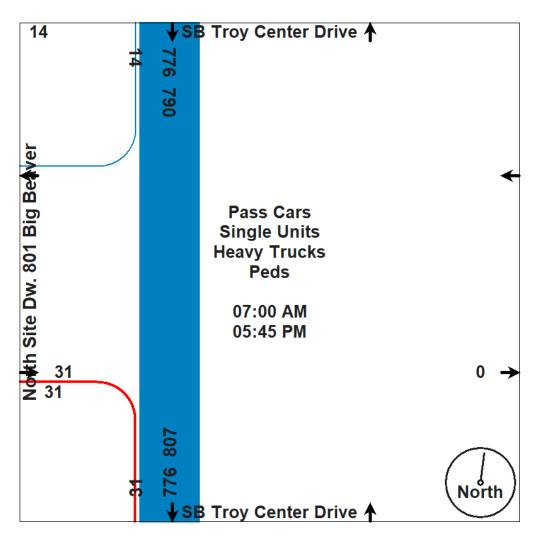
				oups Printed									
	S	B Troy Ce South		'e	S	B Troy Co North	enter Drive bound	•	North	Site Dw. Eastb		Beaver	
Start Time	Right	Thru		App. Total	Thru	Left		App. Total	Right	Left		App. Total	Int. Total
07:00 AM	1	37	0	38	0	0	0	0	2	0	0	2	40
07:15 AM	0	54	0	54	0	0	0	0	0	0	1	1	55
07:30 AM	0	47	0	47	0	0	0	0	0	0	0	0	47
07:45 AM	1	55	0	56	0	0	0	0	0	0	0	0	56
Total	2	193	0	195	0	0	0	0	2	0	1	3	198
08:00 AM	1	56	0	57	0	0	0	0	1	0	0	1	58
08:15 AM	1	54	0	55	0	0	0	0	0	0	0	0	55
08:30 AM	1	52	0	53	0	0	0	0	0	0	0	0	53
08:45 AM	1	64	0	65	0	0	0	0	0	0	1	1	66
Total	4	226	0	230	0	0	0	0	1	0	1	2	232
*** BREAK ***													
04:00 PM	3	41	0	44	0	0	0	0	11	0	0	11	55
04:15 PM	1	39	0	40	0	0	0	0	5	0	1	6	46
04:30 PM	2	47	0	49	0	0	0	0	3	0	0	3	52
04:45 PM	2	45	0	47	0	0	0	0	1	0	0	1	48
Total	8	172	0	180	0	0	0	0	20	0	1	21	201
05:00 PM	0	55	0	55	0	0	0	0	2	0	0	2	57
05:15 PM	0	50	0	50	0	0	0	0	4	0	0	4	54
05:30 PM	0	39	1	40	0	0	0	0	1	0	0	1	41
05:45 PM	0	41	0	41	0	0	0	0	1	0	0	1	42
Total	0	185	1	186	0	0	0	0	8	0	0	8	194
Grand Total	14	776	1	791	0	0	0	0	31	0	3	34	825
Apprch %	1.8	98.1	0.1		0	0	0		91.2	0	8.8		
Total %	1.7	94.1	0.1	95.9	0	0	0	0	3.8	0	0.4	4.1	
Pass Cars	14	770	0	784	0	0	0	0	31	0	0	31	815
% Pass Cars	100	99.2	0	99.1	0	0	0	0	100	0	0	91.2	98.8
Single Units	0	6	0	6	0	0	0	0	0	0	0	0	6
% Single Units	0	0.8	0	0.8	0	0	0	0	0	0	0	0	0.7
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
Peds	0	0		1	0	0	0	0	0	0	3	3	4
% Peds	0	0	100	0.1	0	0	0	0	0	0	100	8.8	0.5

TDC Traffic Comments: Non-signalized "T" intersection. Video VCU camera was located within SE intersection quadrant. Note: Peds. are excluded from peak hour reports. Traffic study was performed for Troy Center Drive Traffic Impact Study for ROWE Professional Services Company.



www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 6H3 SE File Name : TMC_2 SB Troy Center & North Site Dw_8-14-19 Site Code : TMC_2 Start Date : 8/14/2019 Page No : 2



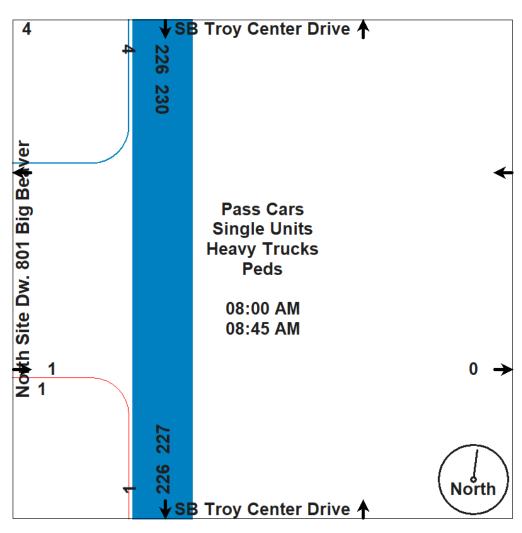


www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: OWE Professional Services Company

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 6H3 SE File Name : TMC_2 SB Troy Center & North Site Dw_8-14-19 Site Code : TMC_2 Start Date : 8/14/2019 Page No : 3

	SB	Froy Center I	Drive	SBT	Froy Center I	Drive	North Sit	e Dw. 801 B	ig Beaver	
		Southbound			Northbound			Eastbound		
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From	m 07:00 AM t	o 12:30 PM -	Peak 1 of 1				-			
Peak Hour for Entire Inte	ersection Beg	ins at 08:00	AM							
08:00 AM	1	56	57	0	0	0	1	0	1	58
08:15 AM	1	54	55	0	0	0	0	0	0	55
08:30 AM	1	52	53	0	0	0	0	0	0	53
08:45 AM	1	64	65	0	0	0	0	0	0	65
Total Volume	4	226	230	0	0	0	1	0	1	231
% App. Total	1.7	98.3		0	0		100	0		
PHF	1.00	.883	.885	.000	.000	.000	.250	.000	.250	.888
Pass Cars	4	223	227	0	0	0	1	0	1	228
% Pass Cars	100	98.7	98.7	0	0	0	100	0	100	98.7
Single Units	0	3	3	0	0	0	0	0	0	3
% Single Units	0	1.3	1.3	0	0	0	0	0	0	1.3
Heavy Trucks	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0



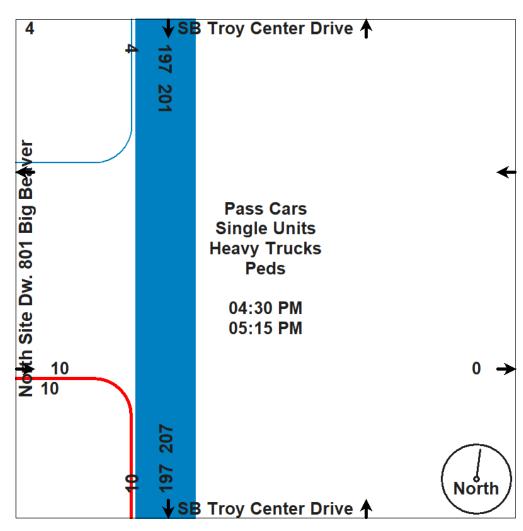


www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: OWE Professional Services Compared

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 6H3 SE File Name : TMC_2 SB Troy Center & North Site Dw_8-14-19 Site Code : TMC_2 Start Date : 8/14/2019 Page No : 4

		Froy Center		SB Troy Center Drive North Site Dw. 801 Big Beaver Northbound Eastbound						
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From	n 12:45 PM t	o 05:45 PM	- Peak 1 of 1				•			
Peak Hour for Entire Inte	rsection Beg	ins at 04:30	PM							
04:30 PM	2	47	49	0	0	0	3	0	3	52
04:45 PM	2	45	47	0	0	0	1	0	1	48
05:00 PM	0	55	55	0	0	0	2	0	2	57
05:15 PM	0	50	50	0	0	0	4	0	4	54
Total Volume	4	197	201	0	0	0	10	0	10	211
% App. Total	2	98		0	0		100	0		
PHF	.500	.895	.914	.000	.000	.000	.625	.000	.625	.925
Pass Cars	4	197	201	0	0	0	10	0	10	211
% Pass Cars	100	100	100	0	0	0	100	0	100	100
Single Units	0	0	0	0	0	0	0	0	0	0
% Single Units	0	0	0	0	0	0	0	0	0	0
Heavy Trucks	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0

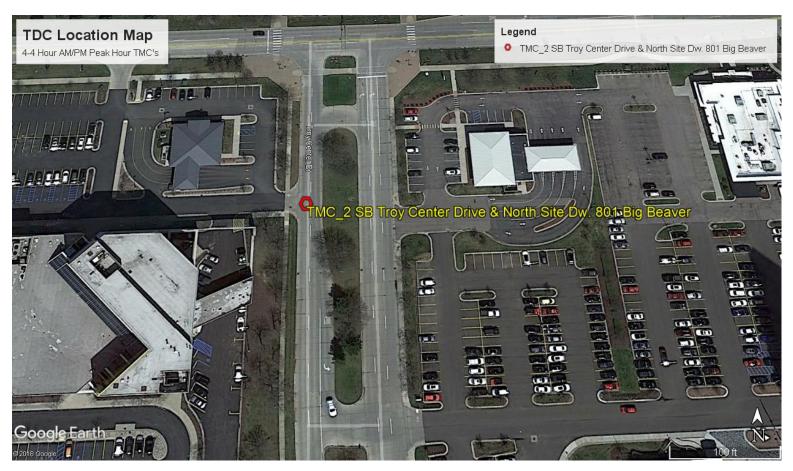




www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 6H3 SE File Name : TMC_2 SB Troy Center & North Site Dw_8-14-19 Site Code : TMC_2 Start Date : 8/14/2019 Page No : 5

Aerial Photo





www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 5DW SE File Name : TMC_3 SB Troy Center & South Site Dw_8-14-19 Site Code : TMC_3 Start Date : 8/14/2019 Page No : 1

4 Hour traffic study was conducted during typical weekday (Thursday) from 7:00 AM - 9:00 AM morning & 4:00 PM - 6:00 PM afternoon peak hours, while school was not in session.

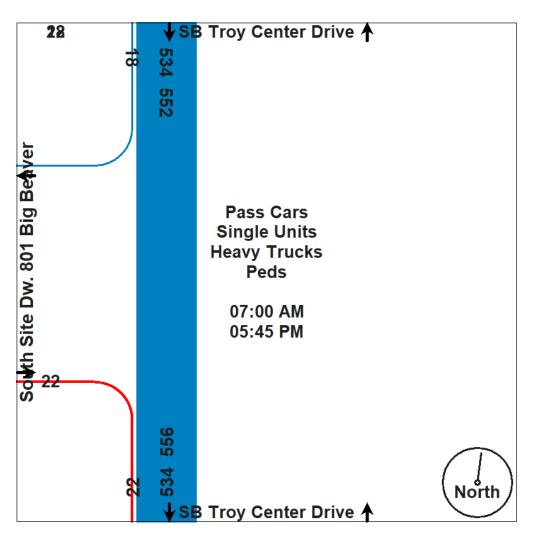
				roups Printec	I- Pass Car	s - Single	Units - H	leavy Trucks					
	S	B Troy Ce South		ve	S	B Troy Co Northl		/e	South	Site Dw. Eastb		Beaver	
Start Time	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Int. Total
07:00 AM	1	21	0	22	0	0	0	0	0	0	0	0	22
07:15 AM	1	29	0	30	0	0	0	0	0	0	0	0	30
07:30 AM	0	33	0	33	0	0	0	0	0	0	0	0	33
07:45 AM	0	31	0	31	0	0	0	0	0	0	0	0	31
Total	2	114	0	116	0	0	0	0	0	0	0	0	116
08:00 AM	3	41	0	44	0	0	0	0	0	0	0	0	44
08:15 AM	0	32	0	32	0	0	0	0	0	0	0	0	32
08:30 AM	2	29	0	31	0	0	0	0	0	0	0	0	31
08:45 AM	2	46	0	48	0	0	0	0	1	0	1	2	50
Total	7	148	0	155	0	0	0	0	1	0	1	2	157
*** BREAK ***													
04:00 PM	0	37	1	38	0	0	0	0	2	0	0	2	40
04:15 PM	2	29	0	31	0	0	0	0	7	0	1	8	39
04:30 PM	4	34	0	38	0	0	0	0	2	0	0	2	40
04:45 PM	1	34	0	35	0	0	0	0	2	0	0	2	37
Total	7	134	1	142	0	0	0	0	13	0	1	14	156
05:00 PM	0	51	0	51	0	0	0	0	5	0	0	5	56
05:15 PM	1	33	0	34	0	0	0	0	0	0	0	0	34
05:30 PM	1	35	0	36	0	0	0	0	1	0	0	1	37
05:45 PM	0	19	0	19	0	0	0	0	2	0	0	2	21
Total	2	138	0	140	0	0	0	0	8	0	0	8	148
Grand Total	18	534	1	553	0	0	0	0	22	0	2	24	577
Apprch %	3.3	96.6	0.2		0	0	0		91.7	0	8.3		
Total %	3.1	92.5	0.2	95.8	0	0	0	0	3.8	0	0.3	4.2	
Pass Cars	18	533	0	551	0	0	0	0	22	0	0	22	573
% Pass Cars	100	99.8	0	99.6	0	0	0	0	100	0	0	91.7	99.3
Single Units	0	1	0	1	0	0	0	0	0	0	0	0	1
% Single Units	0	0.2	0	0.2	0	0	0	0	0	0	0	0	0.2
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0
<u>% Heavy Trucks</u>	0	0	0	0	0	0	0	0	0	0	0	0	03
Peds % Pede	0	0 0	•		0	0 0	0 0	0	0	0	2 100		
% Peds	U	U	100	0.2	U	U	U	0	U	U	100	8.3	0.5

TDC Traffic Comments: Non-signalized "T" intersection. Video VCU camera was located within SE intersection quadrant. Note: Peds. are excluded from peak hour reports. Traffic study was performed for Troy Center Drive Traffic Impact Study for ROWE Professional Services Company.



www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 5DW SE File Name : TMC_3 SB Troy Center & South Site Dw_8-14-19 Site Code : TMC_3 Start Date : 8/14/2019 Page No : 2

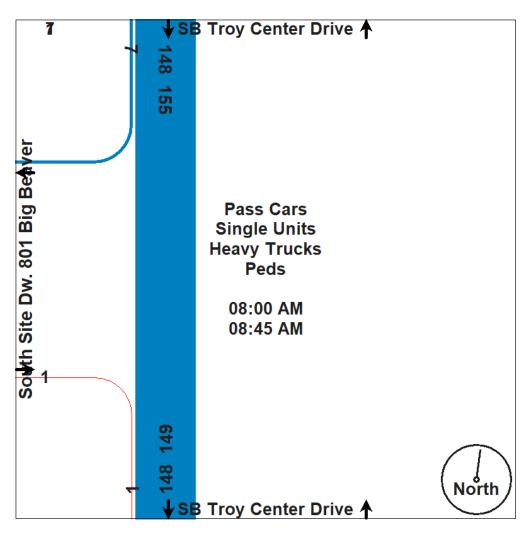




www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 5DW SE File Name : TMC_3 SB Troy Center & South Site Dw_8-14-19 Site Code : TMC_3 Start Date : 8/14/2019 Page No : 3

		oy Center E Southbound		SB Troy Center Drive Northbound			South Site Dw. 801 Big Beaver Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From	n 07:00 AM to	12:30 PM -	Peak 1 of 1				•			
Peak Hour for Entire Inte	ersection Begir	ns at 08:00	AM							
08:00 AM	3	41	44	0	0	0	0	0	0	44
08:15 AM	0	32	32	0	0	0	0	0	0	32
08:30 AM	2	29	31	0	0	0	0	0	0	31
08:45 AM	2	46	48	0	0	0	1	0	1	49
Total Volume	7	148	155	0	0	0	1	0	1	156
% App. Total	4.5	95.5		0	0		100	0		
PHF	.583	.804	.807	.000	.000	.000	.250	.000	.250	.796
Pass Cars	7	147	154	0	0	0	1	0	1	155
% Pass Cars	100	99.3	99.4	0	0	0	100	0	100	99.4
Single Units	0	1	1	0	0	0	0	0	0	1
% Single Units	0	0.7	0.6	0	0	0	0	0	0	0.6
Heavy Trucks	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0





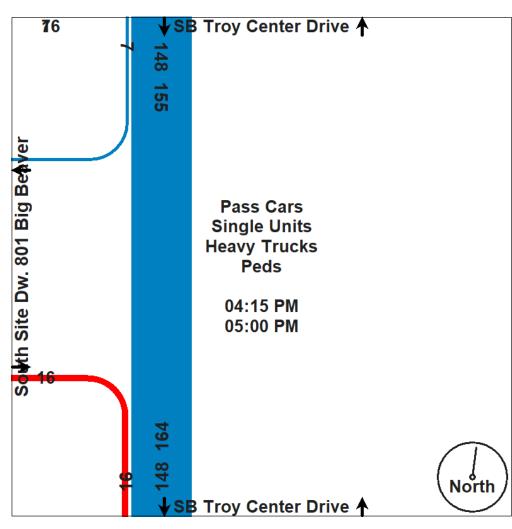
www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For:

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 5DW SE File Name : TMC_3 SB Troy Center & South Site Dw_8-14-19 Site Code : TMC_3 Start Date : 8/14/2019

Page No : 4

		roy Center D		SB Troy Center Drive			South Site			
		Southbound		<u> </u>	lorthbound			Eastbound		
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From	n 12:45 PM to	05:45 PM -	Peak 1 of 1							
Peak Hour for Entire Inte	rsection Begi	ns at 04:15	PM .							
04:15 PM	2	29	31	0	0	0	7	0	7	38
04:30 PM	4	34	38	0	0	0	2	0	2	40
04:45 PM	1	34	35	0	0	0	2	0	2	37
05:00 PM	0	51	51	0	0	0	5	0	5	56
Total Volume	7	148	155	0	0	0	16	0	16	171
% App. Total	4.5	95.5		0	0		100	0		
PHF	.438	.725	.760	.000	.000	.000	.571	.000	.571	.763
Pass Cars	7	148	155	0	0	0	16	0	16	171
% Pass Cars	100	100	100	0	0	0	100	0	100	100
Single Units	0	0	0	0	0	0	0	0	0	0
% Single Units	0	0	0	0	0	0	0	0	0	0
Heavy Trucks	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0

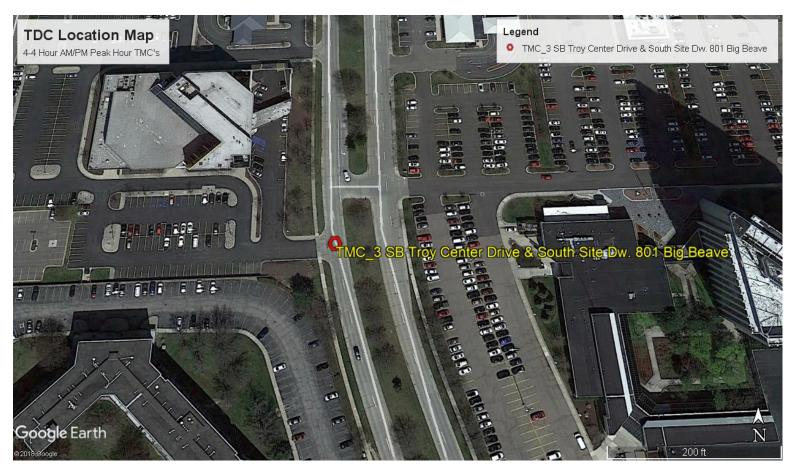




www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 5DW SE File Name : TMC_3 SB Troy Center & South Site Dw_8-14-19 Site Code : TMC_3 Start Date : 8/14/2019 Page No : 5

Aerial Photo





www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4PU NE File Name : TMC_4 NB Troy Center & PNC Dw_8-14-19 Site Code : TMC_4 Start Date : 8/14/2019 Page No : 1

4 Hour traffic study was conducted during typical weekday (Thursday) from 7:00 AM - 9:00 AM morning & 4:00 PM - 6:00 PM afternoon peak hours, while school was not in session.

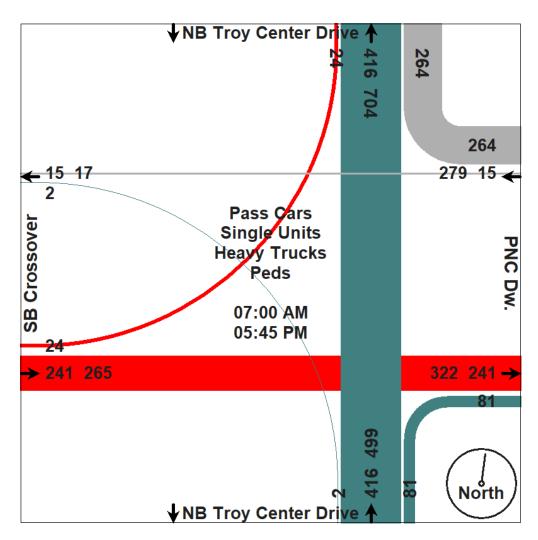
						Group	s Printe	ed- Pa	iss Cai	s - Sing	le Unit	s - Hea	avy Tru	ucks - F	Peds						
		NB Tro			ve		F	NC D	w.			NB Tro	y Cen	ter Driv	/e		SB	Cross	over		
			uthbo	und				estbo					orthbo					<u>astbou</u>	ind		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	0	15	1	0	16	21
07:15 AM	0	0	0	0	0	1	0	0	0	1	5	13	0	0	18	0	24	0	0	24	43
07:30 AM	0	0	0	0	0	1	0	0	1	2	4	5	0	0	9	0	15	0	0	15	26
07:45 AM	0	0	0	0	0	0	0	0	4	4	6	13	0	0	19	0	20	0	0	20	43
Total	0	0	0	0	0	2	0	0	5	7	15	36	0	0	51	0	74	1	0	75	133
08:00 AM	0	0	0	0	0		0	0	0	2	10	31	0	0	41	0	16	1	0	17	60
08:00 AM 08:15 AM	0	0	0 0	0 0	0	2	0	0 0	0 0	2	9	25	0	0 0	34	0	21	1	0	22	60 59
08:15 AM	0	0	0	0	0	3	1	0	0	3	9	25 27	0	0	34 36	0	20	0	0	22	59 60
08:45 AM	0	0	0	0	0	1	1	0	0	4	9	26	0	0	33	0	20	0	0	20 22	57
Total	0	0	0	0	0	9	2	0	0		35	109	0	0	144	0	79	2	0	81	236
	-	-	•	-		-	_	-	•				•	•		-		_	•		
*** BREAK **	*																				
04:00 PM	0	0	0	0	0	53	2	0	0	55	2	49	1	1	53	0	10	3	0	13	121
04:15 PM	0	0	0	0	0	29	0	0	0	29	1	32	0	0	33	0	6	8	0	14	76
04:30 PM	0	0	0	0	0	24	1	0	0	25	2	43	1	0	46	0	10	1	0	11	82
04:45 PM	0	0	0	0	0	36	3	0	0	39	4	30	0	0	34	0	9	1	0	10	83
Total	0	0	0	0	0	142	6	0	0	148	9	154	2	1	166	0	35	13	0	48	362
05:00 PM	0	0	0	0	0	40	3	0	0	43	5	52	0	0	57	0	7	2	0	9	109
05:15 PM	0	0	0	0	0	26	2	0	0	43 28	2	23	0	0	25	0	14	2	0	9 17	70
05:30 PM	0	0	0	0	0	20	2	0	1	28	10	18	0	0	23	0	13	2	0	15	70
05:45 PM	0	0	0	0	0	20	0	0	1	20	5	24	0	0	20	0	19	2	0	20	70
Total	0	0	0	0	0	111	7	0	2	120	22	117	0	0	139	0	53	8	0	61	320
Total	Ū	Ũ	0	Ŭ	0		,	Ŭ	2	120			0	U	100	0	00	0	Ū	01	020
Grand Total	0	0	0	0	0	264	15	0	7	286	81	416	2	1	500	0	241	24	0	265	1051
Apprch %	0	0	0	0		92.3	5.2	0	2.4		16.2	83.2	0.4	0.2		0	90.9	9.1	0		
Total %	0	0	0	0	0	25.1	1.4	0	0.7	27.2	7.7	39.6	0.2	0.1	47.6	0	22.9	2.3	0	25.2	
Pass Cars	0	0	0	0	0	261	15	0	0	276	81	415	2	0	498	0	237	24	0	261	1035
% Pass Cars	0	0	0	0	0	98.9	100	0	0	96.5	100	99.8	100	0	99.6	0	98.3	100	0	98.5	98.5
Single Units	0	0	0	0	0	2	0	0	0	2	0	1	0	0	1	0	4	0	0	4	7
% Single Units	0	0	0	0	0	0.8	0	0	0	0.7	0	0.2	0	0	0.2	0	1.7	0	0	1.5	0.7
Heavy Trucks	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
% Heavy Trucks	0	0	0	0	0	0.4	0	0	0	0.3	0	0	0	0	0	0	0	0	0	0	0.1
Peds	0	0	0	0	0	0	0	0	7	7	0	0	0	1	1	0	0	0	0	0	8
% Peds	0	0	0	0	0	0	0	0	100	2.4	0	0	0	100	0.2	0	0	0	0	0	0.8

TDC Traffic Comments: Non-signalized intersection. Video VCU camera was located within NE ntersection quadrant. Note: Peds. are excluded from peak hour reports. Traffic study was performed for Troy Center Drive Traffic Impact Study for ROWE Professional Services Company.



www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4PU NE File Name : TMC_4 NB Troy Center & PNC Dw_8-14-19 Site Code : TMC_4 Start Date : 8/14/2019 Page No : 2



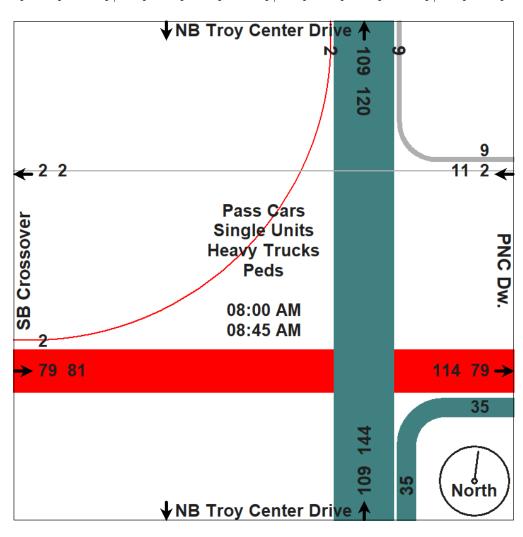


www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: DWE Professional Services Company

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4PU NE File Name : TMC_4 NB Troy Center & PNC Dw_8-14-19 Site Code : TMC_4 Start Date : 8/14/2019 Page No : 3

										. =							
	NB	Troy C		Prive		-	Dw.		NE	3 Troy C		rive			ossover		
		South	bound				bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analy	ysis Fron	n 07:00 /	AM to 1	2:30 PM -	Peak 1	of 1											
Peak Hour for E	ntire Inte	rsection	Begins	at 08:00	AM												
08:00 AM	0	0	0	0	2	0	0	2	10	31	0	41	0	16	1	17	60
08:15 AM	0	0	0	0	3	0	0	3	9	25	0	34	0	21	1	22	59
08:30 AM	0	0	0	0	3	1	0	4	9	27	0	36	0	20	0	20	60
08:45 AM	0	0	0	0	1	1	0	2	7	26	0	33	0	22	0	22	57
Total Volume	0	0	0	0	9	2	0	11	35	109	0	144	0	79	2	81	236
% App. Total	0	0	0		81.8	18.2	0		24.3	75.7	0		0	97.5	2.5		
PHF	.000	.000	.000	.000	.750	.500	.000	.688	.875	.879	.000	.878	.000	.898	.500	.920	.983
Pass Cars	0	0	0	0	7	2	0	9	35	108	0	143	0	77	2	79	231
% Pass Cars	0	0	0	0	77.8	100	0	81.8	100	99.1	0	99.3	0	97.5	100	97.5	97.9
Single Units	0	0	0	0	2	0	0	2	0	1	0	1	0	2	0	2	5
% Single Units	0	0	0	0	22.2	0	0	18.2	0	0.9	0	0.7	0	2.5	0	2.5	2.1
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



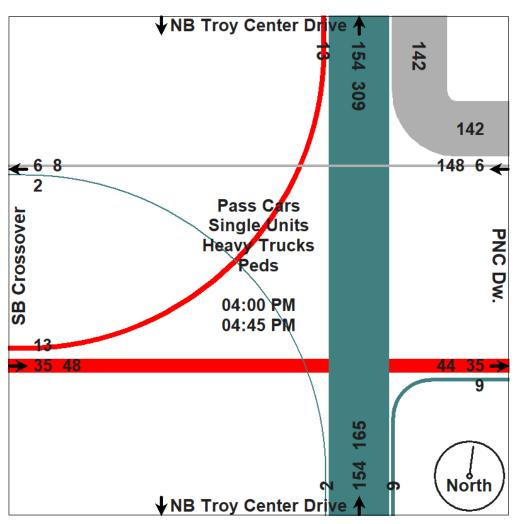


www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: OWE Professional Services Company

ROWE Professional Services Company

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4PU NE File Name : TMC_4 NB Troy Center & PNC Dw_8-14-19 Site Code : TMC_4 Start Date : 8/14/2019 Page No : 4

	NB	Troy C	enter Dri	ve		PNC	Dw.		NE	B Troy C	enter D	rive		SB Cr	ossover		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left 4	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analy	ysis Fron	n 12:45	PM to 05	:45 PM -	Peak 1	of 1											
Peak Hour for E	ntire Inte	rsection	Begins a	at 04:00	PM												
04:00 PM	0	0	0	0	53	2	0	55	2	49	1	52	0	10	3	13	120
04:15 PM	0	0	0	0	29	0	0	29	1	32	0	33	0	6	8	14	76
04:30 PM	0	0	0	0	24	1	0	25	2	43	1	46	0	10	1	11	82
04:45 PM	0	0	0	0	36	3	0	39	4	30	0	34	0	9	1	10	83
Total Volume	0	0	0	0	142	6	0	148	9	154	2	165	0	35	13	48	361
% App. Total	0	0	0		95.9	4.1	0		5.5	93.3	1.2		0	72.9	27.1		
PHF	.000	.000	.000	.000	.670	.500	.000	.673	.563	.786	.500	.793	.000	.875	.406	.857	.752
Pass Cars	0	0	0	0	142	6	0	148	9	154	2	165	0	35	13	48	361
% Pass Cars	0	0	0	0	100	100	0	100	100	100	100	100	0	100	100	100	100
Single Units	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Single Units	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Peds	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Peds	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

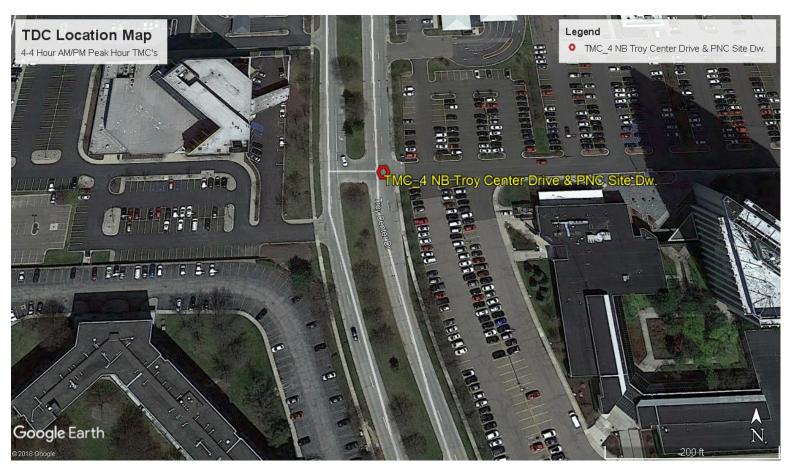




www:tdccounts.com <u>Phone: 586.786-5407</u> Traffic Study Performed For: **ROWE Professional Services Company**

Project: Troy Traffic Impact Study Study:4 Hr. Video Turning Movement Count Weather:: Sunny. Dry Deg's 80s Count By Miovision Video VCU 4PU NE File Name : TMC_4 NB Troy Center & PNC Dw_8-14-19 Site Code : TMC_4 Start Date : 8/14/2019 Page No : 5

Aerial Photo



LEVEL OF SERVICE

OUTPUT REPORTS

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u></u>	1						77	7	र्स	
Traffic Volume (vph)	0	2055	141	0	0	0	0	0	0	150	607	0
Future Volume (vph)	0	2055	141	0	0	0	0	0	0	150	607	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4							5.4	5.4	
Lane Util. Factor		0.91	1.00							0.95	0.95	
Frt		1.00	0.85							1.00	1.00	
Flt Protected		1.00	1.00							0.95	1.00	
Satd. Flow (prot)		5353	1667							1770	1861	
Flt Permitted		1.00	1.00							0.95	1.00	
Satd. Flow (perm)		5353	1667							1770	1861	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2234	153	0	0	0	0	0	0	163	660	0
RTOR Reduction (vph)	0	0	48	0	0	0	0	0	0	74	47	0
Lane Group Flow (vph)	0	2234	105	0	0	0	0	0	0	73	629	0
Turn Type		NA	Perm						Perm	Split	NA	
Protected Phases		2								3	3	
Permitted Phases			2						4			
Actuated Green, G (s)		68.1	68.1							41.1	41.1	
Effective Green, g (s)		68.1	68.1							41.1	41.1	
Actuated g/C Ratio		0.57	0.57							0.34	0.34	
Clearance Time (s)		5.4	5.4							5.4	5.4	
Vehicle Extension (s)		3.0	3.0							3.0	3.0	
Lane Grp Cap (vph)		3037	946							606	637	
v/s Ratio Prot		c0.42								0.04	c0.34	
v/s Ratio Perm			0.06									
v/c Ratio		0.74	0.11							0.12	0.99	
Uniform Delay, d1		19.3	12.0							27.1	39.2	
Progression Factor		0.50	0.21							1.78	1.22	
Incremental Delay, d2		1.2	0.2							0.0	22.7	
Delay (s)		10.8	2.7							48.3	70.3	
Level of Service		В	А							D	Е	
Approach Delay (s)		10.3			0.0			0.0			66.4	
Approach LOS		В			А			А			Е	
Intersection Summary												
HCM 2000 Control Delay			24.7	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	ratio		0.87									
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			16.2			
Intersection Capacity Utilization			98.2%			of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ተተኈ					111	
Traffic Volume (vph)	1968	230	0	0	0	120	
Future Volume (vph)	1968	230	0	0	0	120	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.98					0.85	
Flt Protected	1.00					1.00	
Satd. Flow (prot)	5269					3800	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5269					3800	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	2139	250	0	0	0	130	
RTOR Reduction (vph)	9	0	0	0	0	49	
Lane Group Flow (vph)	2380	0	0	0	0	81	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	97.9					9.5	
Effective Green, g (s)	97.9					9.5	
Actuated g/C Ratio	0.82					0.08	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4298					300	
v/s Ratio Prot	c0.45					c0.02	
v/s Ratio Perm							
v/c Ratio	0.55					0.27	
Uniform Delay, d1	3.7					52.0	
Progression Factor	0.05					1.00	
Incremental Delay, d2	0.4					0.5	
Delay (s)	0.5					52.5	
Level of Service	А					D	
Approach Delay (s)	0.5			0.0	52.5		
Approach LOS	А			А	D		
Intersection Summary							
HCM 2000 Control Delay			3.2	H	CM 2000	Level of Service	
HCM 2000 Volume to Capa	acity ratio		0.53				
Actuated Cycle Length (s)			120.0	Sı	um of lost	t time (s)	
Intersection Capacity Utiliz	ation		57.3%	IC	U Level o	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				† ††	ኘካ			
Traffic Volume (vph)	0	0	0	3565	96	0		
Future Volume (vph)	0	0	0	3565	96	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	0.97			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5353	3614			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5353	3614			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.02	0.02	0.02	3875	104	0		
RTOR Reduction (vph)	0	0	0	0	2	0		
Lane Group Flow (vph)	0	0	0	3875	102	0		
Turn Type				NA	Prot	•		
Protected Phases				6	8			
Permitted Phases				0	U			
Actuated Green, G (s)				97.1	10.3			
Effective Green, g (s)				97.1	10.3			
Actuated g/C Ratio				0.81	0.09			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				4331	310			
v/s Ratio Prot				c0.72	c0.03			
v/s Ratio Perm				00.72	00.00			
v/c Ratio				0.89	0.33			
Uniform Delay, d1				7.9	51.6			
Progression Factor				0.93	0.65			
Incremental Delay, d2				1.1	0.5			
Delay (s)				8.5	34.2			
Level of Service				A	C			
Approach Delay (s)	0.0			8.5	34.2			
Approach LOS	A			A	C			
Intersection Summary								
HCM 2000 Control Delay			9.2	Н	CM 2000	Level of Service	Α	
HCM 2000 Volume to Capacity	/ ratio		0.84					
Actuated Cycle Length (s)			120.0	Si	um of lost	time (s)	12.6	
Intersection Capacity Utilization	n		81.8%		U Level c		D	
Analysis Period (min)			15					
c Critical Lane Group								

HCM Signalized Intersection Capacity Analysis 1004: Big Beaver Rd & I-75 SB Off-Ramp

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	SBR2	NEL	NER	
Lane Configurations					<u> </u>				77		777	
Traffic Volume (vph)	0	0	0	0	2755	0	0	0	810	0	1992	
Future Volume (vph)	0	0	0	0	2755	0	0	0	810	0	1992	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)					6.3				6.0		4.0	
Lane Util. Factor					0.91				0.88		0.76	
Frt					1.00				0.85		0.85	
Flt Protected					1.00				1.00		1.00	
Satd. Flow (prot)					5353				2933		3800	
FIt Permitted					1.00				1.00		1.00	
Satd. Flow (perm)					5353				2933		3800	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	0	2995	0	0	0	880	0	2165	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	21	0	0	
Lane Group Flow (vph)	0	0	0	0	2995	0	0	0	859	0	2165	
Turn Type					NA				Prot		Free	
Protected Phases					2				4			
Permitted Phases											Free	
Actuated Green, G (s)					71.1				36.6		120.0	
Effective Green, g (s)					71.1				36.6		120.0	
Actuated g/C Ratio					0.59				0.31		1.00	
Clearance Time (s)					6.3				6.0			
Vehicle Extension (s)					0.2				3.0			
Lane Grp Cap (vph)					3171				894		3800	
v/s Ratio Prot					c0.56				c0.29			
v/s Ratio Perm											0.57	
v/c Ratio					0.94				0.96		0.57	
Uniform Delay, d1					22.6				41.0		0.0	
Progression Factor					0.55				1.00		1.00	
Incremental Delay, d2					6.4				21.1		0.5	
Delay (s)					18.9				62.1		0.5	
Level of Service					В				E		А	
Approach Delay (s)		0.0			18.9		62.1			0.5		
Approach LOS		А			В		Е			А		
Intersection Summary												
HCM 2000 Control Delay			18.6	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacity	ratio		0.95									
Actuated Cycle Length (s)			120.0		um of lost				12.6			
Intersection Capacity Utilization			Err%	IC	CU Level o	of Service			Н			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis 1005: I-75 SB On/Off-Ramp & Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER	
Lane Configurations		^	1		† ††	1		11			
Traffic Volume (vph)	0	1458	534	0	2755	133	0	788	0	0	
Future Volume (vph)	0	1458	534	0	2755	133	0	788	0	0	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)		5.8	4.0		4.0	4.0		4.8			
Lane Util. Factor		0.91	1.00		0.91	1.00		0.88			
Frt		1.00	0.85		1.00	0.85		0.85			
Flt Protected		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (prot)		5353	1667		5353	1667		2933			
Flt Permitted		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (perm)		5353	1667		5353	1667		2933			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	1585	580	0	2995	145	0	857	0	0	
RTOR Reduction (vph)	0	0	0	0	0	0	0	16	0	0	
Lane Group Flow (vph)	0	1585	580	0	2995	145	0	841	0	0	
Turn Type		NA	Free		NA	Perm		Prot			
Protected Phases		2	1100		Free			4			
Permitted Phases		_	Free			Free					
Actuated Green, G (s)		67.7	120.0		120.0	120.0		41.7			
Effective Green, g (s)		67.7	120.0		120.0	120.0		41.7			
Actuated g/C Ratio		0.56	1.00		1.00	1.00		0.35			
Clearance Time (s)		5.8						4.8			
Vehicle Extension (s)		3.0						3.0			
Lane Grp Cap (vph)		3019	1667		5353	1667		1019			
v/s Ratio Prot		0.30			0.56			c0.29			
v/s Ratio Perm			0.35			0.09					
v/c Ratio		0.53	0.35		0.56	0.09		0.83			
Uniform Delay, d1		16.2	0.0		0.0	0.0		35.8			
Progression Factor		0.22	1.00		1.00	1.00		1.00			
Incremental Delay, d2		0.5	0.5		0.3	0.1		5.5			
Delay (s)		4.1	0.5		0.3	0.1		41.4			
Level of Service		А	А		А	А		D			
Approach Delay (s)		3.1			0.2		41.4		0.0		
Approach LOS		А			А		D		А		
Intersection Summary											
HCM 2000 Control Delay			7.0	Н	CM 2000	Level of S	Service		А		
HCM 2000 Volume to Capacity	ratio		0.69								
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			10.6		
Intersection Capacity Utilization			61.8%			of Service			В		
Analysis Period (min)			15								
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis 1006: Big Beaver Rd & I-75 NB On/Off-Ramp

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NWL	NWR	
Lane Configurations		^	1		^	1		11			
Traffic Volume (vph)	0	1912	334	0	1960	524	0	928	0	0	
Future Volume (vph)	0	1912	334	0	1960	524	0	928	0	0	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)		4.0	4.0		6.7	4.0		5.4			
Lane Util. Factor		0.91	1.00		0.91	1.00		0.88			
Frt		1.00	0.85		1.00	0.85		0.85			
Flt Protected		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (prot)		5353	1667		5353	1667		2933			
Flt Permitted		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (perm)		5353	1667		5353	1667		2933			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	2078	363	0	2130	570	0	1009	0	0	
RTOR Reduction (vph)	0	0	0	0	0	0	0	21	0	0	
Lane Group Flow (vph)	0	2078	363	0	2130	570	0	988	0	0	
Turn Type		NA	Perm		NA	Free		Prot			
Protected Phases		Free			2			4			
Permitted Phases			Free			Free					
Actuated Green, G (s)		120.0	120.0		62.0	120.0		45.9			
Effective Green, g (s)		120.0	120.0		62.0	120.0		45.9			
Actuated g/C Ratio		1.00	1.00		0.52	1.00		0.38			
Clearance Time (s)					6.7			5.4			
Vehicle Extension (s)					0.2			3.0			
Lane Grp Cap (vph)		5353	1667		2765	1667		1121			
v/s Ratio Prot		0.39			c0.40			c0.34			
v/s Ratio Perm			0.22			0.34					
v/c Ratio		0.39	0.22		0.77	0.34		0.88			
Uniform Delay, d1		0.0	0.0		23.3	0.0		34.5			
Progression Factor		1.00	1.00		0.20	1.00		1.00			
Incremental Delay, d2		0.2	0.2		0.7	0.2		8.3			
Delay (s)		0.2	0.2		5.3	0.2		42.9			
Level of Service		А	А		А	А		D			
Approach Delay (s)		0.2			4.2		42.9		0.0		
Approach LOS		А			А		D		А		
Intersection Summary											
HCM 2000 Control Delay			8.9	Н	CM 2000	Level of S	Service		А		
HCM 2000 Volume to Capacity r	atio		0.82								
Actuated Cycle Length (s)			120.0		um of los				12.1		
Intersection Capacity Utilization			76.9%	IC	CU Level	of Service			D		
Analysis Period (min)			15								
c Critical Lane Group											

Intersection

Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	朴朴					1
Traffic Vol, veh/h	2198	7	0	0	0	0
Future Vol, veh/h	2198	7	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e, # 0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2389	8	0	0	0	0

Major/Minor	Major1			Μ	inor1	
Conflicting Flow All	0	0				1199
Stage 1	-				-	-
Stage 2	-	-			-	-
Critical Hdwy	-	-			-	7.14
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	-			-	3.92
Pot Cap-1 Maneuver	-	-			0	153
Stage 1	-	-			0	-
Stage 2	-	-			0	-
Platoon blocked, %	-	-				
Mov Cap-1 Maneuver		-			-	153
Mov Cap-2 Maneuver	-	-			-	-
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Approach	EB				NB	
HCM Control Delay, s					0	
HCM LOS					A	
Minor Lane/Major Mvr	nt	NBLn1	EBT	EBR		
Capacity (veh/h)		NDLIII		LDIX		
HCM Lane V/C Ratio		-	-	-		
HCM Control Delay (s	١	- 0	-	-		
HCM Lane LOS)	A	_	_		
HCM 95th %tile Q(veh	n)	-	_	_		
	'/					

Intersection

Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			- † 12	
Traffic Vol, veh/h	0	1	0	0	226	4
Future Vol, veh/h	0	1	0	0	226	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	1	0	0	246	4

Major/Minor	Minor2			Maj	or2	
Conflicting Flow All	-	125			-	0
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Critical Hdwy	-	6.94			-	-
Critical Hdwy Stg 1	-	-			-	-
Critical Hdwy Stg 2	-	-			-	-
Follow-up Hdwy	-	3.32			-	-
Pot Cap-1 Maneuver	0	902			-	-
Stage 1	0	-			-	-
Stage 2	0	-			-	-
Platoon blocked, %					-	-
Mov Cap-1 Maneuver	-	902			-	-
Mov Cap-2 Maneuver	-	-			-	-
Stage 1	-	-			-	-
Stage 2	-	-			-	-
Annroach	EB				SB	
Approach						
HCM Control Delay, s					0	
HCM LOS	А					
Minor Lane/Major Mvr	nt E	EBLn1	SBT	SBR		
		000			_	_

		-	-
Capacity (veh/h)	902	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	9	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			- † 1-	
Traffic Vol, veh/h	0	1	0	0	139	7
Future Vol, veh/h	0	1	0	0	139	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	1	0	0	151	8

Major/Minor	Minor2			Major2	
Conflicting Flow All	-	80		-	0
Stage 1	-	-		-	-
Stage 2	-	-		-	-
Critical Hdwy	-	6.94		-	-
Critical Hdwy Stg 1	-	-		-	-
Critical Hdwy Stg 2	-	-		-	-
Follow-up Hdwy	-	3.32		-	-
Pot Cap-1 Maneuver	0	964		-	-
Stage 1	0	-		-	-
Stage 2	0	-		-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	-	964		-	-
Mov Cap-2 Maneuver	-	-		-	-
Stage 1	-	-		-	-
Stage 2	-	-		-	-
Approach	EB			SB	
				0	
HCM Control Delay, s				U	
HCM LOS	A				
Minor Lane/Major Mvr	nt E	EBLn1	SBT SB	R	

Capacity (veh/h)	964	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	8.7	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

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09/17/2019

Intersection

Mayamant	EDI	ГРТ					NDI	NDT		CDI	ОРТ	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	<u>٦</u>	- सी				17		- 11	- T				
Traffic Vol, veh/h	2	79	0	0	0	9	0	109	35	0	0	0	
Future Vol, veh/h	2	79	0	0	0	9	0	109	35	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	0	-	-	-	-	0	-	-	50	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	16979	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	2	86	0	0	0	10	0	118	38	0	0	0	

Major/Minor	Minor2		Mi	nor1		Ν	lajor1			
Conflicting Flow All	59	156	-	-	-	59	-	0	0	
Stage 1	0	0	-	-	-	-	-	-	-	
Stage 2	59	156	-	-	-	-	-	-	-	
Critical Hdwy	7.54	6.54	-	-	-	6.94	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.54	5.54	-	-	-	-	-	-	-	
Follow-up Hdwy	3.52	4.02	-	-	-	3.32	-	-	-	
Pot Cap-1 Maneuver	930	735	0	0	0	994	0	-	-	
Stage 1	-	-	0	0	0	-	0	-	-	
Stage 2	946	768	0	0	0	-	0	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	· 921	735	-	-	-	994	-	-	-	
Mov Cap-2 Maneuver	· 921	735	-	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	-	-	-	
Stage 2	937	768	-	-	-	-	-	-	-	
Approach	EB			WB			NB			

Approach	EB	WB	NB	
HCM Control Delay, s	10.5	8.7	0	
HCM LOS	В	А		

Minor Lane/Major Mvmt	NBT	NBR E	EBLn1	EBLn2V	/BLn1
Capacity (veh/h)	-	-	921	736	994
HCM Lane V/C Ratio	-	-	0.002	0.118	0.01
HCM Control Delay (s)	-	-	8.9	10.5	8.7
HCM Lane LOS	-	-	А	В	А
HCM 95th %tile Q(veh)	-	-	0	0.4	0

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Movement	EBT	EBR	WBL	WBT	NBL	NBR				
Lane Configurations	^			^		11				
Traffic Volume (veh/h)	1912	0	0	2484	0	505				
Future Volume (Veh/h)	1912	0	0	2484	0	505				
Sign Control	Free			Free	Stop					
Grade	0%			0%	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92				
Hourly flow rate (vph)	2078	0	0	2700	0	549				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type	None			Raised						
Median storage veh)				1						
Upstream signal (ft)	313									
pX, platoon unblocked										
vC, conflicting volume			2078		2978	693				
vC1, stage 1 conf vol					2078					
vC2, stage 2 conf vol					900					
vCu, unblocked vol			2078		2978	693				
tC, single (s)			4.1		6.8	6.9				
tC, 2 stage (s)					5.8					
tF (s)			2.2		3.5	3.3				
p0 queue free %			100		100	0				
cM capacity (veh/h)			264		64	386				
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2		
Volume Total	693	693	693	900	900	900	274	274		
Volume Left	0	0	0	0	0	0	0	0		
Volume Right	0	0	0	0	0	0	274	274		
cSH	1700	1700	1700	1700	1700	1700	386	386		
Volume to Capacity	0.41	0.41	0.41	0.53	0.53	0.53	0.71	0.71		
Queue Length 95th (ft)	0	0	0	0	0	0	133	133		
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	34.2	34.2		
Lane LOS							D	D		
Approach Delay (s)	0.0			0.0			34.2			
Approach LOS							D			
Intersection Summary										
Average Delay			3.5							
Intersection Capacity Utilizatio	n		58.5%	IC	U Level o	of Service			В	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	1						11	ኘኘ		
Traffic Volume (vph)	0	2546	172	0	0	0	0	0	418	508	0	0
Future Volume (vph)	0	2546	172	0	0	0	0	0	418	508	0	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4						5.4	5.4		
Lane Util. Factor		0.91	1.00						0.88	0.97		
Frt		1.00	0.85						0.85	1.00		
Flt Protected		1.00	1.00						1.00	0.95		
Satd. Flow (prot)		5353	1667						2933	3614		
Flt Permitted		1.00	1.00						1.00	0.95		
Satd. Flow (perm)		5353	1667						2933	3614		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2767	187	0	0	0	0	0	454	552	0	0
RTOR Reduction (vph)	0	0	61	0	0	0	0	0	61	61	0	0
Lane Group Flow (vph)	0	2767	126	0	0	0	0	0	393	491	0	0
Turn Type		NA	Perm						Perm	Prot		
Protected Phases		2								3		
Permitted Phases			2						4			
Actuated Green, G (s)		68.7	68.7						17.5	17.6		
Effective Green, g (s)		68.7	68.7						17.5	17.6		
Actuated g/C Ratio		0.57	0.57						0.15	0.15		
Clearance Time (s)		5.4	5.4						5.4	5.4		
Vehicle Extension (s)		3.0	3.0						3.0	3.0		
Lane Grp Cap (vph)		3064	954						427	530		
v/s Ratio Prot		c0.52								c0.14		
v/s Ratio Perm			0.08						c0.13			
v/c Ratio		0.90	0.13						0.92	0.93		
Uniform Delay, d1		22.7	11.9						50.6	50.6		
Progression Factor		0.42	0.01						1.00	1.03		
Incremental Delay, d2		0.5	0.0						25.1	13.9		
Delay (s)		10.1	0.2						75.7	65.7		
Level of Service		В	А						E	E		
Approach Delay (s)		9.5			0.0			75.7			65.7	
Approach LOS		А			А			E			Е	
Intersection Summary												
HCM 2000 Control Delay			24.9	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	ratio		0.91									
Actuated Cycle Length (s)			120.0		um of lost				16.2			
Intersection Capacity Utilization	I		119.1%	IC	U Level o	of Service			Н			
Analysis Period (min)			15									
 Critical Lana Group 												

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ተተኈ					111	
Traffic Volume (vph)	3271	201	0	0	0	309	
Future Volume (vph)	3271	201	0	0	0	309	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.99					0.85	
Flt Protected	1.00					1.00	
Satd. Flow (prot)	5307					3800	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5307					3800	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	3555	218	0	0	0	336	
RTOR Reduction (vph)	4	0	0	0	0	1	
Lane Group Flow (vph)	3769	0	0	0	0	335	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	91.1					16.3	
Effective Green, g (s)	91.1					16.3	
Actuated g/C Ratio	0.76					0.14	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4028					516	
v/s Ratio Prot	c0.71					c0.09	
v/s Ratio Perm							
v/c Ratio	0.94					0.65	
Uniform Delay, d1	12.0					49.1	
Progression Factor	0.63					1.00	
Incremental Delay, d2	2.4					2.8	
Delay (s)	10.0					52.0	
Level of Service	В					D	
Approach Delay (s)	10.0			0.0	52.0		
Approach LOS	В			А	D		
Intersection Summary							
HCM 2000 Control Delay			13.4	Н	CM 2000	Level of Service	
HCM 2000 Volume to Cap	acity ratio		0.89				
Actuated Cycle Length (s)			120.0		um of lost		1
Intersection Capacity Utiliz	ation		81.6%	IC	U Level o	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				† ††	ኘካ			
Traffic Volume (vph)	0	0	0	2719	407	0		
Future Volume (vph)	0	0	0	2719	407	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	0.97			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5353	3614			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5353	3614			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.02	0.02	0.02	2955	442	0		
RTOR Reduction (vph)	0	0	0	0	5	0		
Lane Group Flow (vph)	0	0	0	2955	437	0		
Turn Type	<u> </u>	<u> </u>		NA	Prot			
Protected Phases				6	8			
Permitted Phases				Ŭ	Ū			
Actuated Green, G (s)				88.1	19.3			
Effective Green, g (s)				88.1	19.3			
Actuated g/C Ratio				0.73	0.16			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				3929	581			
v/s Ratio Prot				c0.55	c0.12			
v/s Ratio Perm				00.00	00.12			
v/c Ratio				0.75	0.75			
Uniform Delay, d1				9.5	48.1			
Progression Factor				0.54	1.16			
Incremental Delay, d2				1.0	2.4			
Delay (s)				6.1	58.3			
Level of Service				A	E			
Approach Delay (s)	0.0			6.1	58.3			
Approach LOS	A			A	E			
Intersection Summary								
HCM 2000 Control Delay			12.9	H	CM 2000	Level of Service	B	
HCM 2000 Volume to Capaci	ty ratio		0.75				_	
Actuated Cycle Length (s)	,		120.0	Si	um of lost	time (s)	12.6	
Intersection Capacity Utilization	on		71.4%		U Level o		C	
Analysis Period (min)			15			-	-	
c Critical Lane Group								

HCM Signalized Intersection Capacity Analysis 1004: Big Beaver Rd & I-75 SB Off-Ramp

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	SBR2	NEL	NER	
Lane Configurations					***				77		777	
Traffic Volume (vph)	0	0	0	0	2257	0	0	0	462	0	3173	
Future Volume (vph)	0	0	0	0	2257	0	0	0	462	0	3173	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)					6.3				6.0		4.0	
Lane Util. Factor					0.91				0.88		0.76	
Frt					1.00				0.85		0.85	
FIt Protected					1.00				1.00		1.00	
Satd. Flow (prot)					5353				2933		3800	
FIt Permitted					1.00				1.00		1.00	
Satd. Flow (perm)					5353				2933		3800	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	0	2453	0	0	0	502	0	3449	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	22	0	0	
Lane Group Flow (vph)	0	0	0	0	2453	0	0	0	480	0	3449	
Turn Type					NA				Prot		Free	
Protected Phases					2				4			
Permitted Phases											Free	
Actuated Green, G (s)					75.1				32.6		120.0	
Effective Green, g (s)					75.1				32.6		120.0	
Actuated g/C Ratio					0.63				0.27		1.00	
Clearance Time (s)					6.3				6.0			
Vehicle Extension (s)					0.2				3.0			
Lane Grp Cap (vph)					3350				796		3800	
v/s Ratio Prot					0.46				0.16			
v/s Ratio Perm											c0.91	
v/c Ratio					0.73				0.60		0.91	
Uniform Delay, d1					15.5				38.1		0.0	
Progression Factor					0.44				1.00		1.00	
Incremental Delay, d2					1.3				1.3		1.9	
Delay (s)					8.1				39.4		1.9	
Level of Service					А				D		А	
Approach Delay (s)		0.0			8.1		39.4			1.9		
Approach LOS		А			А		D			А		
Intersection Summary												
HCM 2000 Control Delay			7.2	Н	CM 2000	Level of S	Service		А			
HCM 2000 Volume to Capacity	ratio		1.01									
Actuated Cycle Length (s)			120.0	S	um of lost	time (s)			12.6			
Intersection Capacity Utilization			Err%			of Service			Н			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis 1005: I-75 SB On/Off-Ramp & Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SEL	SER	
Lane Configurations		†††	1		† ††	1		11			
Traffic Volume (vph)	0	2382	791	0	2257	142	0	680	0	0	
Future Volume (vph)	0	2382	791	0	2257	142	0	680	0	0	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)		5.8	4.0		4.0	4.0		4.8			
Lane Util. Factor		0.91	1.00		0.91	1.00		0.88			
Frt		1.00	0.85		1.00	0.85		0.85			
Flt Protected		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (prot)		5353	1667		5353	1667		2933			
Flt Permitted		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (perm)		5353	1667		5353	1667		2933			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	2589	860	0	2453	154	0	739	0	0	
RTOR Reduction (vph)	0	0	0	0	0	0	0	18	0	0	
Lane Group Flow (vph)	0	2589	860	0	2453	154	0	721	0	0	
Turn Type		NA	Free		NA	Perm		Prot			
Protected Phases		2			Free			4			
Permitted Phases			Free			Free					
Actuated Green, G (s)		76.4	120.0		120.0	120.0		33.0			
Effective Green, g (s)		76.4	120.0		120.0	120.0		33.0			
Actuated g/C Ratio		0.64	1.00		1.00	1.00		0.28			
Clearance Time (s)		5.8						4.8			
Vehicle Extension (s)		3.0						3.0			
Lane Grp Cap (vph)		3408	1667		5353	1667		806			
v/s Ratio Prot		c0.48			0.46			c0.25			
v/s Ratio Perm			0.52			0.09					
v/c Ratio		0.76	0.52		0.46	0.09		0.89			
Uniform Delay, d1		15.3	0.0		0.0	0.0		41.8			
Progression Factor		0.99	1.00		1.00	1.00		1.00			
Incremental Delay, d2		0.7	0.5		0.2	0.1		12.4			
Delay (s)		15.8	0.5		0.2	0.1		54.2			
Level of Service		В	А		А	А		D			
Approach Delay (s)		12.0			0.2		54.2		0.0		
Approach LOS		В			А		D		А		
Intersection Summary											
HCM 2000 Control Delay			12.1	Н	CM 2000	Level of S	Service		В		
HCM 2000 Volume to Capacity r	atio		0.80								
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			10.6		
Intersection Capacity Utilization			75.2%			of Service			D		
Analysis Period (min)			15								
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis 1006: Big Beaver Rd & I-75 NB On/Off-Ramp

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NWL	NWR	
Lane Configurations		^	1		† ††	1		11			
Traffic Volume (vph)	0	2377	685	0	1765	691	0	634	0	0	
Future Volume (vph)	0	2377	685	0	1765	691	0	634	0	0	
(, ,	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	
Total Lost time (s)		4.0	4.0		6.7	4.0		5.4			
Lane Util. Factor		0.91	1.00		0.91	1.00		0.88			
Frt		1.00	0.85		1.00	0.85		0.85			
Flt Protected		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (prot)		5353	1667		5353	1667		2933			
Flt Permitted		1.00	1.00		1.00	1.00		1.00			
Satd. Flow (perm)		5353	1667		5353	1667		2933			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0.02	2584	745	0.02	1918	751	0.02	689	0.02	0.02	
RTOR Reduction (vph)	0	0	0	0	0	0	0 0	25	0	0	
Lane Group Flow (vph)	0	2584	745	0	1918	751	0	664	0	0	
Turn Type		NA	Perm		NA	Free		Prot	<u> </u>		
Protected Phases		Free	I CIIII		2	1166		4			
Permitted Phases		1166	Free		2	Free		-			
Actuated Green, G (s)		120.0	120.0		74.9	120.0		33.0			
Effective Green, g (s)		120.0	120.0		74.9	120.0		33.0			
Actuated g/C Ratio		1.00	1.00		0.62	1.00		0.28			
Clearance Time (s)		1.00	1.00		6.7	1.00		5.4			
Vehicle Extension (s)					0.7			3.0			
Lane Grp Cap (vph)		5353	1667		3341	1667		806			
v/s Ratio Prot		0.48	1007		c0.36	1007		c0.23			
v/s Ratio Prot		0.40	0.45		CU.30	0.45		CU.25			
		0.48	0.45		0.57	0.45		0.82			
v/c Ratio		0.40	0.45		13.2	0.45		40.8			
Uniform Delay, d1		1.00	1.00		0.10	1.00		40.8			
Progression Factor		0.2	0.5		0.10	0.1		6.9			
Incremental Delay, d2		0.2			1.4	0.1		47.6			
Delay (s)			0.5			0.1 A					
Level of Service		A 0.3	А		A 1.0	A	47.6	D	0.0		
Approach Delay (s) Approach LOS		0.3 A			1.0 A		47.6 D		0.0 A		
Intersection Summary									~		
			E A		CM 0000	Louistef	Comiles		٨		
HCM 2000 Control Delay	alia		5.4	H		Level of S	Service		А		
HCM 2000 Volume to Capacity r	atio		0.65			1 there a ()			10.4		
Actuated Cycle Length (s)			120.0		um of los				12.1		
Intersection Capacity Utilization			63.6%	IC	U Level	of Service			В		
Analysis Period (min)			15								
c Critical Lane Group											

Intersection						
Int Delay, s/veh	0.2					
-						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ttp:					1
Traffic Vol, veh/h	3465	7	0	0	0	7
Future Vol, veh/h	3465	7	0	0	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e,#0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
· · ·						

Major/Minor	Major1		Minor1	
Conflicting Flow All	0	0	-	1887
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	-	-	7.14
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	-	-	3.92
Pot Cap-1 Maneuver	-	-	0	51
Stage 1	-	-	0	-
Stage 2	-	-	0	-
Platoon blocked, %	-	-		
Mov Cap-1 Maneuver	· -	-	-	51
Mov Cap-2 Maneuver	· _	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Approach	EB		NB	

Apploach	LD		
HCM Control Delay, s	0	87.6	
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	51	-	-
HCM Lane V/C Ratio	0.149	-	-
HCM Control Delay (s)	87.6	-	-
HCM Lane LOS	F	-	-
HCM 95th %tile Q(veh)	0.5	-	-

h	ntersection	

Int Delay, s/veh

Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			≜ î≽	
Traffic Vol, veh/h	0	10	0	0	197	4
Future Vol, veh/h	0	10	0	0	197	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	e, # 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	11	0	0	214	4

Major/Minor	Minor2			Major2		
Conflicting Flow All	-	109		-	0	
Stage 1	-	-		-	-	
Stage 2	-	-		-	-	
Critical Hdwy	-	6.94		-	-	
Critical Hdwy Stg 1	-	-		-	-	
Critical Hdwy Stg 2	-	-		-	-	
Follow-up Hdwy	-	3.32		-	-	
Pot Cap-1 Maneuver	0	924		-	-	
Stage 1	0	-		-	-	
Stage 2	0	-		-	-	
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver		924		-	-	
Mov Cap-2 Maneuver	• -	-		-	-	
Stage 1	-	-		-	-	
Stage 2	-	-		-	-	
Approach	EB			SB		
HCM Control Delay, s				0		
HCM LOS	А					
Minor Lane/Major Mvr	mt E	BLn1	SBT SE	BR		

,			
Capacity (veh/h)	924	-	-
HCM Lane V/C Ratio	0.012	-	-
HCM Control Delay (s)	8.9	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

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Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			_ ^ ↑₽	
Traffic Vol, veh/h	0	16	0	0	152	7
Future Vol, veh/h	0	16	0	0	152	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	,# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	17	0	0	165	8

Major/Minor	Minor2			Ма	jor2		
Conflicting Flow All	-	87			-	0	
Stage 1	-	-			-	-	
Stage 2	-	-			-	-	
Critical Hdwy	-	6.94			-	-	
Critical Hdwy Stg 1	-	-			-	-	
Critical Hdwy Stg 2	-	-			-	-	
Follow-up Hdwy	-	3.32			-	-	
Pot Cap-1 Maneuver	0	954			-	-	
Stage 1	0	-			-	-	
Stage 2	0	-			-	-	
Platoon blocked, %					-	-	
Mov Cap-1 Maneuver		954			-	-	
Mov Cap-2 Maneuver	-	-			-	-	
Stage 1	-	-			-	-	
Stage 2	-	-			-	-	
Approach	EB				SB		
HCM Control Delay, s					0		
HCM LOS	0.0 A				0		
	7						
Minor Lane/Major Mvr	nt E	BLn1	SBT	SBR			

Capacity (veh/h)	954	-	-
HCM Lane V/C Ratio	0.018	-	-
HCM Control Delay (s)	8.8	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0.1	-	-

5.2

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	1	÷				1		- 11	1				
Traffic Vol, veh/h	13	35	0	0	0	142	0	154	9	0	0	0	
Future Vol, veh/h	13	35	0	0	0	142	0	154	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	0	-	-	-	-	0	-	-	50	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	16979	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	14	38	0	0	0	154	0	167	10	0	0	0	

Major/Minor	Minor2		М	inor1		N	lajor1			
Conflicting Flow All	84	177	-	-	-	84	-	0	0	
Stage 1	0	0	-	-	-	-	-	-	-	
Stage 2	84	177	-	-	-	-	-	-	-	
Critical Hdwy	7.54	6.54	-	-	-	6.94	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.54	5.54	-	-	-	-	-	-	-	
Follow-up Hdwy	3.52	4.02	-	-	-	3.32	-	-	-	
Pot Cap-1 Maneuver	893	716	0	0	0	958	0	-	-	
Stage 1	-	-	0	0	0	-	0	-	-	
Stage 2	915	752	0	0	0	-	0	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	749	716	-	-	-	958	-	-	-	
Mov Cap-2 Maneuver	749	716	-	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	-	-	-	
Stage 2	768	752	-	-	-	-	-	-	-	
Approach	EB			WB			NB			
HCM Control Delay, s	10.2			9.5			0			

HCM LOS	В	А

Minor Lane/Major Mvmt	NBT	NBR I	EBLn1	EBLn2V	WBLn1
Capacity (veh/h)	-	-	749	719	958
HCM Lane V/C Ratio	-	-	0.013	0.059	0.161
HCM Control Delay (s)	-	-	9.9	10.3	9.5
HCM Lane LOS	-	-	А	В	Α
HCM 95th %tile Q(veh)	-	-	0	0.2	0.6

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Movement	EBT	EBR	WBL	WBT	NBL	NBR				
Lane Configurations	<u> </u>			^		11				
Traffic Volume (veh/h)	2377	0	0	2456	0	221				
Future Volume (Veh/h)	2377	0	0	2456	0	221				
Sign Control	Free			Free	Stop					
Grade	0%			0%	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92				
Hourly flow rate (vph)	2584	0	0	2670	0	240				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type	None			Raised						
Median storage veh)				1						
Upstream signal (ft)	313									
pX, platoon unblocked										
vC, conflicting volume			2584		3474	861				
vC1, stage 1 conf vol					2584					
vC2, stage 2 conf vol					890					
vCu, unblocked vol			2584		3474	861				
tC, single (s)			4.1		6.8	6.9				
tC, 2 stage (s)					5.8					
tF (s)			2.2		3.5	3.3				
p0 queue free %			100		100	20				
cM capacity (veh/h)			166		35	299				
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	NB 2		
Volume Total	861	861	861	890	890	890	120	120		
Volume Left	0	0	0	0	0	0	0	0		
Volume Right	0	0	0	0	0	0	120	120		
cSH	1700	1700	1700	1700	1700	1700	299	299		
Volume to Capacity	0.51	0.51	0.51	0.52	0.52	0.52	0.40	0.40		
Queue Length 95th (ft)	0	0	0	0	0	0	47	47		
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	24.9	24.9		
Lane LOS							С	С		
Approach Delay (s)	0.0			0.0			24.9			
Approach LOS							С			
Intersection Summary										
Average Delay			1.1							
Intersection Capacity Utilizati	on		57.6%	IC	CU Level of	of Service			В	
Analysis Period (min)			15							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u>_</u>	1						77	ľ	ę	
Traffic Volume (vph)	0	2157	148	0	0	0	0	0	0	158	637	0
Future Volume (vph)	0	2157	148	0	0	0	0	0	0	158	637	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4							5.4	5.4	
Lane Util. Factor		0.91	1.00							0.95	0.95	
Frt		1.00	0.85							1.00	1.00	
Flt Protected		1.00	1.00							0.95	1.00	
Satd. Flow (prot)		5301	1650							1752	1842	
Flt Permitted		1.00	1.00							0.95	1.00	
Satd. Flow (perm)		5301	1650							1752	1842	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2345	161	0	0	0	0	0	0	172	692	0
RTOR Reduction (vph)	0	0	48	0	0	0	0	0	0	74	47	0
Lane Group Flow (vph)	0	2345	113	0	0	0	0	0	0	81	662	0
Turn Type		NA	Perm						Perm	Split	NA	
Protected Phases		2								3	3	
Permitted Phases			2						4			
Actuated Green, G (s)		68.1	68.1							41.1	41.1	
Effective Green, g (s)		68.1	68.1							41.1	41.1	
Actuated g/C Ratio		0.57	0.57							0.34	0.34	
Clearance Time (s)		5.4	5.4							5.4	5.4	
Vehicle Extension (s)		3.0	3.0							3.0	3.0	
Lane Grp Cap (vph)		3008	936							600	630	
v/s Ratio Prot		c0.44								0.05	c0.36	
v/s Ratio Perm			0.07									
v/c Ratio		0.78	0.12							0.14	1.05	
Uniform Delay, d1		20.1	12.1							27.2	39.5	
Progression Factor		0.68	0.95							1.00	1.00	
Incremental Delay, d2		0.8	0.1							0.1	50.1	
Delay (s)		14.4	11.6							27.3	89.5	
Level of Service		В	В							С	F	
Approach Delay (s)		14.3			0.0			0.0			78.4	
Approach LOS		В			А			А			Е	
Intersection Summary												
HCM 2000 Control Delay			30.7	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	y ratio		0.93									
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			16.2			
Intersection Capacity Utilization	n		103.5%	IC	U Level o	of Service			G			
Analysis Period (min)			15									
o Critical Lano Group												

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ተተኈ					111	
Traffic Volume (vph)	2066	242	0	0	0	126	
Future Volume (vph)	2066	242	0	0	0	126	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.98					0.85	
FIt Protected	1.00					1.00	
Satd. Flow (prot)	5218					3763	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5218					3763	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	2246	263	0	0	0	137	
RTOR Reduction (vph)	8	0	0	0	0	42	
Lane Group Flow (vph)	2501	0	0	0	0	95	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	107.3					10.1	
Effective Green, g (s)	107.3					10.1	
Actuated g/C Ratio	0.83					0.08	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4306					292	
v/s Ratio Prot	c0.48					c0.03	
v/s Ratio Perm							
v/c Ratio	0.58					0.32	
Uniform Delay, d1	3.8					56.7	
Progression Factor	1.00					1.00	
Incremental Delay, d2	0.6					0.6	
Delay (s)	4.4					57.4	
Level of Service	А					E	
Approach Delay (s)	4.4			0.0	57.4		
Approach LOS	А			А	Е		
Intersection Summary							
HCM 2000 Control Delay			7.1	H	CM 2000	Level of Service	
HCM 2000 Volume to Cap	acity ratio		0.56				
Actuated Cycle Length (s)			130.0	Si	um of lost	t time (s)	
Intersection Capacity Utiliz	zation		59.4%	IC	U Level o	of Service	
Analysis Period (min)			15				
a Critical Lana Crown							

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations			1102	^	ኘኘ	11BIX		
Traffic Volume (vph)	0	0	0	3793	96	0		
Future Volume (vph)	0	0	0	3793	96	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	0.97			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5301	3579			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5301	3579			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.02	0.02	0.02	4123	104	0		
RTOR Reduction (vph)	0 0	0	0	0	2	0		
Lane Group Flow (vph)	0	0	0	4123	102	0		
Turn Type	-	-		NA	Prot	-		
Protected Phases				6	8			
Permitted Phases				•	Ū			
Actuated Green, G (s)				109.2	8.2			
Effective Green, g (s)				109.2	8.2			
Actuated g/C Ratio				0.84	0.06			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				4452	225			
v/s Ratio Prot				c0.78	c0.03			
v/s Ratio Perm								
v/c Ratio				0.93	0.45			
Uniform Delay, d1				7.5	58.7			
Progression Factor				0.33	1.23			
Incremental Delay, d2				3.4	1.2			
Delay (s)				5.9	73.2			
Level of Service				А	Е			
Approach Delay (s)	0.0			5.9	73.2			
Approach LOS	А			А	E			
Intersection Summary								
HCM 2000 Control Delay			7.5	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capa	acity ratio		0.89					
Actuated Cycle Length (s)	·		130.0	S	um of lost	time (s)	12.6	
Intersection Capacity Utilization	ation		109.3%		CU Level c		Н	
Analysis Period (min)			15					
o Critical Lano Group								

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Movement	EBL	EBT	WBT	WBR	SWL	SWR	
Lane Configurations			<u>_</u>			11	
Traffic Volume (vph)	0	0	2923	0	0	870	
Future Volume (vph)	0	0	2923	0	0	870	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)			4.0			6.0	
Lane Util. Factor			0.91			*0.95	
Frt			1.00			1.00	
Flt Protected			1.00			1.00	
Satd. Flow (prot)			5301			3689	
Flt Permitted			1.00			1.00	
Satd. Flow (perm)			5301			3689	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	3177	0	0	946	
RTOR Reduction (vph)	0	0	0	0	0	0	
Lane Group Flow (vph)	0	0	3177	0	0	946	
Turn Type			NA			Prot	
Protected Phases			Free!			4!	
Permitted Phases							
Actuated Green, G (s)			130.0			39.0	
Effective Green, g (s)			130.0			39.0	
Actuated g/C Ratio			1.00			0.30	
Clearance Time (s)						6.0	
Lane Grp Cap (vph)			5301			1106	
v/s Ratio Prot			0.60			c0.26	
v/s Ratio Perm							
v/c Ratio			0.60			0.86	
Uniform Delay, d1			0.0			42.8	
Progression Factor			1.00			1.00	
Incremental Delay, d2			0.1			8.5	
Delay (s)			0.1			51.4	
Level of Service			A			D	
Approach Delay (s)		0.0	0.1		51.4	-	
Approach LOS		A	A		D		
Intersection Summary							
HCM 2000 Control Delay			11.9	H	CM 2000	Level of Service	
HCM 2000 Volume to Capacity	ratio		0.71		2		
Actuated Cycle Length (s)			130.0	Si	um of lost	time (s)	
Intersection Capacity Utilization	1		90.9%			of Service	
Analysis Period (min)	-		15	.0			
Phase conflict between lane	arouns		10				
	groupo						

Timing Plan: AM Peak

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		1111									^	
Traffic Volume (vph)	0	1526	0	0	0	0	0	0	0	0	2923	0
Future Volume (vph)	0	1526	0	0	0	0	0	0	0	0	2923	0
	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.86									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		6680									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		6680									5301	
· · · /	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1659	0	0	0	0	0	0	0	0	3177	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1659	0	0	0	0	0	0	0	0	3177	0
Turn Type		NA				-					NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		39.0									79.0	
Effective Green, g (s)		39.0									79.0	
Actuated g/C Ratio		0.30									0.61	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2004									3221	
v/s Ratio Prot		c0.25									c0.60	
v/s Ratio Perm												
v/c Ratio		0.83									0.99	
Uniform Delay, d1		42.4									25.0	
Progression Factor		1.09									1.21	
Incremental Delay, d2		3.4									12.0	
Delay (s)		49.6									42.2	
Level of Service		D									D	
Approach Delay (s)		49.6			0.0			0.0			42.2	
Approach LOS		D			А			А			D	
Intersection Summary												
HCM 2000 Control Delay			44.8	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		0.93									
Actuated Cycle Length (s)			130.0		um of lost				12.0			
Intersection Capacity Utilization			90.9%	IC	CU Level of	of Service			E			
Analysis Period (min)			15									
Description: West side												
 Critical Lana Group 												

-	>	+	t	×	¢	4		
Movement	EBL	EBT	WBT	WBR	SEL	SER		
Lane Configurations		1111			ኘኘ			
Traffic Volume (vph)	0	1526	0	0	840	0		
Future Volume (vph)	0	1526	0	0	840	0		
	2000	2000	2000	2000	2000	2000		
Total Lost time (s)		4.0			6.0			
Lane Util. Factor		0.86			*0.95			
Frt		1.00			1.00			
Flt Protected		1.00			1.00			
Satd. Flow (prot)		6680			3689			
Flt Permitted		1.00			1.00			
Satd. Flow (perm)		6680			3689			
0 /	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.52	1659	0.52	0.52	913	0.32		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	0	1659	0	0	913	0		
Turn Type	0	NA	0	0	Prot	v		_
Protected Phases		Free!			2!			
Permitted Phases		Tiee:			Ζ:			
Actuated Green, G (s)		130.0			79.0			
Effective Green, g (s)		130.0			79.0			
Actuated g/C Ratio		1.00			0.61			
Clearance Time (s)		1.00			6.0			
		6680			2241			_
Lane Grp Cap (vph)								
v/s Ratio Prot		0.25			c0.25			
v/s Ratio Perm		0.05			0.44			
v/c Ratio		0.25			0.41			
Uniform Delay, d1		0.0			13.3			
Progression Factor		1.00			1.00			
Incremental Delay, d2		0.0			0.6			
Delay (s)		0.0			13.8			
Level of Service		A	0.0		12 B			
Approach Delay (s)		0.0	0.0		13.8			
Approach LOS		A	A		В			
Intersection Summary								
HCM 2000 Control Delay			4.9	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capacity ra	atio		0.37					
Actuated Cycle Length (s)			130.0		um of lost		12.0	
Intersection Capacity Utilization			54.5%	IC	U Level o	of Service	А	
Analysis Period (min)			15					
Phase conflict between lane g	roups.							
- Oritical Lana Oracia								

4 ≁ 5 1 WBL NWR Movement EBT EBR WBT NWL Lane Configurations ******* ኘኘ Traffic Volume (vph) 0 0 0 2023 1040 0 Future Volume (vph) 0 0 0 2023 1040 0 Ideal Flow (vphpl) 2000 2000 2000 2000 2000 2000 Total Lost time (s) 4.0 6.0 Lane Util. Factor 0.91 *0.95 Frt 1.00 1.00 1.00 Flt Protected 1.00 5301 Satd. Flow (prot) 3689 Flt Permitted 1.00 1.00 Satd. Flow (perm) 5301 3689 0.92 0.92 Peak-hour factor, PHF 0.92 0.92 0.92 0.92 Adj. Flow (vph) 0 0 0 2199 1130 0 RTOR Reduction (vph) 0 0 0 0 0 0 Lane Group Flow (vph) 0 0 0 2199 1130 0 Turn Type NA Prot Protected Phases Free! 4! Permitted Phases Actuated Green, G (s) 130.0 60.0 130.0 Effective Green, g (s) 60.0 Actuated g/C Ratio 1.00 0.46 Clearance Time (s) 6.0 Lane Grp Cap (vph) 5301 1702 v/s Ratio Prot 0.41 c0.31 v/s Ratio Perm v/c Ratio 0.41 0.66 Uniform Delay, d1 0.0 27.2 Progression Factor 1.00 1.00 Incremental Delay, d2 0.1 2.1 Delay (s) 0.1 29.2 Level of Service С А 0.0 29.2 Approach Delay (s) 0.1 Approach LOS А А С Intersection Summary HCM 2000 Control Delay 10.0 HCM 2000 Level of Service А HCM 2000 Volume to Capacity ratio 0.56 Actuated Cycle Length (s) 130.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 73.7% ICU Level of Service D Analysis Period (min) 15 Phase conflict between lane groups. ļ

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		^									^	
Traffic Volume (vph)	0	1996	0	0	0	0	0	0	0	0	2023	0
Future Volume (vph)	0	1996	0	0	0	0	0	0	0	0	2023	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.91									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		5301									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		5301									5301	
	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2170	0	0	0	0	0	0	0	0	2199	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2170	0	0	0	0	0	0	0	0	2199	0
Turn Type		NA							-		NA	
Protected Phases		4									2	
Permitted Phases											-	
Actuated Green, G (s)		60.0									58.0	
Effective Green, g (s)		60.0									58.0	
Actuated g/C Ratio		0.46									0.45	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2446									2365	
v/s Ratio Prot		c0.41									c0.41	
v/s Ratio Perm												
v/c Ratio		0.89									0.93	
Uniform Delay, d1		31.9									34.1	
Progression Factor		0.68									0.48	
Incremental Delay, d2		5.1									3.3	
Delay (s)		26.8									19.6	
Level of Service		С									В	
Approach Delay (s)		26.8			0.0			0.0			19.6	
Approach LOS		С			A			A			В	
Intersection Summary												
HCM 2000 Control Delay			23.2	H	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity ra	atio		0.91									
Actuated Cycle Length (s)			130.0	Si	um of lost	time (s)			12.0			
Intersection Capacity Utilization			83.8%	IC	U Level o	of Service			E			
Analysis Period (min)			15									
Description: East side												
a Critical Lana Craun												

	→	7	*	-	•	/		
Movement	EBT	EBR	WBL	WBT	NEL	NER		
Lane Configurations	<u>†</u> ††					11		
Traffic Volume (vph)	1996	0	0	0	0	570		
Future Volume (vph)	1996	0	0	0	0	570		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)	4.0					6.0		
Lane Util. Factor	0.91					0.88		
Frt	1.00					1.00		
Flt Protected	1.00					1.00		
Satd. Flow (prot)	5301					3417		
Flt Permitted	1.00					1.00		
Satd. Flow (perm)	5301					3417		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	2170	0.52	0.52	0.52	0.52	620		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	2170	0	0	0	0	620		
Turn Type	NA	0	0	0	0	Prot		
Protected Phases	Free!					2!		
Permitted Phases	1166:					۷:		
Actuated Green, G (s)	130.0					58.0		
Effective Green, g (s)	130.0					58.0		
Actuated g/C Ratio	1.00					0.45		
Clearance Time (s)	1.00					6.0		
Lane Grp Cap (vph)	5301					1524		
v/s Ratio Prot	0.41					0.18		
v/s Ratio Perm	0.41					0.10		
v/c Ratio	0.41					0.41		
Uniform Delay, d1	0.41					24.4		
Progression Factor	1.00					1.00		
Incremental Delay, d2	0.1					0.8		
Delay (s)	0.1					25.2		
Level of Service	0.1 A					23.2 C		
Approach Delay (s)	0.1			0.0	25.2	0		
Approach LOS	0.1 A			0.0 A	23.2 C			
Approach 200	~			~	U			
Intersection Summary								
HCM 2000 Control Delay			5.7	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capa	acity ratio		0.45					
Actuated Cycle Length (s)			130.0		um of lost		12.0	
Intersection Capacity Utiliza	ation		83.8%	IC	U Level o	of Service	E	
Analysis Period (min)			15					
Phase conflict between	lane groups.							

Intersection

Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	≜ ≜¢					1
Traffic Vol, veh/h	2308	7	0	0	0	0
Future Vol, veh/h	2308	7	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e, # 0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	2509	8	0	0	0	0

Major/Minor	Major1		Minor1		
Conflicting Flow All	0	0	-	1259	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	-	-	7.16	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	-	-	3.93	
Pot Cap-1 Maneuver	-	-	0	138	
Stage 1	-	-	0	-	
Stage 2	-	-	0	-	
Platoon blocked, %	-	-			
Mov Cap-1 Maneuver	r -	-	-	138	
Mov Cap-2 Maneuver	r -	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	

Approach	EB	NB	
HCM Control Delay, s	0	0	
HCM LOS		А	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	-	-	-
HCM Lane V/C Ratio	-	-	-
HCM Control Delay (s)	0	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	-	-	-

Intersection

Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			- † 1-	
Traffic Vol, veh/h	0	1	0	0	238	4
Future Vol, veh/h	0	1	0	0	238	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	1	0	0	259	4

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	132	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.96	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	3.33	-	-	
Pot Cap-1 Maneuver	0	890	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuve		890	-	-	
Mov Cap-2 Maneuve	r -	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	

Approach	EB	SB
HCM Control Delay, s	9.1	0
HCM LOS	А	

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	890	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	9.1	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Intersection

Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1				
Traffic Vol, veh/h	0	1	0	0	147	7
Future Vol, veh/h	0	1	0	0	147	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	1	0	0	160	8

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	84	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.96	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	3.33	-	-	
Pot Cap-1 Maneuver	0	955	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuve	r -	955	-	-	
Mov Cap-2 Maneuve	r -	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	

Approach	EB	SB	
HCM Control Delay, s	8.8	0	
HCM LOS	А		

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	955	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	8.8	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

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Intersection

Int Delay, s/veh

Movement EBL EBT EBR WBL WBT WBR NBL NBR SBL SBT SBR Lane Configurations Image: Configurations
Traffic Vol, veh/h 2 83 0 0 0 9 0 115 37 0 0 0 Future Vol, veh/h 2 83 0 0 0 9 0 115 37 0 0 0 Conflicting Peds, #/hr 0
Future Vol, veh/h 2 83 0 0 0 9 0 115 37 0 0 0 Conflicting Peds, #/hr 0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 Sign Control Stop Stop Stop Stop Stop Stop Stop Free Free Free Stop Stop Stop RT Channelized - - None - - None - - None Storage Length 0 - - 0 - 0 - - None Grade, % - 0 - 0 - - 0 - 0 - 0 - - 0 - - 0 - - 0 -
Sign ControlStopStopStopStopStopStopFreeFreeFreeFreeStopStopStopRT ChannelizedNoneNoneNone-NoneStorage Length0050Veh in Median Storage, #00-0-16979-Grade, %-0-0-0-0-0Peak Hour Factor92929292929292929292
RT Channelized - None - None - None - None Storage Length 0 - - 0 - 50 - - None Veh in Median Storage, # 0 - - 0 - - 0 - - 16979 - Grade, % - 0 - - 0 - - 0 - - 0 - Peak Hour Factor 92<
Storage Length 0 - - 0 - - 50 - - - Veh in Median Storage, # - 0 - - 0 - - 16979 - Grade, % - 0 - - 0 - - 0 - Peak Hour Factor 92 9
Veh in Median Storage, # - 0 - - 0 - - 16979 - Grade, % - 0 - - 0 - - 0 - - 0 - Peak Hour Factor 92
Grade, % - 0 0 0 0 0<
Peak Hour Factor 92 92 92 92 92 92 92 92 92 92 92 92 92
Heavy Vehicles, % 3 3 3 3 3 3 3 3 3 3 3 3 3 3
Mvmt Flow 2 90 0 0 0 10 0 125 40 0 0 0

Major/Minor	Minor2		Mir	nor1		М	ajor1			
Conflicting Flow All	63	165	-	-	-	63	-	0	0	
Stage 1	0	0	-	-	-	-	-	-	-	
Stage 2	63	165	-	-	-	-	-	-	-	
Critical Hdwy	7.56	6.56	-	-	-	6.96	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.56	5.56	-	-	-	-	-	-	-	
Follow-up Hdwy	3.53	4.03	-	-	-	3.33	-	-	-	
Pot Cap-1 Maneuver	921	724	0	0	0	985	0	-	-	
Stage 1	-	-	0	0	0	-	0	-	-	
Stage 2	938	758	0	0	0	-	0	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	· 912	724	-	-	-	985	-	-	-	
Mov Cap-2 Maneuver	· 912	724	-	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	-	-	-	
Stage 2	929	758	-	-	-	-	-	-	-	

Approach	EB	WB	NB	
HCM Control Delay, s	10.7	8.7	0	
HCM LOS	В	А		

Minor Lane/Major Mvmt	NBT	NBR B	EBLn1	EBLn2V	/BLn1
Capacity (veh/h)	-	-	912	725	985
HCM Lane V/C Ratio	-	-	0.002	0.125	0.01
HCM Control Delay (s)	-	-	9	10.7	8.7
HCM Lane LOS	-	-	А	В	А
HCM 95th %tile Q(veh)	-	-	0	0.4	0

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Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	4111					
Traffic Volume (veh/h)	1526	570	0	0	0	0
Future Volume (Veh/h)	1526	570	0	0	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1659	620	0	0	0	0
Pedestrians			, ,	·	·	•
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	NONG			Nono		
Upstream signal (ft)	585			197		
pX, platoon unblocked	505		0.89	131	0.89	0.89
vC, conflicting volume			2279		1969	725
vC1, stage 1 conf vol			2215		1303	125
vC2, stage 2 conf vol						
vCu, unblocked vol			1835		1488	95
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)			7.4		0.3	1.0
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			290		100	839
,					102	009
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	474	474	474	857		
Volume Left	0	0	0	0		
Volume Right	0	0	0	620		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.28	0.28	0.28	0.50		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	tion		84.7%	IC	Ulevelo	of Service
Analysis Period (min)			15			
			15			

	-	\mathbf{P}	*	-	5	/
Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations			<u> </u>	^		
Traffic Volume (veh/h)	0	0	140	2923	0	0
Future Volume (Veh/h)	0	0	140	2923	0	0
Sign Control	Free	v		Free	Stop	v
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0.02	0.02	152	3177	0.02	0.02
Pedestrians	Ŭ	Ū	102	0111	Ŭ	Ū
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	None			None		
Upstream signal (ft)	196			573		
pX, platoon unblocked	100			010		
vC, conflicting volume			0		1363	0
vC1, stage 1 conf vol			U		1000	U
vC2, stage 2 conf vol						
vCu, unblocked vol			0		1363	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)			7.4		0.0	7.0
tF (s)			2.2		3.5	3.3
p0 queue free %			91		100	100
cM capacity (veh/h)			1614		125	1081
Direction, Lane #	WB 1	WB 2	WB 3	WB 4	120	1001
Volume Total	152	1059	1059	1059		
Volume Left	152	0	0	0		
	0	0	0	0		
Volume Right cSH	1614	1700	1700	1700		
Volume to Capacity	0.09	0.62	0.62	0.62		
Queue Length 95th (ft)	0.09	0.02	0.02	0.02		
Control Delay (s)	7.5	0.0	0.0	0.0		
Lane LOS	7.5 A	0.0	0.0	0.0		
Approach Delay (s)	0.3					
Approach LOS	0.5					
••						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utiliz	zation		73.7%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	۲	^				
Traffic Volume (veh/h)	370	1996	0	0	0	0
Future Volume (Veh/h)	370	1996	0	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	402	2170	0	0	0	0
Pedestrians			-		-	-
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		473	280			
pX, platoon unblocked		110	200			
vC, conflicting volume	0				1527	0
vC1, stage 1 conf vol	0				1021	v
vC2, stage 2 conf vol						
vCu, unblocked vol	0				1527	0
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)	1.2				0.0	1.0
tF (s)	2.2				3.5	3.3
p0 queue free %	75				100	100
cM capacity (veh/h)	1614				80	1081
					00	1001
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	402	723	723	723		
Volume Left	402	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.25	0.43	0.43	0.43		
Queue Length 95th (ft)	25	0	0	0		
Control Delay (s)	8.0	0.0	0.0	0.0		
Lane LOS	А					
Approach Delay (s)	1.2					
Approach LOS						
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utiliz	zation		78.0%	IC	U Level o	of Service
Analysis Period (min)			15			
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Movement	EBL	EBT	WBT	WBR	SEL	SER
Lane Configurations			^	1		
Traffic Volume (veh/h)	0	0	2023	590	0	0
Future Volume (Veh/h)	0	0	2023	590	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	2199	641	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		250				
pX, platoon unblocked						
vC, conflicting volume	2840				2199	733
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2840				2199	733
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	129				38	361
		WB 2				•••
Direction, Lane #	WB 1		WB 3	WB 4		
Volume Total	733	733	733	641		
Volume Left	0	0	0	0		
Volume Right	0	0	0	641		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.43	0.43	0.43	0.38		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		82.1%	IC	U Level c	of Service
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis 1001: EB Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u> </u>	1						77	ሻሻ		
Traffic Volume (vph)	0	2674	181	0	0	0	0	0	439	533	0	0
Future Volume (vph)	0	2674	181	0	0	0	0	0	439	533	0	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4						5.4	5.4		
Lane Util. Factor		0.91	1.00						0.88	0.97		
Frt		1.00	0.85						0.85	1.00		
Flt Protected		1.00	1.00						1.00	0.95		
Satd. Flow (prot)		5301	1650						2905	3579		
Flt Permitted		1.00	1.00						1.00	0.95		
Satd. Flow (perm)		5301	1650						2905	3579		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2907	197	0	0	0	0	0	477	579	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2907	197	0	0	0	0	0	477	579	0	0
Turn Type		NA	Perm						Perm	Prot		
Protected Phases		2								3		
Permitted Phases			2						4			
Actuated Green, G (s)		68.6	68.6						17.6	17.6		
Effective Green, g (s)		68.6	68.6						17.6	17.6		
Actuated g/C Ratio		0.57	0.57						0.15	0.15		
Clearance Time (s)		5.4	5.4						5.4	5.4		
Vehicle Extension (s)		3.0	3.0						3.0	3.0		
Lane Grp Cap (vph)		3030	943						426	524		
v/s Ratio Prot		c0.55								c0.16		
v/s Ratio Perm			0.12						c0.16			
v/c Ratio		0.96	0.21						1.12	1.10		
Uniform Delay, d1		24.4	12.5						51.2	51.2		
Progression Factor		0.40	0.53						1.00	1.00		
Incremental Delay, d2		1.2	0.0						80.4	71.2		
Delay (s)		11.1	6.7						131.6	122.4		
Level of Service		В	А						F	F		
Approach Delay (s)		10.8			0.0			131.6			122.4	
Approach LOS		В			A			F			F	
Intersection Summary												
HCM 2000 Control Delay			40.2	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		1.01									
Actuated Cycle Length (s)			120.0	Si	um of lost	time (s)			16.2			
Intersection Capacity Utilization			125.7%			of Service			Н			
Analysis Period (min)			15									
c Critical Lane Group												

09/17/2019

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	† †Ъ	LBIX				777	
Traffic Volume (vph)	3435	211	0	0	0	325	
Future Volume (vph)	3435	211	0	0	0	325	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.99					0.85	
Flt Protected	1.00					1.00	
Satd. Flow (prot)	5255					3763	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5255					3763	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	3734	229	0.02	0.02	0.02	353	
RTOR Reduction (vph)	4	0	0	0	0	1	
Lane Group Flow (vph)	3959	0	0	0	0	352	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	109.0					18.4	
Effective Green, g (s)	109.0					18.4	
Actuated g/C Ratio	0.78					0.13	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4091					494	
v/s Ratio Prot	c0.75					c0.09	
v/s Ratio Perm							
v/c Ratio	0.97					0.71	
Uniform Delay, d1	13.9					58.3	
Progression Factor	1.00					1.00	
Incremental Delay, d2	8.4					4.8	
Delay (s)	22.3					63.1	
Level of Service	С					E	
Approach Delay (s)	22.3			0.0	63.1		
Approach LOS	С			А	E		
Intersection Summary							
HCM 2000 Control Delay			25.6	H	CM 2000	Level of Service	
HCM 2000 Volume to Capa	acity ratio		0.93				
Actuated Cycle Length (s)			140.0	Si	um of lost	t time (s)	
Intersection Capacity Utiliz	ation		85.2%			of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				^	ኘ			
Traffic Volume (vph)	0	0	0	2907	407	0		
Future Volume (vph)	0	0	0	2907	407	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	*1.00			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5301	3689			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5301	3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.02	0.02	0.02	3160	442	0		
RTOR Reduction (vph)	Ŭ Ŭ	0	Ũ	0	5	0		
Lane Group Flow (vph)	0	0	0	3160	437	0		
Turn Type				NA	Prot			
Protected Phases				6	8			
Permitted Phases				Ű	Ū			
Actuated Green, G (s)				106.3	21.1			
Effective Green, g (s)				106.3	21.1			
Actuated g/C Ratio				0.76	0.15			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				4024	555			
v/s Ratio Prot				c0.60	c0.12			
v/s Ratio Perm				00.00	00.12			
v/c Ratio				0.79	0.79			
Uniform Delay, d1				10.0	57.3			
Progression Factor				0.41	0.99			
Incremental Delay, d2				1.5	2.9			
Delay (s)				5.6	59.7			
Level of Service				0.0 A	55.7 E			
Approach Delay (s)	0.0			5.6	59.7			
Approach LOS	A			A	E			
Intersection Summary								
HCM 2000 Control Delay			12.2	Н	CM 2000	Level of Service	В	
HCM 2000 Volume to Capacit	tv ratio		0.79					
Actuated Cycle Length (s)	.,		140.0	S	um of lost	time (s)	12.6	
Intersection Capacity Utilization	on		115.5%		U Level o		H	
Analysis Period (min)			15		5 _5767 0			
c Critical Lane Group								

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Movement	EBL	EBT	WBT	WBR	SWL	SWR			
Lane Configurations			^		•••=	11			
Traffic Volume (vph)	0	0	2367	0	0	540			
Future Volume (vph)	0	0	2367	0	0	540			
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000			
Total Lost time (s)			4.0			6.0			
Lane Util. Factor			0.91			*0.95			
Frt			1.00			1.00			
-It Protected			1.00			1.00			
Satd. Flow (prot)			5301			3689			
-It Permitted			1.00			1.00			
Satd. Flow (perm)			5301			3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92			
Adj. Flow (vph)	0	0	2573	0	0	587			
RTOR Reduction (vph)	0	0	0	0	0	0			
ane Group Flow (vph)	0	0	2573	0	0	587			
Furn Type			NA	-		Prot			
Protected Phases			Free!			4!			
ermitted Phases									
ctuated Green, G (s)			140.0			61.0			
ffective Green, g (s)			140.0			61.0			
ctuated g/C Ratio			1.00			0.44			
Clearance Time (s)						6.0			
ane Grp Cap (vph)			5301			1607			
's Ratio Prot			0.49			0.16			
/s Ratio Perm									
c Ratio			0.49			0.37			
niform Delay, d1			0.0			26.5			
Progression Factor			1.00			1.00			
ncremental Delay, d2			0.0			0.6			
Delay (s)			0.0			27.2			
evel of Service			А			С			
Approach Delay (s)		0.0	0.0		27.2				
pproach LOS		А	А		С				
ntersection Summary									
ICM 2000 Control Delay			5.1	H	CM 2000	Level of Service)	А	
ICM 2000 Volume to Capaci	ity ratio		0.53						
ctuated Cycle Length (s)			140.0	Si	um of lost	time (s)		12.0	
ntersection Capacity Utilization	on		86.8%			of Service		Е	
Analysis Period (min)			15						
Phase conflict between lar	ne groups.								

HCM Signalized Intersection Capacity Analysis 1005: WB Big Beaver Rd & EB Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		1111									^	
Traffic Volume (vph)	0	2423	0	0	0	0	0	0	0	0	2367	0
Future Volume (vph)	0	2423	0	0	0	0	0	0	0	0	2367	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.86									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		6680									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		6680									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2634	0	0	0	0	0	0	0	0	2573	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2634	0	0	0	0	0	0	0	0	2573	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		61.0									67.0	
Effective Green, g (s)		61.0									67.0	
Actuated g/C Ratio		0.44									0.48	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2910									2536	
v/s Ratio Prot		c0.39									c0.49	
v/s Ratio Perm												
v/c Ratio		0.91									1.01	
Uniform Delay, d1		36.8									36.5	
Progression Factor		0.89									1.46	
Incremental Delay, d2		2.1									21.1	
Delay (s)		34.8									74.4	
Level of Service		С									E	
Approach Delay (s)		34.8			0.0			0.0			74.4	
Approach LOS		С			А			А			E	
Intersection Summary												
HCM 2000 Control Delay			54.3	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		0.96									
Actuated Cycle Length (s)			140.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilization			86.8%	IC	U Level o	of Service			Е			
Analysis Period (min)			15									
Description: West side												
o Critical Lano Group												

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Movement	EBL	EBT	WBT	WBR	SEL	SER		
Lane Configurations		tttt			ኘካ			
Traffic Volume (vph)	0	2423	0	0	800	0		
Future Volume (vph)	0	2423	0	0	800	0		
(1)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)		4.0			6.0			
Lane Util. Factor		0.86			*0.95			
Frt		1.00			1.00			
Flt Protected		1.00			1.00			
Satd. Flow (prot)		6680			3689			
Flt Permitted		1.00			1.00			
Satd. Flow (perm)		6680			3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	2634	0	0	870	0		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	0	2634	0	0	870	0		
Turn Type		NA			Prot			
Protected Phases		Free!			2!			
Permitted Phases								
Actuated Green, G (s)		140.0			67.0			
Effective Green, g (s)		140.0			67.0			
Actuated g/C Ratio		1.00			0.48			
Clearance Time (s)					6.0			
Lane Grp Cap (vph)		6680			1765			
v/s Ratio Prot		0.39			c0.24			
v/s Ratio Perm								
v/c Ratio		0.39			0.49			
Uniform Delay, d1		0.0			24.9			
Progression Factor		1.00			1.00			
Incremental Delay, d2		0.1			1.0			
Delay (s)		0.1			25.9			
Level of Service		А			С			
Approach Delay (s)		0.1	0.0		25.9			
Approach LOS		А	А		С			
Intersection Summary								
HCM 2000 Control Delay			6.5	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capacity	ratio		0.46					
Actuated Cycle Length (s)			140.0		um of lost		12.0	
Intersection Capacity Utilization			63.4%	IC	U Level o	of Service	В	
Analysis Period (min)			15					
! Phase conflict between lane	groups							
 Critical Lana Group 								

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Movement	EBT	EBR	WBL	WBT	NWL	NWR		
Lane Configurations				† ††	ኘኘ			
Traffic Volume (vph)	0	0	0	1847	690	0		
Future Volume (vph)	0	0	0	1847	690	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				4.0	6.0			
Lane Util. Factor				0.91	*0.95			
Frt				1.00	1.00			
Flt Protected				1.00	1.00			
Satd. Flow (prot)				5301	3689			
Flt Permitted				1.00	1.00			
Satd. Flow (perm)				5301	3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	0	0	2008	750	0		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	0	0	0	2008	750	0		
Turn Type				NA	Prot			
Protected Phases				Free!	4!			
Permitted Phases								
Actuated Green, G (s)				140.0	78.0			
Effective Green, g (s)				140.0	78.0			
Actuated g/C Ratio				1.00	0.56			
Clearance Time (s)					6.0			
Lane Grp Cap (vph)				5301	2055			
v/s Ratio Prot				0.38	0.20			
v/s Ratio Perm								
v/c Ratio				0.38	0.36			
Uniform Delay, d1				0.0	17.2			
Progression Factor				1.00	1.00			
Incremental Delay, d2				0.0	0.5			
Delay (s)				0.0	17.7			
Level of Service				А	В			
Approach Delay (s)	0.0			0.0	17.7			
Approach LOS	А			А	В			
Intersection Summary								
HCM 2000 Control Delay			4.8	H	CM 2000	Level of Service	A	
HCM 2000 Volume to Capacity	ratio		0.41					
Actuated Cycle Length (s)			140.0	Si	um of lost	t time (s)	12.0	
Intersection Capacity Utilization	1		60.9%			of Service	В	
Analysis Period (min)			15					
Phase conflict between lane	groups							
c Critical Lane Group	0 10							

HCM Signalized Intersection Capacity Analysis 1008: WB Big Beaver Rd & EB Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		<u></u>									***	
Traffic Volume (vph)	0	2473	0	0	0	0	0	0	0	0	1847	0
Future Volume (vph)	0	2473	0	0	0	0	0	0	0	0	1847	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.91									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		5301									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		5301									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2688	0	0	0	0	0	0	0	0	2008	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2688	0	0	0	0	0	0	0	0	2008	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		78.0									50.0	
Effective Green, g (s)		78.0									50.0	
Actuated g/C Ratio		0.56									0.36	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2953									1893	
v/s Ratio Prot		c0.51									c0.38	
v/s Ratio Perm												
v/c Ratio		0.91									1.06	
Uniform Delay, d1		27.9									45.0	
Progression Factor		1.07									0.42	
Incremental Delay, d2		5.1									28.8	
Delay (s)		34.8									47.6	
Level of Service		С									D	
Approach Delay (s)		34.8			0.0			0.0			47.6	
Approach LOS		С			Α			А			D	
Intersection Summary												
HCM 2000 Control Delay			40.3	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		0.97									
Actuated Cycle Length (s)			140.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilization			89.3%			of Service			Е			
Analysis Period (min)			15									
Description: East side												
 Critical Lana Group 												

	-	7	F	←	3	/		
Movement	EBT	EBR	WBL	WBT	NEL	NER		
Lane Configurations	† ††					11		
Traffic Volume (vph)	2473	0	0	0	0	240		
Future Volume (vph)	2473	0	0	0	0	240		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)	4.0					6.0		
Lane Util. Factor	0.91					0.88		
Frt	1.00					1.00		
Flt Protected	1.00					1.00		
Satd. Flow (prot)	5301					3417		
Flt Permitted	1.00					1.00		
Satd. Flow (perm)	5301					3417		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	Ī	
Adj. Flow (vph)	2688	0	0	0	0	261		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	2688	0	0	0	0	261		
Turn Type	NA					Prot		
Protected Phases	Free!					2!		
Permitted Phases								
Actuated Green, G (s)	140.0					50.0		
Effective Green, g (s)	140.0					50.0		
Actuated g/C Ratio	1.00					0.36		
Clearance Time (s)						6.0		
Lane Grp Cap (vph)	5301					1220		
v/s Ratio Prot	0.51					0.08		
v/s Ratio Perm								
v/c Ratio	0.51					0.21		
Uniform Delay, d1	0.0					31.3		
Progression Factor	1.00					1.00		
Incremental Delay, d2	0.1					0.4		
Delay (s)	0.1					31.7		
Level of Service	А					С		
Approach Delay (s)	0.1			0.0	31.7			
Approach LOS	А			А	С			
Intersection Summary								
HCM 2000 Control Delay			2.9	H	CM 2000	Level of Service		А
HCM 2000 Volume to Capa	acity ratio		0.55					
Actuated Cycle Length (s)			140.0		um of lost			12.0
Intersection Capacity Utilization	ation		89.3%	IC	U Level o	of Service		E
Analysis Period (min)			15					
Phase conflict between	lane groups.							
a Critical Lana Croup								

1.1						
Intersection						
Int Delay, s/veh	0.2					
	FDT					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u> ↑↑₽					1
Traffic Vol, veh/h	3639	7	0	0	0	7
Future Vol, veh/h	3639	7	0	0	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Stop	Stop	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e,# 0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	3955	8	0	0	0	8

Major/Minor	Major1		Minor1	
Conflicting Flow All	0	0	-	1982
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	-	-	7.16
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	-	-	3.93
Pot Cap-1 Maneuver	-	-	0	43
Stage 1	-	-	0	-
Stage 2	-	-	0	-
Platoon blocked, %	-	-		
Mov Cap-1 Maneuver	• -	-	-	43
Mov Cap-2 Maneuver	• -	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	NB	
HCM Control Delay, s	0	105.9	
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	43	-	-
HCM Lane V/C Ratio	0.177	-	-
HCM Control Delay (s)	105.9	-	-
HCM Lane LOS	F	-	-
HCM 95th %tile Q(veh)	0.6	-	-

Intersection	
Int Delay s/veh	

Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			- † 12	
Traffic Vol, veh/h	0	11	0	0	207	4
Future Vol, veh/h	0	11	0	0	207	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	,# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	12	0	0	225	4

Major/Minor	Minor2		Major2	
Conflicting Flow All	-	115	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	6.96	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	3.33	-	-
Pot Cap-1 Maneuver	0	912	-	-
Stage 1	0	-	-	-
Stage 2	0	-	-	-
Platoon blocked, %			-	-
Mov Cap-1 Maneuver		912	-	-
Mov Cap-2 Maneuver	· -	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Approach	EB		SB	
HCM Control Delay, s			0	
HCM LOS	A			

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	912	-	-
HCM Lane V/C Ratio	0.013	-	-
HCM Control Delay (s)	9	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Int	Dolay	. s/veh	
m	Delav	. s/ven	

Int Delay, s/veh	0.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			∱ î≽	
Traffic Vol, veh/h	0	17	0	0	160	7
Future Vol, veh/h	0	17	0	0	160	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	, # 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	18	0	0	174	8

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	91	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.96	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	3.33	-	-	
Pot Cap-1 Maneuver	0	945	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuver		945	-	-	
Mov Cap-2 Maneuver	r -	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Approach	EB		SB		
HCM Control Delay, s			0		
HCM LOS	A 0.9		0		
	A				

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	945	-	-
HCM Lane V/C Ratio	0.02	-	-
HCM Control Delay (s)	8.9	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0.1	-	-

5.3

Intersection

Int Delay, s/veh

5.													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦.	्र				1		- 11	1				
Traffic Vol, veh/h	14	37	0	0	0	149	0	162	9	0	0	0	
Future Vol, veh/h	14	37	0	0	0	149	0	162	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	0	-	-	-	-	0	-	-	50	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	16979	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3	
Mvmt Flow	15	40	0	0	0	162	0	176	10	0	0	0	

Major/Minor	Minor2		М	inor1		Ν	lajor1			
Conflicting Flow All	88	186	-	-	-	88	-	0	0	
Stage 1	0	0	-	-	-	-	-	-	-	
Stage 2	88	186	-	-	-	-	-	-	-	
Critical Hdwy	7.56	6.56	-	-	-	6.96	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.56	5.56	-	-	-	-	-	-	-	
Follow-up Hdwy	3.53	4.03	-	-	-	3.33	-	-	-	
Pot Cap-1 Maneuver	885	705	0	0	0	950	0	-	-	
Stage 1	-	-	0	0	0	-	0	-	-	
Stage 2	907	742	0	0	0	-	0	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	735	705	-	-	-	950	-	-	-	
Mov Cap-2 Maneuver	735	705	-	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	-	-	-	
Stage 2	752	742	-	-	-	-	-	-	-	
•				14/5						

Approach	EB	WB	NB	
HCM Control Delay, s	10.3	9.6	0	
HCM LOS	В	А		

Minor Lane/Major Mvmt	NBT	NBR I	EBLn1	EBLn2V	/BLn1
Capacity (veh/h)	-	-	735	708	950
HCM Lane V/C Ratio	-	-	0.014	0.064	0.17
HCM Control Delay (s)	-	-	10	10.4	9.6
HCM Lane LOS	-	-	В	В	А
HCM 95th %tile Q(veh)	-	-	0	0.2	0.6

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Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	4111					
Traffic Volume (veh/h)	2423	930	0	0	0	0
Future Volume (Veh/h)	2423	930	0	0	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2634	1011	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)	585			197		
pX, platoon unblocked			0.36		0.36	0.36
vC, conflicting volume			3645		3140	1164
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0		0	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)					0.0	
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			576		364	386
Direction, Lane #	EB 1	EB 2	EB 3	EB 4	2.	
Volume Total	753	753	753	1387		
Volume Left	755 0	155	155	1307		
Volume Right	0	0	0	1011		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.44	0.44	0.44	0.82		
. ,	0.44	0.44	0.44	0.82		
Queue Length 95th (ft)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	0.0					
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	ation		86.8%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations			5	^		
Traffic Volume (veh/h)	0	0	170	2367	0	0
Future Volume (Veh/h)	0	0	170	2367	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	185	2573	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	Nono			None		
Upstream signal (ft)	199			573		
pX, platoon unblocked	100			010		
vC, conflicting volume			0		1228	0
vC1, stage 1 conf vol			U		1220	U
vC2, stage 2 conf vol						
vCu, unblocked vol			0		1228	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)			7.4		0.0	1.0
tF (s)			2.2		3.5	3.3
p0 queue free %			89		100	100
cM capacity (veh/h)			1614		150	1081
					100	1001
Direction, Lane #	WB 1	WB 2	WB 3	WB 4		
Volume Total	185	858	858	858		
Volume Left	185	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.11	0.50	0.50	0.50		
Queue Length 95th (ft)	10	0	0	0		
Control Delay (s)	7.5	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.5					
Approach LOS						
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliz	zation		60.9%	IC	U Level o	of Service
Analysis Period (min)			15			
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Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	ሻ	<u></u>				
Traffic Volume (veh/h)	750	2473	0	0	0	0
Future Volume (Veh/h)	750	2473	0	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	815	2688	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	10110			
Upstream signal (ft)		473	281			
pX, platoon unblocked		10	201			
vC, conflicting volume	0				2526	0
vC1, stage 1 conf vol	U				2020	U
vC2, stage 2 conf vol						
vCu, unblocked vol	0				2526	0
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)	7.2				0.3	1.0
tF (s)	2.2				3.5	3.3
p0 queue free %	50				100	100
cM capacity (veh/h)	1614				11	1081
					11	1001
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	815	896	896	896		
Volume Left	815	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.50	0.53	0.53	0.53		
Queue Length 95th (ft)	74	0	0	0		
Control Delay (s)	9.5	0.0	0.0	0.0		
Lane LOS	А					
Approach Delay (s)	2.2					
Approach LOS						
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utiliza	ation		96.2%	IC	ULevelo	of Service
Analysis Period (min)			15	10	0 2010.0	
			15			

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Movement	EBL	EBT	WBT	WBR	SEL	SER
Lane Configurations			^	1		
Traffic Volume (veh/h)	0	0	1847	750	0	0
Future Volume (Veh/h)	0	0	1847	750	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	2008	815	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		252				
pX, platoon unblocked						
vC, conflicting volume	2823				2008	669
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2823				2008	669
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	131				51	398
Direction, Lane #	WB 1	WB 2	WB 3	WB 4		
Volume Total	669	669	669	815		
Volume Left	000	0	0	0		
Volume Right	0	0	0	815		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.39	0.39	0.39	0.48		
Queue Length 95th (ft)	0.00	0.00	0.00	0.40		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	0.0	0.0	0.0	0.0		
Approach Delay (s)	0.0					
Approach LOS	0.0					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		89.3%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u> </u>	1						77	ľ	ę	
Traffic Volume (vph)	0	2188	148	0	0	0	0	0	0	160	637	0
Future Volume (vph)	0	2188	148	0	0	0	0	0	0	160	637	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4							5.4	5.4	
Lane Util. Factor		0.91	1.00							0.95	0.95	
Frt		1.00	0.85							1.00	1.00	
Flt Protected		1.00	1.00							0.95	1.00	
Satd. Flow (prot)		5301	1650							1752	1842	
Flt Permitted		1.00	1.00							0.95	1.00	
Satd. Flow (perm)		5301	1650							1752	1842	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2378	161	0	0	0	0	0	0	174	692	0
RTOR Reduction (vph)	0	0	47	0	0	0	0	0	0	75	47	0
Lane Group Flow (vph)	0	2378	114	0	0	0	0	0	0	82	662	0
Turn Type		NA	Perm						Perm	Split	NA	
Protected Phases		2								3	3	
Permitted Phases			2						4			
Actuated Green, G (s)		68.1	68.1							41.1	41.1	
Effective Green, g (s)		68.1	68.1							41.1	41.1	
Actuated g/C Ratio		0.57	0.57							0.34	0.34	
Clearance Time (s)		5.4	5.4							5.4	5.4	
Vehicle Extension (s)		3.0	3.0							3.0	3.0	
Lane Grp Cap (vph)		3008	936							600	630	
v/s Ratio Prot		c0.45								0.05	c0.36	
v/s Ratio Perm			0.07									
v/c Ratio		0.79	0.12							0.14	1.05	
Uniform Delay, d1		20.4	12.1							27.2	39.5	
Progression Factor		0.68	0.99							1.00	1.00	
Incremental Delay, d2		0.9	0.1							0.1	50.1	
Delay (s)		14.8	12.1							27.3	89.5	
Level of Service		В	В							С	F	
Approach Delay (s)		14.6			0.0			0.0			78.2	
Approach LOS		В			А			А			Е	
Intersection Summary												
HCM 2000 Control Delay			30.8	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity	y ratio		0.93									
Actuated Cycle Length (s)			120.0	S	um of lost	time (s)			16.2			
Intersection Capacity Utilizatio	n		104.1%	IC	U Level o	of Service			G			
Analysis Period (min)			15									
 Critical Lana Group 												

	-	\mathbf{F}	4	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ተተኈ					111	
Traffic Volume (vph)	2076	259	0	0	0	136	
Future Volume (vph)	2076	259	0	0	0	136	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.98					0.85	
Flt Protected	1.00					1.00	
Satd. Flow (prot)	5213					3763	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5213					3763	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	2257	282	0	0	0	148	
RTOR Reduction (vph)	9	0	0	0	0	41	
Lane Group Flow (vph)	2530	0	0	0	0	107	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	107.1					10.3	
Effective Green, g (s)	107.1					10.3	
Actuated g/C Ratio	0.82					0.08	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4294					298	
v/s Ratio Prot	c0.49					c0.03	
v/s Ratio Perm							
v/c Ratio	0.59					0.36	
Uniform Delay, d1	3.9					56.7	
Progression Factor	1.00					1.00	
Incremental Delay, d2	0.6					0.7	
Delay (s)	4.5					57.5	
Level of Service	А					E	
Approach Delay (s)	4.5			0.0	57.5		
Approach LOS	А			А	Е		
Intersection Summary							
HCM 2000 Control Delay			7.4	H	CM 2000	Level of Service	
HCM 2000 Volume to Capa	acity ratio		0.57				
Actuated Cycle Length (s)			130.0	Si	um of lost	t time (s)	
Intersection Capacity Utiliz	ation		59.9%	IC	U Level o	of Service	
Analysis Period (min)			15				
a Critical Lana Croup							

	-	\mathbf{r}	1	+	1	1		
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				† ††	ኘካ			
Traffic Volume (vph)	0	0	0	3794	97	0		
Future Volume (vph)	0	0	0	3794	97	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	0.97			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5301	3579			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5301	3579			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	0	0	4124	105	0		
RTOR Reduction (vph)	0	0	0	0	2	0		
Lane Group Flow (vph)	0	0	0	4124	103	0		
Turn Type				NA	Prot			
Protected Phases				6	8			
Permitted Phases				-	-			
Actuated Green, G (s)				109.2	8.2			
Effective Green, g (s)				109.2	8.2			
Actuated g/C Ratio				0.84	0.06			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				4452	225			
v/s Ratio Prot				c0.78	c0.03			
v/s Ratio Perm								
v/c Ratio				0.93	0.46			
Uniform Delay, d1				7.5	58.8			
Progression Factor				0.33	1.22			
Incremental Delay, d2				3.4	1.2			
Delay (s)				5.9	72.8			
Level of Service				A	E			
Approach Delay (s)	0.0			5.9	72.8			
Approach LOS	A			A	E			
Intersection Summary								
HCM 2000 Control Delay			7.6	Н	CM 2000	Level of Service	А	
HCM 2000 Volume to Capa	acity ratio		0.89					
Actuated Cycle Length (s)			130.0	S	um of lost	time (s)	12.6	
Intersection Capacity Utiliza	ation		109.6%		CU Level c	()	Н	
Analysis Period (min)			15					
o Critical Lano Group								

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↑↑↑ řř 0 2924 0 0 870 0 2924 0 0 870 0 2924 0 0 200 0 2000 2000 2000 2000			EBL	Movement
0 2924 0 0 870 0 2924 0 0 870 0 2000 2000 2000 2000				
0 2924 0 0 870 0 2924 0 0 870 0 2000 2000 2000 2000	TTT			Lane Configurations
0 2000 2000 2000 2000			0	Traffic Volume (vph)
	2924 0	0	0	Future Volume (vph)
10 60	2000 2000 20	2000	2000	Ideal Flow (vphpl)
ч.0 0.0	4.0			Total Lost time (s)
0.91 *0.95				Lane Util. Factor
1.00 1.00	1.00			Frt
1.00 1.00	1.00			Flt Protected
5301 3689	5301	;		Satd. Flow (prot)
1.00 1.00	1.00			FIt Permitted
5301 3689	5301	;		Satd. Flow (perm)
2 0.92 0.92 0.92 0.92	0.92 0.92 0.	0.92	0.92	Peak-hour factor, PHF
0 3178 0 0 946	3178 0	0	0	Adj. Flow (vph)
0 0 0 0	0 0	0	0	RTOR Reduction (vph)
0 3178 0 0 946	3178 0	0	0	Lane Group Flow (vph)
NA Prot	NA			Turn Type
Free! 4!	Free!			Protected Phases
				Permitted Phases
130.0 39.0	130.0	1		Actuated Green, G (s)
130.0 39.0	130.0	1		Effective Green, g (s)
1.00 0.30	1.00			Actuated g/C Ratio
6.0				Clearance Time (s)
5301 1106	5301			Lane Grp Cap (vph)
0.60 c0.26	0.60			v/s Ratio Prot
				v/s Ratio Perm
0.60 0.86	0.60			v/c Ratio
0.0 42.8				Uniform Delay, d1
1.00 1.00	1.00			Progression Factor
0.1 8.5	0.1			Incremental Delay, d2
0.1 51.4	0.1			Delay (s)
A D	А			Level of Service
0 0.1 51.4	0.1 51	0.0		Approach Delay (s)
A A D	А	А		Approach LOS
				Intersection Summary
11.9 HCM 2000 Level of Service				HCM 2000 Control Delay
0.71			/ ratio	HCM 2000 Volume to Capaci
130.0 Sum of lost time (s)				Actuated Cycle Length (s)
90.9% ICU Level of Service		9	n	Intersection Capacity Utilization
15	15			Analysis Period (min)
			e groups.	Phase conflict between lar

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Beav	/er			Timing Plan: AM Peak						
*	`+	×	4	*	×	4				
WBR	SEL	SET	SER	NWL	NWT	NWR				
					111					

Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		1111									***	
Traffic Volume (vph)	0	1545	0	0	0	0	0	0	0	0	2924	0
Future Volume (vph)	0	1545	0	0	0	0	0	0	0	0	2924	0
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.86									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		6680									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		6680									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	1679	0	0	0	0	0	0	0	0	3178	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	1679	0	0	0	0	0	0	0	0	3178	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		39.0									79.0	
Effective Green, g (s)		39.0									79.0	
Actuated g/C Ratio		0.30									0.61	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2004									3221	
v/s Ratio Prot		c0.25									c0.60	
v/s Ratio Perm												
v/c Ratio		0.84									0.99	
Uniform Delay, d1		42.5									25.0	
Progression Factor		1.09									1.21	
Incremental Delay, d2		3.7									12.0	
Delay (s)		50.2									42.3	
Level of Service		D									D	
Approach Delay (s)		50.2			0.0			0.0			42.3	
Approach LOS		D			А			А			D	
Intersection Summary												
HCM 2000 Control Delay			45.0	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity r	atio		0.94									
Actuated Cycle Length (s)			130.0	S	um of lost	t time (s)			12.0			
Intersection Capacity Utilization			90.9%	IC	U Level o	of Service			E			
Analysis Period (min)			15									
Description: West side												
 Critical Lana Group 												

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Movement	EBL	EBT	WBT	WBR	SEL	SER		
Lane Configurations		1111			ኘኘ			
Traffic Volume (vph)	0	1545	0	0	840	0		
Future Volume (vph)	0	1545	0	0	840	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)		4.0			6.0			
Lane Util. Factor		0.86			*0.95			
Frt		1.00			1.00			
Flt Protected		1.00			1.00			
Satd. Flow (prot)		6680			3689			
Flt Permitted		1.00			1.00			
Satd. Flow (perm)		6680			3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	1679	0	0	913	0		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	0	1679	0	0	913	0		
Turn Type		NA			Prot			
Protected Phases		Free!			2!			
Permitted Phases								
Actuated Green, G (s)		130.0			79.0			
Effective Green, g (s)		130.0			79.0			
Actuated g/C Ratio		1.00			0.61			
Clearance Time (s)					6.0			
Lane Grp Cap (vph)		6680			2241			
v/s Ratio Prot		0.25			c0.25			
v/s Ratio Perm								
v/c Ratio		0.25			0.41			
Uniform Delay, d1		0.0			13.3			
Progression Factor		1.00			1.00			
Incremental Delay, d2		0.0			0.6			
Delay (s)		0.0			13.8			
Level of Service		А			В			
Approach Delay (s)		0.0	0.0		13.8			
Approach LOS		А	А		В			
Intersection Summary								
HCM 2000 Control Delay			4.9	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capacity ra	atio		0.37					
Actuated Cycle Length (s)			130.0		um of lost		12.0	
Intersection Capacity Utilization			54.5%	IC	U Level o	of Service	А	
Analysis Period (min)			15					
Phase conflict between lane g	groups							
 Critical Lana Crown 								

4 ≁ 5 24 WBL NWR Movement EBT EBR WBT NWL Lane Configurations *** ኘኘ Traffic Volume (vph) 0 0 0 2024 1040 0 Future Volume (vph) 0 0 0 2024 1040 0 Ideal Flow (vphpl) 2000 2000 2000 2000 2000 2000 Total Lost time (s) 4.0 6.0 Lane Util. Factor 0.91 *0.95 Frt 1.00 1.00 1.00 Flt Protected 1.00 5301 Satd. Flow (prot) 3689 Flt Permitted 1.00 1.00 Satd. Flow (perm) 5301 3689 0.92 0.92 Peak-hour factor, PHF 0.92 0.92 0.92 0.92 Adj. Flow (vph) 0 0 0 2200 1130 0 RTOR Reduction (vph) 0 0 0 0 0 0 Lane Group Flow (vph) 0 0 0 2200 1130 0 Turn Type NA Prot Protected Phases Free! 4! Permitted Phases Actuated Green, G (s) 130.0 60.0 130.0 Effective Green, g (s) 60.0 Actuated g/C Ratio 1.00 0.46 Clearance Time (s) 6.0 Lane Grp Cap (vph) 5301 1702 v/s Ratio Prot 0.42 c0.31 v/s Ratio Perm v/c Ratio 0.42 0.66 Uniform Delay, d1 0.0 27.2 Progression Factor 1.00 1.00 Incremental Delay, d2 0.1 2.1 Delay (s) 0.1 29.2 Level of Service С А 0.0 29.2 Approach Delay (s) 0.1 Approach LOS А А С Intersection Summary HCM 2000 Control Delay 10.0 HCM 2000 Level of Service А HCM 2000 Volume to Capacity ratio 0.56 Actuated Cycle Length (s) 130.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 73.7% ICU Level of Service D Analysis Period (min) 15 Phase conflict between lane groups. ļ

Timing	Plan:	AM Pea	ak

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		ተተተ									^	
Traffic Volume (vph)	0	2015	0	0	0	0	0	0	0	0	2024	0
Future Volume (vph)	0	2015	0	0	0	0	0	0	0	0	2024	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.91									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		5301									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		5301									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2190	0	0	0	0	0	0	0	0	2200	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2190	0	0	0	0	0	0	0	0	2200	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		60.0									58.0	
Effective Green, g (s)		60.0									58.0	
Actuated g/C Ratio		0.46									0.45	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2446									2365	
v/s Ratio Prot		c0.41									c0.42	
v/s Ratio Perm												
v/c Ratio		0.90									0.93	
Uniform Delay, d1		32.1									34.1	
Progression Factor		0.67									0.48	
Incremental Delay, d2		5.4									3.4	
Delay (s)		27.0									19.6	
Level of Service		С									В	
Approach Delay (s)		27.0			0.0			0.0			19.6	
Approach LOS		С			А			А			В	
Intersection Summary												
HCM 2000 Control Delay			23.3	H	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capacity ra	atio		0.91									
Actuated Cycle Length (s)			130.0		um of lost				12.0			
Intersection Capacity Utilization			84.1%	IC	CU Level of	of Service			E			
Analysis Period (min)			15									
Description: East side												
 Critical Lana Croup 												

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Movement	EBT	EBR	WBL	WBT	NEL	NER			
Lane Configurations	† ††					11			
Traffic Volume (vph)	2015	0	0	0	0	570			
Future Volume (vph)	2015	0	0	0	0	570			
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000			
Total Lost time (s)	4.0					6.0			
Lane Util. Factor	0.91					0.88			
Frt	1.00					1.00			
Flt Protected	1.00					1.00			
Satd. Flow (prot)	5301					3417			
Flt Permitted	1.00					1.00			
Satd. Flow (perm)	5301					3417			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92			
Adj. Flow (vph)	2190	0.02	0.52	0.02	0.02	620			
RTOR Reduction (vph)	0	0	0	0	0	0			
Lane Group Flow (vph)	2190	0	0	0	0	620			
Turn Type	NA	<u> </u>	<u> </u>	<u> </u>		Prot			
Protected Phases	Free!					2!			
Permitted Phases	1100.					۷.			
Actuated Green, G (s)	130.0					58.0			
Effective Green, g (s)	130.0					58.0			
Actuated g/C Ratio	1.00					0.45			
Clearance Time (s)						6.0			
Lane Grp Cap (vph)	5301					1524			
v/s Ratio Prot	0.41					0.18			
v/s Ratio Perm	0.11					0.10			
v/c Ratio	0.41					0.41			
Uniform Delay, d1	0.0					24.4			
Progression Factor	1.00					1.00			
Incremental Delay, d2	0.1					0.8			
Delay (s)	0.1					25.2			
Level of Service	A					C			
Approach Delay (s)	0.1			0.0	25.2	-			
Approach LOS	A			A	C				
Intersection Summary									
HCM 2000 Control Delay			5.6	H	CM 2000	Level of Service		A	
HCM 2000 Volume to Capacity ratio			0.46		2000			, ,	
Actuated Cycle Length (s)			130.0	Si	um of lost	time (s)		12.0	
Intersection Capacity Utilization			84.1%			of Service		E	
Analysis Period (min)			15		2 201010			_	
 Phase conflict between 	lane groups.								

Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	朴朴					1
Traffic Vol, veh/h	2325	23	0	0	0	10
Future Vol, veh/h	2325	23	0	0	0	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e, # 0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	2527	25	0	0	0	11

Major/Minor	Major1		Minor1					
Conflicting Flow All	0	0	-	1276				
Stage 1	-	-	-	-				
Stage 2	-	-	-	-				
Critical Hdwy	-	-	-	7.16				
Critical Hdwy Stg 1	-	-	-	-				
Critical Hdwy Stg 2	-	-	-	-				
Follow-up Hdwy	-	-	-	3.93				
Pot Cap-1 Maneuver	-	-	0	134				
Stage 1	-	-	0	-				
Stage 2	-	-	0	-				
Platoon blocked, %	-	-						
Mov Cap-1 Maneuve		-	-	134				
Mov Cap-2 Maneuve	r -	-	-	-				
Stage 1	-	-	-	-				
Stage 2	-	-	-	-				

Approach	EB	NB	
HCM Control Delay, s	0	34.2	
HCM LOS		D	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	134	-	-
HCM Lane V/C Ratio	0.081	-	-
HCM Control Delay (s)	34.2	-	-
HCM Lane LOS	D	-	-
HCM 95th %tile Q(veh)	0.3	-	-

Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			_ ≜ 1}	
Traffic Vol, veh/h	0	11	0	0	254	13
Future Vol, veh/h	0	11	0	0	254	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	12	0	0	276	14

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	145	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.96	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy		3.33	-	-	
Pot Cap-1 Maneuver	0	873	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuve		873	-	-	
Mov Cap-2 Maneuve	r -	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	

Approach	EB	SB	
HCM Control Delay, s	9.2	0	
HCM LOS	А		

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	873	-	-
HCM Lane V/C Ratio	0.014	-	-
HCM Control Delay (s)	9.2	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			_ ≜ 1}	
Traffic Vol, veh/h	0	1	0	0	152	23
Future Vol, veh/h	0	1	0	0	152	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	1	0	0	165	25

Major/Minor	Minor2		Major2	
Conflicting Flow All	-	95	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	6.96	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	3.33	-	-
Pot Cap-1 Maneuver	0	940	-	-
Stage 1	0	-	-	-
Stage 2	0	-	-	-
Platoon blocked, %			-	-
Mov Cap-1 Maneuve		940	-	-
Mov Cap-2 Maneuve	r -	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	SB
HCM Control Delay, s	8.8	0
HCM LOS	А	

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	940	-	-
HCM Lane V/C Ratio	0.001	-	-
HCM Control Delay (s)	8.8	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0	-	-

Int Delay, s/veh	3.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ľ	र्च				1		- 11	1				
Traffic Vol, veh/h	7	83	0	0	0	9	0	128	37	0	0	0	
Future Vol, veh/h	7	83	0	0	0	9	0	128	37	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	0	-	-	-	-	0	-	-	50	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	16979	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3	
Mvmt Flow	8	90	0	0	0	10	0	139	40	0	0	0	

Major/Minor	Minor2		Mir	nor1		М	ajor1			
Conflicting Flow All	70	179	-	-	-	70	-	0	0	
Stage 1	0	0	-	-	-	-	-	-	-	
Stage 2	70	179	-	-	-	-	-	-	-	
Critical Hdwy	7.56	6.56	-	-	-	6.96	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.56	5.56	-	-	-	-	-	-	-	
Follow-up Hdwy	3.53	4.03	-	-	-	3.33	-	-	-	
Pot Cap-1 Maneuver	911	711	0	0	0	975	0	-	-	
Stage 1	-	-	0	0	0	-	0	-	-	
Stage 2	929	748	0	0	0	-	0	-	-	
Platoon blocked, %								-	-	
Mov Cap-1 Maneuver	902	711	-	-	-	975	-	-	-	
Mov Cap-2 Maneuver	902	711	-	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	-	-	-	
Stage 2	920	748	-	-	-	-	-	-	-	

Approach	EB	WB	NB	
HCM Control Delay, s	10.7	8.7	0	
HCM LOS	В	А		

Minor Lane/Major Mvmt	NBT	NBR E	BLn1 E	BLn2W	/BLn1
Capacity (veh/h)	-	-	902	715	975
HCM Lane V/C Ratio	-	- (0.006	0.13	0.01
HCM Control Delay (s)	-	-	9	10.8	8.7
HCM Lane LOS	-	-	А	В	А
HCM 95th %tile Q(veh)	-	-	0	0.4	0

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Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	4111					
Traffic Volume (veh/h)	1545	570	0	0	0	0
Future Volume (Veh/h)	1545	570	0	0	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1679	620	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)				110110		
Upstream signal (ft)	585			197		
pX, platoon unblocked	000		0.89	107	0.89	0.89
vC, conflicting volume			2299		1989	730
vC1, stage 1 conf vol			2200		1000	100
vC2, stage 2 conf vol						
vCu, unblocked vol			1842		1494	79
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)			т. ∠		0.0	1.0
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			287		100	856
					100	000
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	480	480	480	860		
Volume Left	0	0	0	0		
Volume Right	0	0	0	620		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.28	0.28	0.28	0.51		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilizat	tion		84.9%	IC	Ulevelo	of Service
Analysis Period (min)			15			
			10			

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Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations			<u> </u>	^		11=11
Traffic Volume (veh/h)	0	0	140	2924	0	0
Future Volume (Veh/h)	0	0	140	2924	0	0
Sign Control	Free	Ŭ		Free	Stop	Ű
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	152	3178	0	0
Pedestrians	, i i i i i i i i i i i i i i i i i i i	•		••	·	Ū
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	Nono			Nono		
Upstream signal (ft)	196			573		
pX, platoon unblocked	100			010		
vC, conflicting volume			0		1363	0
vC1, stage 1 conf vol			Ŭ		1000	Ű
vC2, stage 2 conf vol						
vCu, unblocked vol			0		1363	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)					0.0	
tF (s)			2.2		3.5	3.3
p0 queue free %			91		100	100
cM capacity (veh/h)			1614		125	1081
Direction, Lane #	WB 1	WB 2	WB 3	WB 4	-	
Volume Total	152	1059	1059	1059		
Volume Left	152	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.09	0.62	0.62	0.62		
Queue Length 95th (ft)	8	0.02	0.02	0.02		
Control Delay (s)	7.5	0.0	0.0	0.0		
Lane LOS	A	0.0	0.0	0.0		
Approach Delay (s)	0.3					
Approach LOS	0.0					
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utili	zation		73.7%	IC	U Level o	of Service
Analysis Period (min)			15			•
			10			

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Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	۲	^				
Traffic Volume (veh/h)	370	2015	0	0	0	0
Future Volume (Veh/h)	370	2015	0	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	402	2190	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	110110			
Upstream signal (ft)		473	280			
pX, platoon unblocked		011	200			
vC, conflicting volume	0				1534	0
vC1, stage 1 conf vol	J				1004	U
vC2, stage 2 conf vol						
vCu, unblocked vol	0				1534	0
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)	7.2				0.5	7.0
tF (s)	2.2				3.5	3.3
p0 queue free %	75				100	100
cM capacity (veh/h)	1614				80	1081
					00	1001
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	402	730	730	730		
Volume Left	402	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.25	0.43	0.43	0.43		
Queue Length 95th (ft)	25	0	0	0		
Control Delay (s)	8.0	0.0	0.0	0.0		
Lane LOS	А					
Approach Delay (s)	1.2					
Approach LOS						
Intersection Summary						
Average Delay			1.2			
Intersection Capacity Utiliza	ation		78.4%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SEL	SER
Lane Configurations			^	1		
Traffic Volume (veh/h)	0	0	2024	590	0	0
Future Volume (Veh/h)	0	0	2024	590	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	2200	641	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		250				
pX, platoon unblocked						
vC, conflicting volume	2841				2200	733
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2841				2200	733
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	129				38	361
		WB 2				•••
Direction, Lane #	WB 1		WB 3	WB 4		
Volume Total	733	733	733	641		
Volume Left	0	0	0	0		
Volume Right	0	0	0	641		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.43	0.43	0.43	0.38		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		82.5%	IC	U Level c	of Service
Analysis Period (min)			15			
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HCM Signalized Intersection Capacity Analysis 1001: EB Big Beaver Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<u> </u>	1						77	ሻሻ		
Traffic Volume (vph)	0	2723	181	0	0	0	0	0	439	543	0	0
Future Volume (vph)	0	2723	181	0	0	0	0	0	439	543	0	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		5.4	5.4						5.4	5.4		
Lane Util. Factor		0.91	1.00						0.88	0.97		
Frt		1.00	0.85						0.85	1.00		
Flt Protected		1.00	1.00						1.00	0.95		
Satd. Flow (prot)		5301	1650						2905	3579		
Flt Permitted		1.00	1.00						1.00	0.95		
Satd. Flow (perm)		5301	1650						2905	3579		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2960	197	0	0	0	0	0	477	590	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2960	197	0	0	0	0	0	477	590	0	0
Turn Type		NA	Perm						Perm	Prot		
Protected Phases		2								3		
Permitted Phases			2						4			
Actuated Green, G (s)		68.6	68.6						17.6	17.6		
Effective Green, g (s)		68.6	68.6						17.6	17.6		
Actuated g/C Ratio		0.57	0.57						0.15	0.15		
Clearance Time (s)		5.4	5.4						5.4	5.4		
Vehicle Extension (s)		3.0	3.0						3.0	3.0		
Lane Grp Cap (vph)		3030	943						426	524		
v/s Ratio Prot		c0.56								c0.16		
v/s Ratio Perm			0.12						c0.16			
v/c Ratio		0.98	0.21						1.12	1.13		
Uniform Delay, d1		24.9	12.5						51.2	51.2		
Progression Factor		0.40	0.53						1.00	1.00		
Incremental Delay, d2		1.9	0.0						80.4	78.8		
Delay (s)		11.9	6.6						131.6	130.0		
Level of Service		В	А						F	F		
Approach Delay (s)		11.6			0.0			131.6			130.0	
Approach LOS		В			А			F			F	
Intersection Summary												
HCM 2000 Control Delay			41.7	H	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		1.03									
Actuated Cycle Length (s)			120.0	Si	um of lost	time (s)			16.2			
Intersection Capacity Utilization			126.7%	IC	U Level o	of Service			Н			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<u>ተተ</u> ኑ					111	
Traffic Volume (vph)	3447	239	0	0	0	356	
Future Volume (vph)	3447	239	0	0	0	356	
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000	
Total Lost time (s)	6.3					6.3	
Lane Util. Factor	0.91					0.76	
Frt	0.99					0.85	
Flt Protected	1.00					1.00	
Satd. Flow (prot)	5249					3763	
Flt Permitted	1.00					1.00	
Satd. Flow (perm)	5249					3763	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	3747	260	0	0	0	387	
RTOR Reduction (vph)	4	0	0	0	0	1	
Lane Group Flow (vph)	4003	0	0	0	0	386	
Turn Type	NA					Prot	
Protected Phases	2					4	
Permitted Phases							
Actuated Green, G (s)	107.8					19.6	
Effective Green, g (s)	107.8					19.6	
Actuated g/C Ratio	0.77					0.14	
Clearance Time (s)	6.3					6.3	
Vehicle Extension (s)	0.2					3.0	
Lane Grp Cap (vph)	4041					526	
v/s Ratio Prot	c0.76					c0.10	
v/s Ratio Perm							
v/c Ratio	0.99					0.73	
Uniform Delay, d1	15.6					57.7	
Progression Factor	1.00					1.00	
Incremental Delay, d2	12.1					5.3	
Delay (s)	27.7					63.0	
Level of Service	С					E	
Approach Delay (s)	27.7			0.0	63.0		
Approach LOS	С			А	Е		
Intersection Summary							
HCM 2000 Control Delay			30.8	H	CM 2000	Level of Service	
HCM 2000 Volume to Capa	acity ratio		0.95				
Actuated Cycle Length (s)			140.0	Su	um of lost	t time (s)	
Intersection Capacity Utilization	ation		86.7%			of Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations				† ††	ኘኘ			
Traffic Volume (vph)	0	0	0	2917	412	0		
Future Volume (vph)	0	0	0	2917	412	0		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)				6.3	6.3			
Lane Util. Factor				0.91	*1.00			
Frt				1.00	1.00			
Flt Protected				1.00	0.95			
Satd. Flow (prot)				5301	3689			
Flt Permitted				1.00	0.95			
Satd. Flow (perm)				5301	3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	 	
Adj. Flow (vph)	0.02	0.02	0.02	3171	448	0		
RTOR Reduction (vph)	Ũ	Ũ	0	0	4	0		
Lane Group Flow (vph)	0	0	0	3171	444	0		
Turn Type	<u> </u>	<u> </u>	<u> </u>	NA	Prot			
Protected Phases				6	8			
Permitted Phases				0	U			
Actuated Green, G (s)				106.2	21.2			
Effective Green, g (s)				106.2	21.2			
Actuated g/C Ratio				0.76	0.15			
Clearance Time (s)				6.3	6.3			
Vehicle Extension (s)				0.2	3.0			
Lane Grp Cap (vph)				4021	558			
v/s Ratio Prot				c0.60	c0.12			
v/s Ratio Perm				00.00	00.12			
v/c Ratio				0.79	0.80			
Uniform Delay, d1				10.2	57.3			
Progression Factor				0.41	0.99			
Incremental Delay, d2				1.5	2.7			
Delay (s)				5.7	59.3			
Level of Service				A	E			
Approach Delay (s)	0.0			5.7	59.3			
Approach LOS	A			A	55.5 E			
Intersection Summary								
HCM 2000 Control Delay			12.3	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capacity	v ratio		0.79					
Actuated Cycle Length (s)			140.0	S	um of lost	time (s)	12.6	
Intersection Capacity Utilizatio	n		116.4%		U Level o		Н	
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	WBT	WBR	SWL	SWR		
Lane Configurations			^		•••=	11		
Traffic Volume (vph)	0	0	2377	0	0	540		
Future Volume (vph)	0	0	2377	0	0	540		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)			4.0			6.0		
Lane Util. Factor			0.91			*0.95		
Frt			1.00			1.00		
Flt Protected			1.00			1.00		
Satd. Flow (prot)			5301			3689		
Flt Permitted			1.00			1.00		
Satd. Flow (perm)			5301			3689		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0	0	2584	0	0	587		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	0	0	2584	0	0	587		
Turn Type			NA			Prot		
Protected Phases			Free!			4!		
Permitted Phases								
Actuated Green, G (s)			140.0			61.0		
Effective Green, g (s)			140.0			61.0		
Actuated g/C Ratio			1.00			0.44		
Clearance Time (s)						6.0		
ane Grp Cap (vph)			5301			1607		
/s Ratio Prot			0.49			0.16		
/s Ratio Perm								
/c Ratio			0.49			0.37		
Jniform Delay, d1			0.0			26.5		
Progression Factor			1.00			1.00		
ncremental Delay, d2			0.0			0.6		
Delay (s)			0.0			27.2		
_evel of Service			А			С		
Approach Delay (s)		0.0	0.0		27.2			
Approach LOS		А	А		С			
Intersection Summary								
HCM 2000 Control Delay			5.0	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capacit	ty ratio		0.53					
Actuated Cycle Length (s)			140.0		um of lost		12.0	
Intersection Capacity Utilization	on		87.5%	IC	U Level o	of Service	Е	
Analysis Period (min)			15					
Phase conflict between lan	ne groups.							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	SEL	SET	SER	NWL	NWT	NWR
Lane Configurations		1111									***	
Traffic Volume (vph)	0	2461	0	0	0	0	0	0	0	0	2377	0
Future Volume (vph)	0	2461	0	0	0	0	0	0	0	0	2377	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.86									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		6680									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		6680									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2675	0	0	0	0	0	0	0	0	2584	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2675	0	0	0	0	0	0	0	0	2584	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		61.0									67.0	
Effective Green, g (s)		61.0									67.0	
Actuated g/C Ratio		0.44									0.48	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2910									2536	
v/s Ratio Prot		c0.40									c0.49	
v/s Ratio Perm												
v/c Ratio		0.92									1.02	
Uniform Delay, d1		37.2									36.5	
Progression Factor		0.88									1.46	
Incremental Delay, d2		2.2									22.3	
Delay (s)		34.8									75.5	
Level of Service		С									E	
Approach Delay (s)		34.8			0.0			0.0			75.5	
Approach LOS		С			А			А			Е	
Intersection Summary												
HCM 2000 Control Delay			54.8	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		0.97									
Actuated Cycle Length (s)			140.0		um of lost				12.0			
Intersection Capacity Utilization			87.5%	IC	U Level o	of Service			E			
Analysis Period (min)			15									
Description: West side												
 Critical Lana Croup 												

	۲	-	-	*	\searrow	4		
Movement	EBL	EBT	WBT	WBR	SEL	SER		
Lane Configurations		tttt			ኘካ			
Traffic Volume (vph)	0	2461	0	0	800	0		
Future Volume (vph)	0	2461	0	0	800	0		
(, ,	2000	2000	2000	2000	2000	2000		
Total Lost time (s)		4.0			6.0			
Lane Util. Factor		0.86			*0.95			
Frt		1.00			1.00			
Flt Protected		1.00			1.00			
Satd. Flow (prot)		6680			3689			
Flt Permitted		1.00			1.00			
Satd. Flow (perm)		6680			3689			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	0.02	2675	0.02	0.02	870	0		
RTOR Reduction (vph)	Ũ	0	0	0	0	0		
Lane Group Flow (vph)	0	2675	0	0	870	0		
Turn Type	<u> </u>	NA			Prot	-		
Protected Phases		Free!			2!			
Permitted Phases		1100.						
Actuated Green, G (s)		140.0			67.0			
Effective Green, g (s)		140.0			67.0			
Actuated g/C Ratio		1.00			0.48			
Clearance Time (s)					6.0			
Lane Grp Cap (vph)		6680			1765			
v/s Ratio Prot		0.40			c0.24			
v/s Ratio Perm		•••••						
//c Ratio		0.40			0.49			
Jniform Delay, d1		0.0			24.9			
Progression Factor		1.00			1.00			
Incremental Delay, d2		0.1			1.0			
Delay (s)		0.1			25.9			
Level of Service		A			С			
Approach Delay (s)		0.1	0.0		25.9			
Approach LOS		А	А		С			
Intersection Summary								
HCM 2000 Control Delay			6.4	H	CM 2000	Level of Service	А	
HCM 2000 Volume to Capacity	ratio		0.47					
Actuated Cycle Length (s)			140.0	Si	um of lost	time (s)	12.0	
Intersection Capacity Utilization			63.9%	IC	U Level o	of Service	В	
Analysis Period (min)			15					
Phase conflict between lane	groups							
o Critical Lana Group								

	-	-*	5	-	•	4			
Movement	EBT	EBR	WBL	WBT	NWL	NWR			
Lane Configurations	201	2011		^	ኘካ				
Traffic Volume (vph)	0	0	0	1857	690	0			
Future Volume (vph)	0	0	0	1857	690	0			
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000			
Total Lost time (s)	2000	2000	2000	4.0	6.0	2000			
Lane Util. Factor				0.91	*0.95				
Frt				1.00	1.00				
Flt Protected				1.00	1.00				
Satd. Flow (prot)				5301	3689				
Flt Permitted				1.00	1.00				
Satd. Flow (perm)				5301	3689				
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92			
Adj. Flow (vph)	0.92	0.92	0.92	2018	750	0.92			
RTOR Reduction (vph)	0	0	0	2010	0	0			
Lane Group Flow (vph)	0	0	0	2018	750	0			
Turn Type	0	0	0	NA	Prot	0			
Protected Phases				Free!	P101 4!				
Permitted Phases				Fiee:	4!				
Actuated Green, G (s)				140.0	78.0				
Effective Green, g (s)				140.0	78.0				
Actuated g/C Ratio				140.0	0.56				
Clearance Time (s)				1.00	0.56 6.0				
				E204					
Lane Grp Cap (vph)				5301	2055				
v/s Ratio Prot				0.38	0.20				
v/s Ratio Perm				0.20	0.20				
v/c Ratio				0.38	0.36				
Uniform Delay, d1				0.0	17.2				
Progression Factor				1.00	1.00				
Incremental Delay, d2				0.0	0.5				
Delay (s)				0.0	17.7 D				
Level of Service	0.0			A	8				
Approach Delay (s)	0.0			0.0	17.7 D				
Approach LOS	A			A	В				
Intersection Summary			1.0	, .	014 0000				
HCM 2000 Control Delay			4.8	H	CM 2000	Level of Service	;	А	
HCM 2000 Volume to Capacity	ratio		0.42					40.0	
Actuated Cycle Length (s)			140.0		um of lost			12.0	
Intersection Capacity Utilization	1		61.1%	IC	U Level o	of Service		В	
Analysis Period (min)			15						
Phase conflict between lane	groups.								
c Critical Lane Group									

HCM Signalized Intersection Capacity Analysis 1008: WB Big Beaver Rd & EB Big Beaver Rd

		-	7	*	-	۲	•	×	/	6	*	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		<u>_</u>									***	
Traffic Volume (vph)	0	2511	0	0	0	0	0	0	0	0	1857	0
Future Volume (vph)	0	2511	0	0	0	0	0	0	0	0	1857	0
Ideal Flow (vphpl) 2	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000	2000
Total Lost time (s)		6.0									6.0	
Lane Util. Factor		0.91									0.91	
Frt		1.00									1.00	
Flt Protected		1.00									1.00	
Satd. Flow (prot)		5301									5301	
Flt Permitted		1.00									1.00	
Satd. Flow (perm)		5301									5301	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	2729	0	0	0	0	0	0	0	0	2018	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	2729	0	0	0	0	0	0	0	0	2018	0
Turn Type		NA									NA	
Protected Phases		4									2	
Permitted Phases												
Actuated Green, G (s)		78.0									50.0	
Effective Green, g (s)		78.0									50.0	
Actuated g/C Ratio		0.56									0.36	
Clearance Time (s)		6.0									6.0	
Lane Grp Cap (vph)		2953									1893	
v/s Ratio Prot		c0.51									c0.38	
v/s Ratio Perm												
v/c Ratio		0.92									1.07	
Uniform Delay, d1		28.3									45.0	
Progression Factor		1.07									0.42	
Incremental Delay, d2		5.8									31.0	
Delay (s)		36.0									49.9	
Level of Service		D									D	
Approach Delay (s)		36.0			0.0			0.0			49.9	
Approach LOS		D			A			A			D	
Intersection Summary												
HCM 2000 Control Delay			41.9	Н	CM 2000	Level of S	Service		D			
HCM 2000 Volume to Capacity ra	atio		0.98									
Actuated Cycle Length (s)			140.0		um of lost				12.0			
Intersection Capacity Utilization			90.2%	IC	CU Level of	of Service			E			
Analysis Period (min)			15									
Description: East side												
 Critical Lana Croup 												

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Movement	EBT	EBR	WBL	WBT	NEL	NER		
Lane Configurations	<u> </u>					11		
Traffic Volume (vph)	2511	0	0	0	0	240		
Future Volume (vph)	2511	0	0	0	0	240		
Ideal Flow (vphpl)	2000	2000	2000	2000	2000	2000		
Total Lost time (s)	4.0					6.0		
Lane Util. Factor	0.91					0.88		
Frt	1.00					1.00		
Flt Protected	1.00					1.00		
Satd. Flow (prot)	5301					3417		
Flt Permitted	1.00					1.00		
Satd. Flow (perm)	5301					3417		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Adj. Flow (vph)	2729	0	0	0	0	261		
RTOR Reduction (vph)	0	0	0	0	0	0		
Lane Group Flow (vph)	2729	0	0	0	0	261		
Turn Type	NA					Prot	ĺ	
Protected Phases	Free!					2!		
Permitted Phases								
Actuated Green, G (s)	140.0					50.0		
Effective Green, g (s)	140.0					50.0		
Actuated g/C Ratio	1.00					0.36		
Clearance Time (s)						6.0		
Lane Grp Cap (vph)	5301					1220		
v/s Ratio Prot	0.51					0.08		
v/s Ratio Perm								
v/c Ratio	0.51					0.21		
Uniform Delay, d1	0.0					31.3		
Progression Factor	1.00					1.00		
Incremental Delay, d2	0.1					0.4		
Delay (s)	0.1					31.7		
Level of Service	Α					С		
Approach Delay (s)	0.1			0.0	31.7			
Approach LOS	А			А	С			
Intersection Summary								
HCM 2000 Control Delay			2.9	H	CM 2000	Level of Servic	e	е
HCM 2000 Volume to Capa	acity ratio		0.56					
Actuated Cycle Length (s)			140.0		um of lost			
Intersection Capacity Utiliz	ation		90.2%	IC	U Level o	of Service		
Analysis Period (min)			15					
Phase conflict between	lane groups.							
 Critical Lana Croup 								

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	朴朴					1
Traffic Vol, veh/h	3667	38	0	0	0	19
Future Vol, veh/h	3667	38	0	0	0	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Stop	Stop	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage	e, # 0	-	-	16983	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	3986	41	0	0	0	21

Major/Minor	Major1		Minor1	
Conflicting Flow All	0	0	-	2014
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	-	-	7.16
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	-	-	3.93
Pot Cap-1 Maneuver	-	-	0	41
Stage 1	-	-	0	-
Stage 2	-	-	0	-
Platoon blocked, %	-	-		
Mov Cap-1 Maneuver		-	-	41
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Approach	EB		NB	

HCM Control Delay, s 0 161.1 HCM LOS F	Approach	EB	NB	
HCM LOS F	HCM Control Delay, s	0		
	HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR
Capacity (veh/h)	41	-	-
HCM Lane V/C Ratio	0.504	-	-
HCM Control Delay (s)	161.1	-	-
HCM Lane LOS	F	-	-
HCM 95th %tile Q(veh)	1.8	-	-

Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1				
Traffic Vol, veh/h	0	29	0	0	238	22
Future Vol, veh/h	0	29	0	0	238	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	32	0	0	259	24

Major/Minor	Minor2		Major2		
Conflicting Flow All	-	142	-	0	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Critical Hdwy	-	6.96	-	-	
Critical Hdwy Stg 1	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	
Follow-up Hdwy	-	3.33	-	-	
Pot Cap-1 Maneuver	0	877	-	-	
Stage 1	0	-	-	-	
Stage 2	0	-	-	-	
Platoon blocked, %			-	-	
Mov Cap-1 Maneuver	-	877	-	-	
Mov Cap-2 Maneuver	-	-	-	-	
Stage 1	-	-	-	-	
Stage 2	-	-	-	-	
Approach	EB		SB		
HCM Control Delay, s	9.3		0		

HCM LOS А

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	877	-	-
HCM Lane V/C Ratio	0.036	-	-
HCM Control Delay (s)	9.3	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0.1	-	-

Int Delay, s/veh	1.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1			- † 1-	
Traffic Vol, veh/h	0	45	0	0	166	38
Future Vol, veh/h	0	45	0	0	166	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	,# 0	-	-	16974	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	0	49	0	0	180	41

Major/Minor	Minor2		Major2	
Conflicting Flow All	-	111	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	6.96	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	3.33	-	-
Pot Cap-1 Maneuver	0	918	-	-
Stage 1	0	-	-	-
Stage 2	0	-	-	-
Platoon blocked, %			-	-
Mov Cap-1 Maneuver		918	-	-
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Approach	EB		SB	
HCM Control Delay, s			0	
HCM LOS	A			

Minor Lane/Major Mvmt	EBLn1	SBT	SBR
Capacity (veh/h)	918	-	-
HCM Lane V/C Ratio	0.053	-	-
HCM Control Delay (s)	9.1	-	-
HCM Lane LOS	А	-	-
HCM 95th %tile Q(veh)	0.2	-	-

5

Intersection

Int Delay, s/veh

		FDT			MOT			NDT		0.01	ODT	000	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	- ሽ	- सी				1		- 11	1				
Traffic Vol, veh/h	26	37	0	0	0	149	0	202	9	0	0	0	
Future Vol, veh/h	26	37	0	0	0	149	0	202	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	0	-	-	-	-	0	-	-	50	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	16979	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3	
Mvmt Flow	28	40	0	0	0	162	0	220	10	0	0	0	

Major/Minor	Minor2		М	inor1		Ν	lajor1				
Conflicting Flow All	110	230	-	-	-	110	-	0	0		
Stage 1	0	0	-	-	-	-	-	-	-		
Stage 2	110	230	-	-	-	-	-	-	-		
Critical Hdwy	7.56	6.56	-	-	-	6.96	-	-	-		
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-		
Critical Hdwy Stg 2	6.56	5.56	-	-	-	-	-	-	-		
Follow-up Hdwy	3.53	4.03	-	-	-	3.33	-	-	-		
Pot Cap-1 Maneuver	854	666	0	0	0	919	0	-	-		
Stage 1	-	-	0	0	0	-	0	-	-		
Stage 2	880	710	0	0	0	-	0	-	-		
Platoon blocked, %								-	-		
Mov Cap-1 Maneuver	r 704	666	-	-	-	919	-	-	-		
Mov Cap-2 Maneuver	r 704	666	-	-	-	-	-	-	-		
Stage 1	-	-	-	-	-	-	-	-	-		
Stage 2	725	710	-	-	-	-	-	-	-		
Approach	EB			WB			NB				

Approach	EB	VVB	NB	
HCM Control Delay, s	10.7	9.8	0	
HCM LOS	В	A		

Minor Lane/Major Mvmt	NBT	NBR I	EBLn1	EBLn2	WBLn1
Capacity (veh/h)	-	-	704	673	919
HCM Lane V/C Ratio	-	-	0.027	0.074	0.176
HCM Control Delay (s)	-	-	10.3	10.8	9.8
HCM Lane LOS	-	-	В	В	А
HCM 95th %tile Q(veh)	-	-	0.1	0.2	0.6

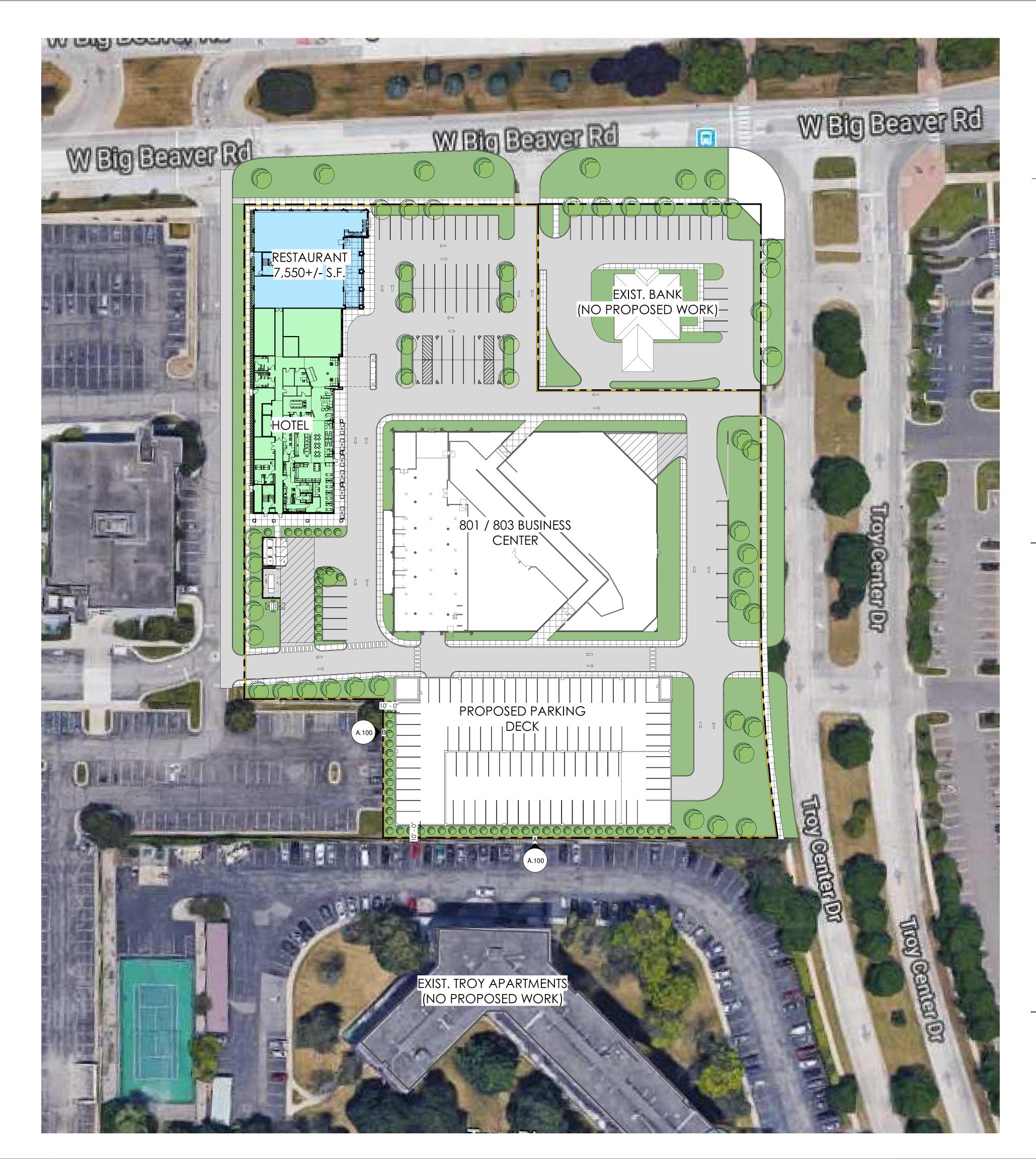
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Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	4111					
Traffic Volume (veh/h)	2461	930	0	0	0	0
Future Volume (Veh/h)	2461	930	0	0	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2675	1011	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)	585			197		
pX, platoon unblocked			0.30		0.30	0.30
vC, conflicting volume			3686		3180	1174
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			0		0	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			479		303	321
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	764	764	764	1393		
Volume Left	04	04	04	0		
Volume Right	0	0	0	1011		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.45	0.45	0.45	0.82		
	0.45	0.45	0.45	0.82		
Queue Length 95th (ft)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS	0.0					
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization	ation		87.5%	IC	U Level o	of Service
Analysis Period (min)			15			
			10			

	-	\mathbf{F}	*	+	•	/
Movement	EBT	EBR	WBL	WBT	NEL	NER
Lane Configurations			٢	†††		
Traffic Volume (veh/h)	0	0	170	2377	0	0
Future Volume (Veh/h)	0	0	170	2377	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	185	2584	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)	199			573		
pX, platoon unblocked	100			0.0		
vC, conflicting volume			0		1231	0
vC1, stage 1 conf vol			Ţ			•
vC2, stage 2 conf vol						
vCu, unblocked vol			0		1231	0
tC, single (s)			4.2		6.9	7.0
tC, 2 stage (s)					0.0	
tF (s)			2.2		3.5	3.3
p0 queue free %			89		100	100
cM capacity (veh/h)			1614		149	1081
Direction, Lane #	WB 1	WB 2	WB 3	WB 4		
Volume Total	185	861	861	861		
Volume Left	185	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.11	0.51	0.51	0.51		
Queue Length 95th (ft)	10	0	0.01	0.01		
Control Delay (s)	7.5	0.0	0.0	0.0		
Lane LOS	A	0.0	0.0	0.0		
Approach Delay (s)	0.5					
Approach LOS	0.0					
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utiliz	ation		61.1%	IC	Ulevelo	of Service
Analysis Period (min)	ation		15	10		
			13			

	*	+	ł	۲	6	*
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	ሻ	†††				
Traffic Volume (veh/h)	750	2511	0	0	0	0
Future Volume (Veh/h)	750	2511	0	0	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	815	2729	0	0	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		473	281			
pX, platoon unblocked						
vC, conflicting volume	0				2540	0
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	0				2540	0
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	50				100	100
cM capacity (veh/h)	1614				11	1081
Direction, Lane #	EB 1	EB 2	EB 3	EB 4		
Volume Total	815	910	910	910		
Volume Left	815	0	0	0		
Volume Right	0	0	0	0		
cSH	1614	1700	1700	1700		
Volume to Capacity	0.50	0.54	0.54	0.54		
Queue Length 95th (ft)	74	0	0	0		
Control Delay (s)	9.5	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	2.2					
Approach LOS						
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utiliza	ation		96.9%	IC	U Level o	of Service
Analysis Period (min)			15			

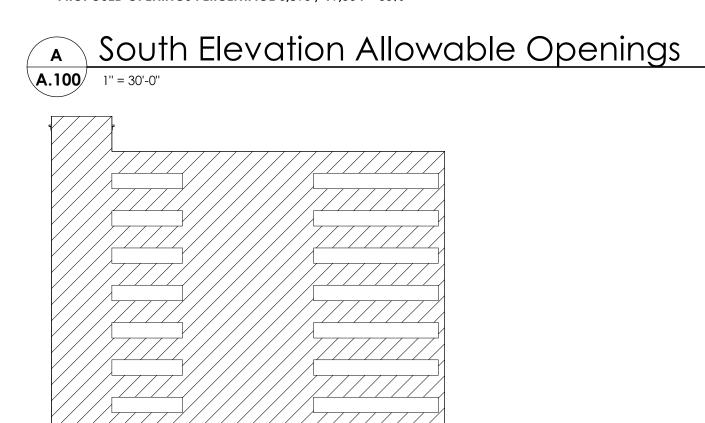
	۲	-	+	*	\searrow	4
Movement	EBL	EBT	WBT	WBR	SEL	SER
Lane Configurations			^	1		
Traffic Volume (veh/h)	0	0	1857	750	0	0
Future Volume (Veh/h)	0	0	1857	750	0	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	2018	815	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)		252				
pX, platoon unblocked						
vC, conflicting volume	2833				2018	673
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2833				2018	673
tC, single (s)	4.2				6.9	7.0
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				100	100
cM capacity (veh/h)	130				50	396
Direction, Lane #	WB 1	WB 2	WB 3	WB 4		
Volume Total	673	673	673	815		
Volume Left	0	0	0	0		
Volume Right	0	0	0	815		
cSH	1700	1700	1700	1700		
Volume to Capacity	0.40	0.40	0.40	0.48		
Queue Length 95th (ft)	0	0	0	0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		90.2%	IC	U Level c	of Service
Analysis Period (min)			15			
			10			

SITE PLANS

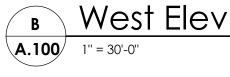


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WALL SQUARE FOOTAGE: 19,504 OPENINGS SQUARE FOOTAGE: 6,895



WALL SQUARE FOOTAGE: 10,683



Parking Calculations Breakdown

REQUIRED PARKING FOR PROPOSED DESIGN

EXISTING 801/803 BUSINESS CENTER USABLE SQ. FT. ASSESSED BY THE CITY OF TROY: 122,386 SQ. FT. (CITY ORDINANCE REQ'S 1 PARKING PER. 300 SQ. FT.) 122,386 SQ. FT. / 300 SQ. FT. = **408** PARKING SPACES

PROPOSED HOTEL W/ RESTAURANT HOTEL UNIT COUNT: 2ND FLOOR = 11

3RD FLOOR = 31 4TH FLOOR = 31 5TH FLOOR = 31 <u>6TH FLOOR = 31</u> TOTAL UNITS = 135 UNITS

EMPLOYEE DURING PEEK EMPLOYEE SHIFT)

TOTAL PARKING FOR PROPOSED HOTEL: 135 + 16 = **151** PARKING SPACES

RESTAURANT SQUARE SQ. FT.: 7,550 SF +/-(CITY ORDINANCE REQ'S 1 PARKING SPACE PER 2 SEATS IN THE RESTAURANT. SEATING IS TO BE DETERMINED BY OCCUPANT LOAD WITHIN THE CURRENT MBC CODE)

ESTIMATED NUMBER OF MAX. OCCUPANCY (PATRONS & EMPLOYEES): 43 PEOPLE REQUIRED PARKING FOR PROPOSED RESTURANT: 43/2 = 21.5 ~ 22 PARKING SPACES

TOTAL PARKING REQUIRED

TOTAL PARKING REQ'D FOR EXISTING 801/803 BUSINESS CENTER: 408 TOTAL PARKING REQ'D FOR PROPOSED HOTEL: 135 TOTAL PARKING REQ'D FOR PROPOSED RESTAURANT WITHIN HOTEL: TOTAL PARKING REQ'D 565 PARKING SPACES

IN ACCORDANCE W/ MBC 2015 SECTION 1106 THE REQ'D ADA PARKING SPACES NEEDS TO 2% OF THE TOTAL REQUIRED PARKING: 566(0.02) = 11.32 ~ **12 ADA SPACES**

Proposed P	rov		
Family			
Parking Spaces	708		
	• •		
Proposed Prov	/ide		
Family			
Parking Spaces	15		
5 - 1	10		

Off-Street Loading Requirements

EXISTING 801/803 BUSINESS CENTER •

- USABLE SQ. FT. ASSESSED BY THE CITY OF TROY: 122,386 SQ. FT. OFF-STREET LOADING REQUIREMENTS: (3) SPACES OR 1,500 SQ. FT. PROVIDED LOADING AREA: 1,560 SQ. FT. •
- PROPOSED HOTEL EST. USABLE SQ. FT. FOR THE HOTEL: 15,778. SQ. FT. +/-•
- PROPOSED HOTEL RESTAURANT •
- EST. USABLE SQ. FT. FOR HOTEL RESTAURANT: 5,622 SQ. FT. +/-
- TOTAL REQUIRED OFF-STREET LOADING REQUIREMENTS FOR PROPOSED HOTEL & RESTAURANT: 15,778 SQ. FT. + 5,622 SQ. FT. =
- 21,400 SQ. FT. OFF-STREET LOADING REQUIREMENTS: (1) SPACE
- PROVIDED LOADING AREA: 2,823 SQ. FT.

\square

MAXIMUM SQUARE FOOTAGE PERCENTAGE FOR 10 FT SEPARATION DISTANCE = 45% PROPOSED OPENINGS PERCENTAGE 6,895 / 19,504 = 35%

OPENINGS SQUARE FOOTAGE: 2,055 MAXIMUM SQUARE FOOTAGE PERCENTAGE FOR 10 FT SEPARATION DISTANCE = 45% PROPOSED OPENINGS PERCENTAGE 2,055 / 10,683 = 19.2%

West Elevation Allowable Openings

(CITY ORDINANCE REQ'S 1 PARKING SPACE FOR EA. HOTEL UNIT & 1 PARKING SPACE FOR EA.

ESITAMATED PEEK EMPLOYEES PRESENT = 16

vided Parking

Count

ded ADA Parking Count

TABLE 13.09-A EA. GFA R EA. GFA

krieger klatt

architecture interiors consulting 1412 E. 11 Mile Rd. | Royal Oak, MI 48067 **P:** 248.414.9270 **F:** 248.414.9275 www.kriegerklatt.com

Client:

Owner

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\bigcirc	Project No		
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Site Plan

Project Number:

Project Number

Sheet Number:



17 (D)													
OFF-STREET LOADING REQUIREMENTS													
UP TO 5,000 SQ. FT. GFA	1 SPACE												
5,001 TO 60,000 SQ. FT. GFA	1 SPACE, PLUS 1 SPACE PER E												
	ADDITIONAL 20,000 SQ. FT. G												
60,001 SQ. FT. GFA & OVER	3 SPACES, PLUS 1 SPACE PER												
	ADDITIONAL 50,000 SQ. FT. G												



Memorandum

To:	David Hunter, PE, PS
From:	Michael J. Labadie, PE and Alyssa Wambold, PE
Date:	October 14, 2019
RE:	Shared Parking Analysis for Proposed Hotel and Restaurant Addition to 801/803 Big Beaver Business Center Site, Troy, MI

A shared parking analysis was completed for the site in accordance with both Urban Land Institute (ULI) and City of Troy standards. The City of Troy allows for a shared parking analysis based on parking supply rates specified in the City Ordinance and ULI rates for hourly, daily, and monthly variations in parking demand for the various land uses specified. Additionally, ULI rates for employees versus visitors were also utilized for the various land uses within the proposed site.

This analysis was performed in order to determine the number of parking spaces necessary on site in comparison to the resulting number surface spaces available once the proposed hotel and restaurant are completed. The difference is proposed to be included in a new parking deck to be constructed on site.

The uses proposed to be added to the site (that already includes a 122,386-gross-square-foot office building which has approximately 77,874 gross leasable area) are a 140-room business hotel and a 232-seat fine/casual dining restaurant. The hotel has 2,949 square feet (SF) of meeting space, split between four separate rooms of 684 SF, 214 SF, 1,030, and 1,021 SF. This space will either be used by hotel guests or will be used by non-guests during the day while hotel guests are not utilizing the parking facilities.

The ULI shared parking methodologies were implemented to determine an accurate parking demand for the proposed site. It is common for sites with a mixture of uses to have varying peak hourly and monthly parking demands, as well as being shared destinations (i.e. a hotel patron may also visit a restaurant). Therefore, a shared parking analysis is necessary to accurately determine the amount in which the number of required parking spaces could be reduced by, as compared to the sum of the individual parking requirements for each land use on the site.

ROWE analyzed the differences in activity patterns, which include monthly and time of day variances, for each land use. This is because the combination of land uses within the site do not have peak utilization during the same time periods. Time-of-Day factors for weekdays and weekends and monthly adjustment factors for the various land uses can be found in the attached ULI Shared Parking spreadsheets.

David Hunter, PE, PS October 14, 2019 Page 2

The final step of the analysis was to determine the critical parking periods for the site. All hours of operation from 6 a.m. to midnight were analyzed for each month of the year. The result showed that the month of June has the highest peak demands of the year. Furthermore, the overall peak time of the day for the entire site was determined to be 2 p.m. for weekdays and 11 p.m. for weekends. The overall peak period of the proposed site would be 2 p.m. on a weekday, resulting in a maximum demand of 449 required parking spaces. Detailed tables can be found in the attached ULI Shared Parking spreadsheet.

Attachment

APPENDICES

Table Project: Description:

Big Beaver Business Center Mixed-Use Development

SHARED PARKING DEMAND SUMMARY

PEAK MONTH: JUNE -- PEAK PERIOD: 2 PM, WEEKDAY

										Weeker	nd			Weekday	1	Weekend		
					Non-					Non-			Peak Hr	Peak Mo	Estimated	Peak Hr	Peak Mo	Estimated
	Project	Data	Base	Mode	Captive Project			Base	Mode	Captive	Project		Adj	Adj	Parking	Adj	Adj	Parking
Land Use	Quantity	Unit	Rate	Adj	Ratio	Rate	Unit	Rate	Adj	Ratio	Rate	Unit	2 PM	June	Demand	11 PM	June	Demand
Fine/Casual Dining Restaurant	232	Seats	0.43	1.00	1.00	0.43	/seat	0.42	1.00	1.00	0.42	/seat	0.65	0.95	62	0.90	0.95	83
Employee			0.07	1.00	1.00	0.07	/seat	0.08	1.00	1.00	0.08	/seat	0.90	1.00	14	0.85	1.00	16
Hotel-Business	140	rooms	1.00	1.00	1.00	1.00	/rooms	1.00	1.00	1.00	1.00	/rooms	0.60	1.00	84	1.00	1.00	140
Employee			0.21	1.00	1.00	0.21	/rooms	0.21	1.00	1.00	0.21	/rooms	1.00	1.00	30	0.45	1.00	14
Office 100 to 500 ksf	77,874	sf GFA	0.27	1.00	1.00	0.27	/ksf GLA	0.03	1.00	1.00	0.03	/ksf GLA	1.00	1.00	21	0.00	1.00	0
Employee			3.06	1.00	1.00	3.06	/ksf GLA	0.31	1.00	1.00	0.31	/ksf GLA	1.00	1.00	238	0.00	1.00	0
ULI base data have been modified	ed from defa	ault valu	les.										Cus	tomer	167	Cust	tomer	223
													Employee		282	Employee		30
													Reserved		0	Reserved		0
													Total		449	Тс	otal	253

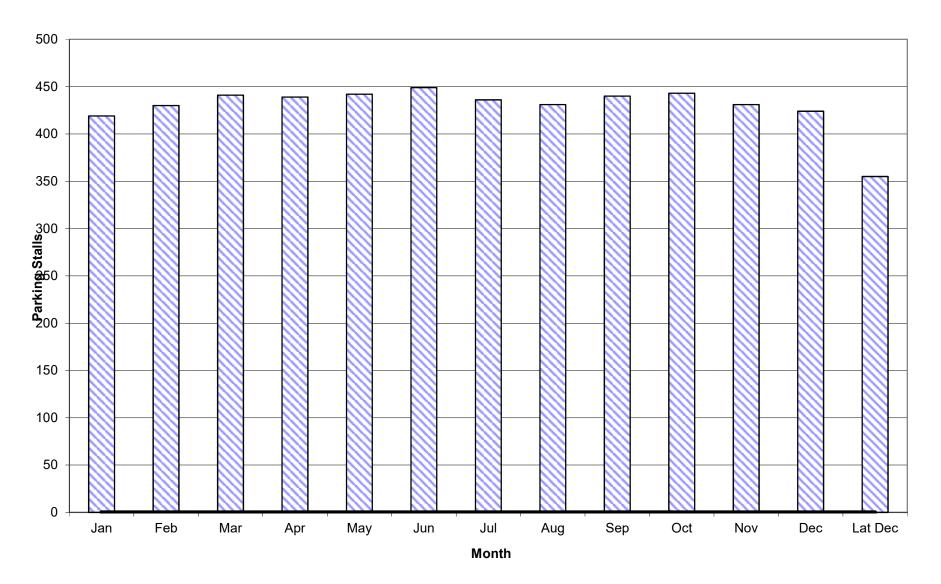
10/14/2019

										Jun	e														
	Weekday Estimated Peak-Hour Parking Demand																								
																	Overall Pk	AM Peak Hr	PM Peak Hr	Eve Peak Hr					
	Monthly Adj.	6 AM	7 AM	8 AM	9 AM	10 AM	11 AM	12 PM	1 PM	2 PM	3 PM	4 PM	5 PM	6 PM	7 PM	8 PM	9 PM	10 PM	11 PM	12 AM	2 PM	11 AM	2 PM	6 PM	
Fine/Casual Dining Restaurant	95%	-	-	-	-	14	38	71	71	62	38	48	71	90	95	95	95	90	71	24	62	38	62	90	
Employee	100%	-	3	8	12	14	14	14	14	14	12	12	16	16	16	16	16	16	14	6	14	14	14	16	
Hotel-Business	100%	133	126	112	98	84	84	77	77	84	84	91	98	105	105	112	119	133	140	140	84	84	84	105	
Employee	100%	2	9	27	27	30	30	30	30	30	30	27	21	12	6	6	6	6	3	2	30		30		
Office 100 to 500 ksf	100%	-	-	4	13	21	9	3	9	21	9	3	2	1	-	-	-	-	-	-	21	9	21	1	
Employee	100%	7	71	179	226	238	238	214	214	238	238	214	119	60	24	17	7	2	-	-	238	238	238	60	
	Customer	133	126	116	111	119	131	151	157	167	131	142	171	196	200	207	214	223	211	164	167	131	167	196	
TOTAL DEMAND	Employee	9	83	214	265	282	282	258	258	282	280	253	156	88	46	39	29	24	17	8	282	282	282	88	
	Reserved	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
		142	209	330	376	401	413	409	415	449	411	395	327	284	246	246	243	247	228	172	449	413	449	284	
ULI base data have been modified from defau	ult values.																				449	413	449		

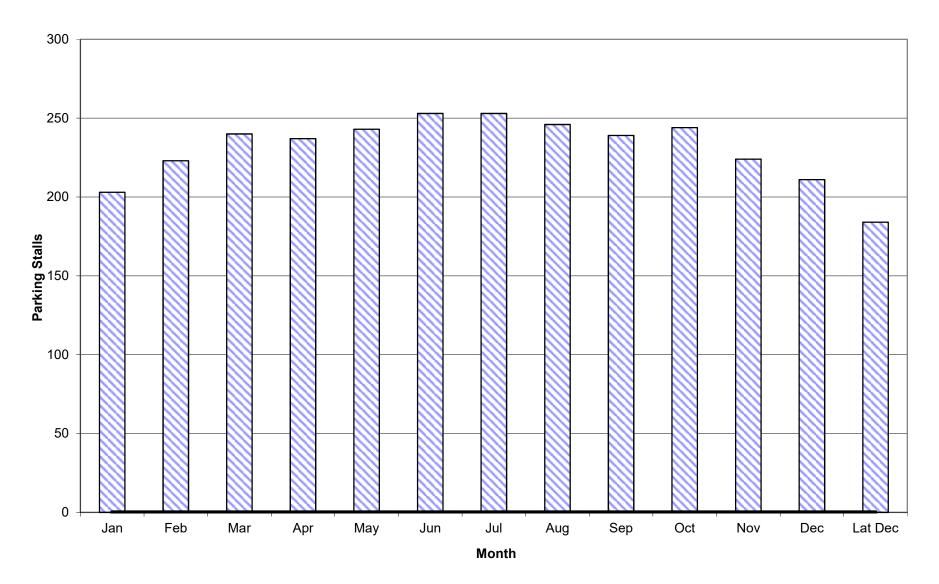
Footnote(s):

[Jun	e													
							W	eekend	Estimat	ed Peak	-Hour P	arking D	emand											
														Overall Pk	AM Peak Hr	PM Peak Hr	Eve Peak Hr							
		6 AM	7 AM	8 AM	9 AM	10 AM	11 AM	12 PM	1 PM	2 PM	3 PM	4 PM	5 PM	6 PM	7 PM	8 PM	9 PM	10 PM	11 PM	12 AM	11 PM	11 AM	5 PM	11 PM
Fine/Casual Dining Restaurant	95%	-	-	-	-	-	14	46	51	42	42	42	55	83	88	92	83	83	83	46	83	14	55	83
Employee	100%	-	4	6	11	14	14	14	14	14	14	14	19	19	19	19	19	19	16	10	16	14	19	16
Hotel-Business	100%	133	126	112	98	84	84	77	77	84	84	91	98	105	105	112	119	133	140	140	140	84	98	140
Employee	100%	2	9	27	27	30	30	30	30	30	30	27	23	18	17	17	17	14	14	9	14	30	23	14
Office 100 to 500 ksf	100%	-	-	1	2	2	2	2	2	1	1	-	-	-	-	-	-	-	-	-	-	2	-	-
Employee	100%	-	5	14	19	22	24	22	19	14	10	5	2	1	-	-	-	-		-	-	24	2	-
	Customer	133	126	113	100	86	100	125	130	127	127	133	153	188	193	204	202	216	223	186		100	153	223
TOTAL DEMAND	Employee	2	18	47	57	66	68	66	63	58	54	46	44	38	36	36	36	33	30	19	30	68	44	30
	Reserved	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-
		135	144	160	157	152	168	191	193	185	181	179	197	226	229	240	238	249	253	205	253	168	197	253
ULI base data have been modified from default values.																					253	168	197	253

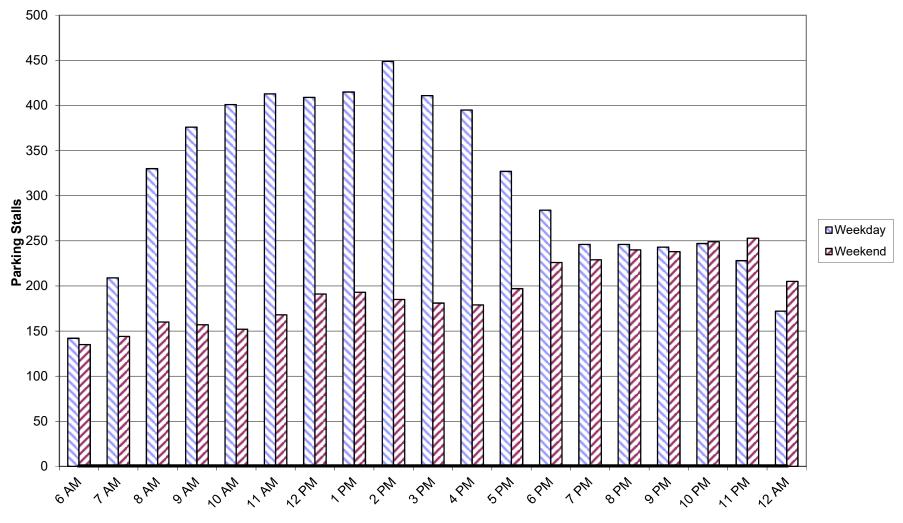
Weekday Month-by-Month Estimated Parking Demand



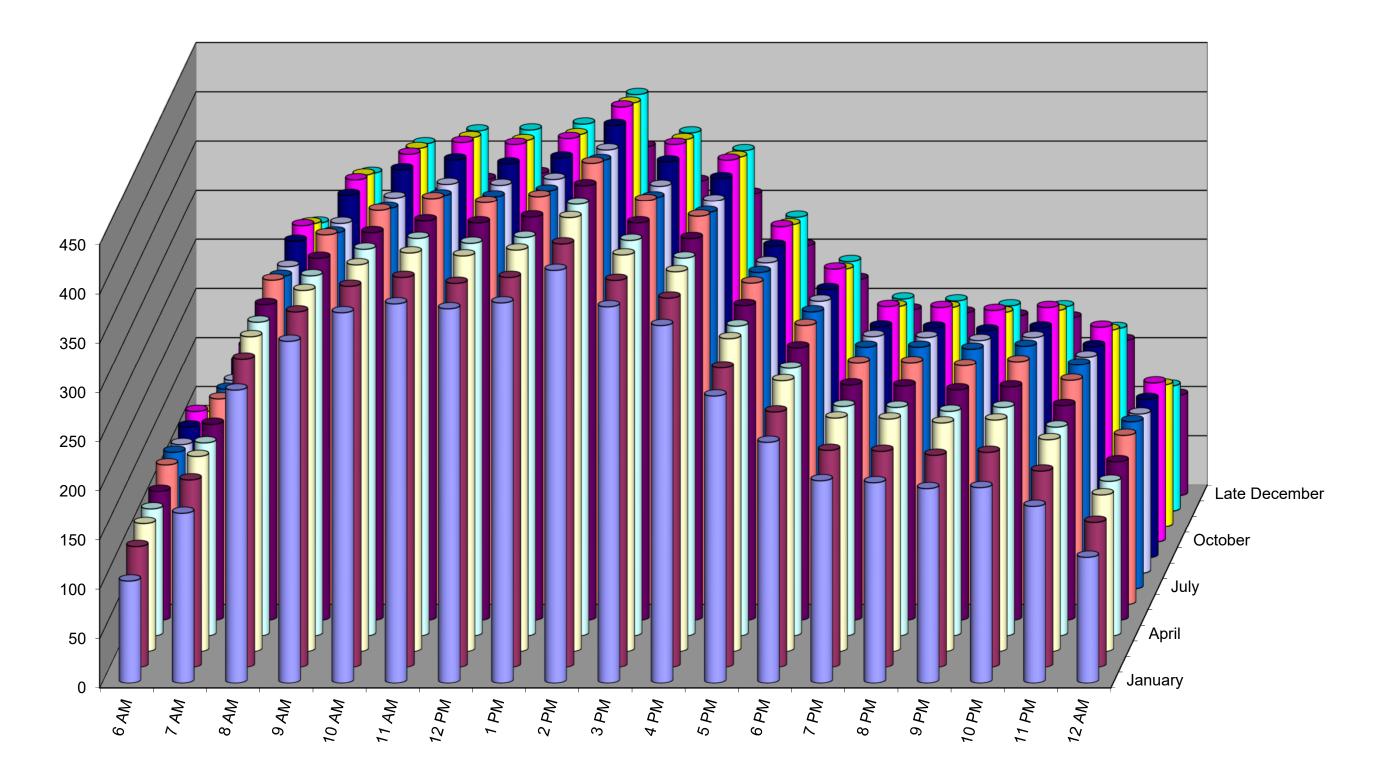
Weekend Month-by-Month Estimated Parking Demand



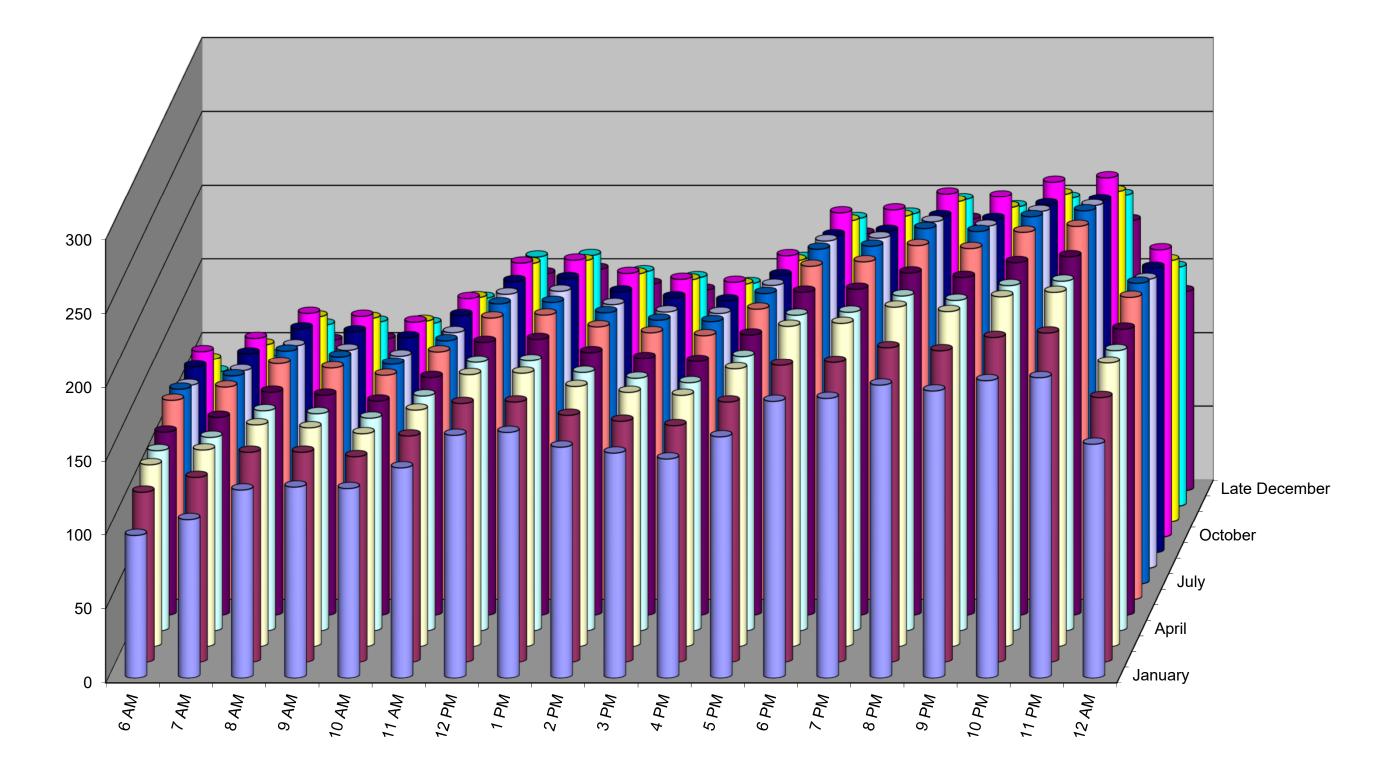
Peak Month Daily Parking Demand by Hour



Hour



Weekend Comparison by Month and by Hour





memorandum

- Date: October 23, 2019
 - To: Bill Huotari, PE
- From: Stephan Maxe, PE & Stephen Dearing, PE, PTOE
 - Re: Big Beaver Hyatt & Fords Garage Preliminary Site Plan Review

We have reviewed the Preliminary Site Plan, Traffic Impact Assessment (TIA) and Shared Parking Analysis for the Hyatt Hotel and Fords Garage restaurant. The proposed site will include a 140 room, 7-story hotel and conference center and a 8,538 SF restaurant. A 361 space 5-story parking structure will be constructed by others to serve the proposed hotel as well as the existing office building. The plans were prepared by PEA Inc, and are dated October 14th, 2019.

OHM <u>does not recommend approval</u> of the preliminary site plan, traffic impact assessment or shared parking analysis at this time, primarily due to concerns about parking discrepancies and traffic impacts.

OHM's comments are as follows:

- 1. Site Plan Review:
 - a. The proposed parking area east of the existing office building consists of angle parking and should indicate one-way traffic.
 - b. Sidewalk with overhanging parking spaces shall be 7'. The ADA spaces at the north end of the site and the 2 parking spaces in the southwest lot appear to be overhanging 5' sidewalk.
 - c. The southbound to northbound crossover on Troy Center Drive is directly across from the south access to the site. We would recommend that this crossover be relocated farther south to avoid traffic from the site blocking southbound Troy Center when entering the crossover directly.
 - d. The conceptual site plan included with the TIS states 708 parking spaces will be provided, adding up the parking spots shown on the plans the total comes to 465.
- 2. Traffic Impact Assessment Review:
 - a. The architectural drawing attached to the TIA and showing the parking calculations differs from the preliminary site plan and the provided parking.



- b. The figures showing the site generated traffic volumes appear to be incomplete. There is no traffic shown going through the I-75 interchange.
- c. The background traffic shows a total for the site driveways of 19 during the AM peak, with 17 entering and 2 exiting. During the PM peak the total is 51, with 17 entering and 34 exiting. According to ITE trip generation calculations the existing office building on site would be expected to generate approximately 76 entering and 13 exiting for a total of 89 during the AM peak. The total is 89 during the PM peak, with 14 exiting and 75 exiting. OHM questions what the building occupancy was at the time of the counts. If substantially unoccupied, the background traffic should be adjusted to reflect the trip generation of a reasonably occupied site.
- d. There is a large disparity in the collected turning volumes at Troy Center Drive and Big Beaver when compared to the MDOT study. The counts in 2017 were more than double the counts collected in 2019. These recent counts were likely impacted by Big Beaver and I-75 construction and should not be relied on. The study should instead use the 2017 counts taken for Big Beaver at Town Center and at the crossovers flanking this intersection. Regarding the site driveway volumes, reference the concern noted above.
- 3. Shared Parking Analysis
 - a. The analysis method must be revised. While Troy allows for the use of ULI / ITE methodologies for shared parking, the actual parking rates used MUST reflect city zoning ordinance required rates.
 - b. The analysis states an assumption that the 2,949 SF of conference space will be used by nonguests during the day and by hotel guests at night. The peak period for weekday parking is 2pm which is a time when the conference facilities could well be used based on this assumption. OHM believes that the conference facilities be considered in the parking analysis.

From:	Ron Wilson
To:	Eric Rabin; Brent Savidant
Cc:	Merl Potter
Subject:	RE: Prelim Site Plan Q&A.docx
Date:	Thursday, December 5, 2019 12:57:21 PM
Attachments:	Troy Planning Commission March 2018.docx
	2019 Troy Hotels .xlsx

Hi Brent,

If you could have the attached ready to put up on the screen for the Planning Commission meeting next week, that would be most helpful. I may or may not use them.

Ron Wilson - CEO Hotel Investment Services, Inc. www.his-corp.com 248-689-4110 Ext 122 From: Eric Rabin Sent: Tuesday, November 26, 2019 2:24 PM To: Brent Savidant Cc: Ron Wilson ; Merl Potter Subject: Prelim Site Plan Q&A.docx

Brent,

Here is the questionnaire I believe you were referring to, we did send this over with are submittal package.

I have included Ron Wilson, and Merl Potter (Owners rep for hotel, restaurant, and parking deck) on this email in efforts of streamlining coordination for planning commission meeting. We will send you the videos and any PowerPoint slides that we would like to present. Per your suggestion, we will assume the following flow of the presentation.

1) Brent or Ben will present project

- 2) We will then show brief videos
- 3) Dennis Bostick probably give brief intro

4) Ron Willson shares

5) Merl Potter (Can share about architecture and construction for parking deck, restaurant, and hotel.

6) Questions

We appreciate your input, and any further insights? It is our goal to send over a full package of what we would like to share early next week.

Let us know any questions you may have?

Thanks,

Eric 561.212.5687 Eric W. Rabin

OTHER BUSINESS

 <u>HOTELS IN THE CITY OF TROY</u> – Presentation by Ron Wilson of Hotel Investment Services, Inc.

Mr. Savidant gave a brief introduction of Ron Wilson.

Mr. Wilson gave an excellent presentation on the hotel industry, addressing data within the STR (Smith Travel Research) report prepared for the City of Troy. He said the data reveals an overall good and healthy hotel market for the City based on supply and demand, noting the City could well be served by another three to four hotels. Mr. Wilson encouraged the Commission to look for good hotel brands and good hotel operators in their consideration of potential hotels in the City.

Troy Hotels as of 2019

TripAdvior.com Rank	Hotel	Open Date	# of Rooms	Category - Address	Typical Peak Room Rate	Typical Off-Peak Room Rate
9	Embassy Suites by Hilton	May 1987	251	Upper Upscale - 850 Tower Dr.	\$ 227	\$ 128
6	Marriott	February 1990	350	Upper Upscale - 200 W. Big Beaver Rd.	356	129
	HYATT PLACE		137	Upper Upscale - 801 W. Big Beaver Rd.	275	129
13	Courtyard	June 1986	147	Upscale - 1525 East Maple Rd.	161	109
4	Hilton Garden Inn	April 2016	114	Upscale - 200 Wilshire Dr.	227	112
2	Homewood Suites by Hilton	September 2002	150	Upscale - 1495 Equity Dr.	151	107
3	Drury Inn & Suites	May 1984	216	Upper Mid - 575 W. Big Beaver	182	105
12	Fairfield Inn & Suites by Marriott	November 2016	90	Upper Mid - 225 Stephenson Highway	176	114
5	Hampton Inn	May 2015	122	Upper Mid - 100 Wilshire Dr.	197	109
7	Holiday Inn Express & Suites	May 2013	117	Upper Mid - 400 Stephenson Highway	135	95
15	Townplace Suites	August 2016	87	Upper Mid - 325 Stephenson Highway	176	109
1	Candlewood Suites	June 1998	118	Midscale - 2550 Troy Center Dr.	103	85
11	Hawthorn Suites by Wyndham	October 1985	152	Midscale - 2600 Livernois	101	99
14	Quality Inn	November 1971	150	Midscale - 2537 Rochester Court	76	69
8	Red Roof Inn	November 1975	109	Economy - 2350 Rochester Court	52	64
10	Sommerset Inn	June 1973	250	Independent - 2601 W. Big Beaver	139	129
			2,423	with Hyatt Place 2,560		
	UNDER DEVELOPMENT	Building Permit Issued?				
	Springhill Suites Hotel-Proposed & Approved May 14, 2019	No	123	East Side of Rochester, South of Big Beaver		
	Holiday Inn Hotel -Proposed & Approved Feb. 26, 2019	No	136	East of Crooks, North side of Tower - 900 Tower		
	Home 2 Suites-Proposed & Approved in May 15, 2018	Yes	97	North of Big Beaver, West of Crooks		
	Tru Hotel-Proposed & Approved March 13, 2018	Yes	124	NE corner of Maple and Research adjacent to I-75		
	Marriott Courtyard - Plans just submitted	No	133	North off Crooks		

PRELIMINARY SITE PLANS FOR HYATT PLACE PART OF THE NE SECTION 28, T. 2N., R. 11E. CITY OF TROY, OAKLAND COUNTY, MICHIGAN

OWNER:

801 BOSTICK LLC 801 W. BIG BEAVER ROAD TROY, MICHIGAN 48084 CONTACT: DENNIS BOSTICK PHONE: (248) 509-7977 EMAIL: DENNISBOSTICK@ATT.NET

CONTACT: ERIC RABIN PHONE: (561) 212–5687 EMAIL: ERICRABIN@ATT.NET

ARCHITECT (HOTEL/RESTAURANT):

LEGAT ARCHITECTS 2015 SPRING ROAD, SUITE 175 OAK BROOK, ILLINOIS 60523 CONTACT: APRIL MAIFIELD, AIA, NCARB, LEED AP BD+C PHONE: (630) 645–1935 EMAIL: AMAIFÍELD@LEGAT.COM

ARCHITECT (PARKING DECK):

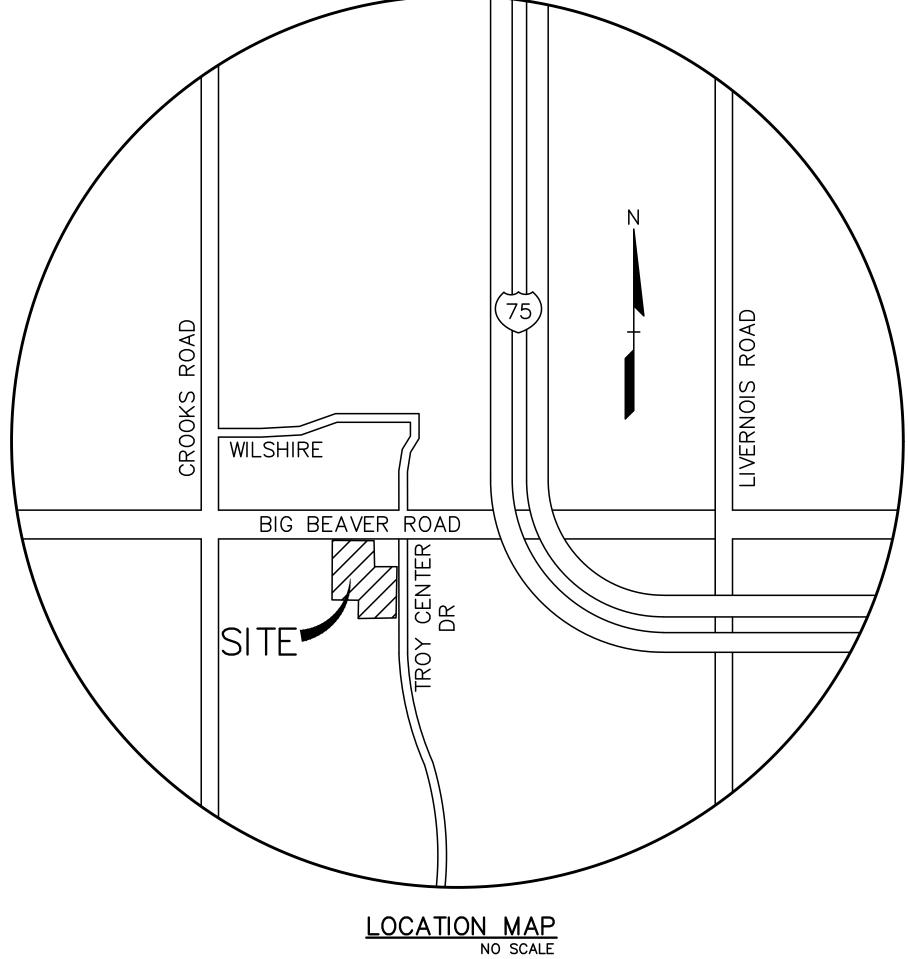
RICH & ASSOCIATES 26877 NORTHWESTERN HIGHWAY, SUITE 208 SOUTHFIELD, MICHIGAN 48033 CONTACT: MATTHEW J. JOBIN, RA PHONE: (248) 353-5080 FAX: (248-353-3830 EMAIL: RICHASSOC.COM

CIVIL ENGINEER:

PEA, INC. 2430 ROCHESTER CT, SUITE 100 TROY, MI 48083 CONTACT: BECKY KLEIN, PE PHONE: (248) 689-9090 EXT. 1157 FAX: (248) 689-1044 EMAIL: BKLEIN@PEAINC.COM

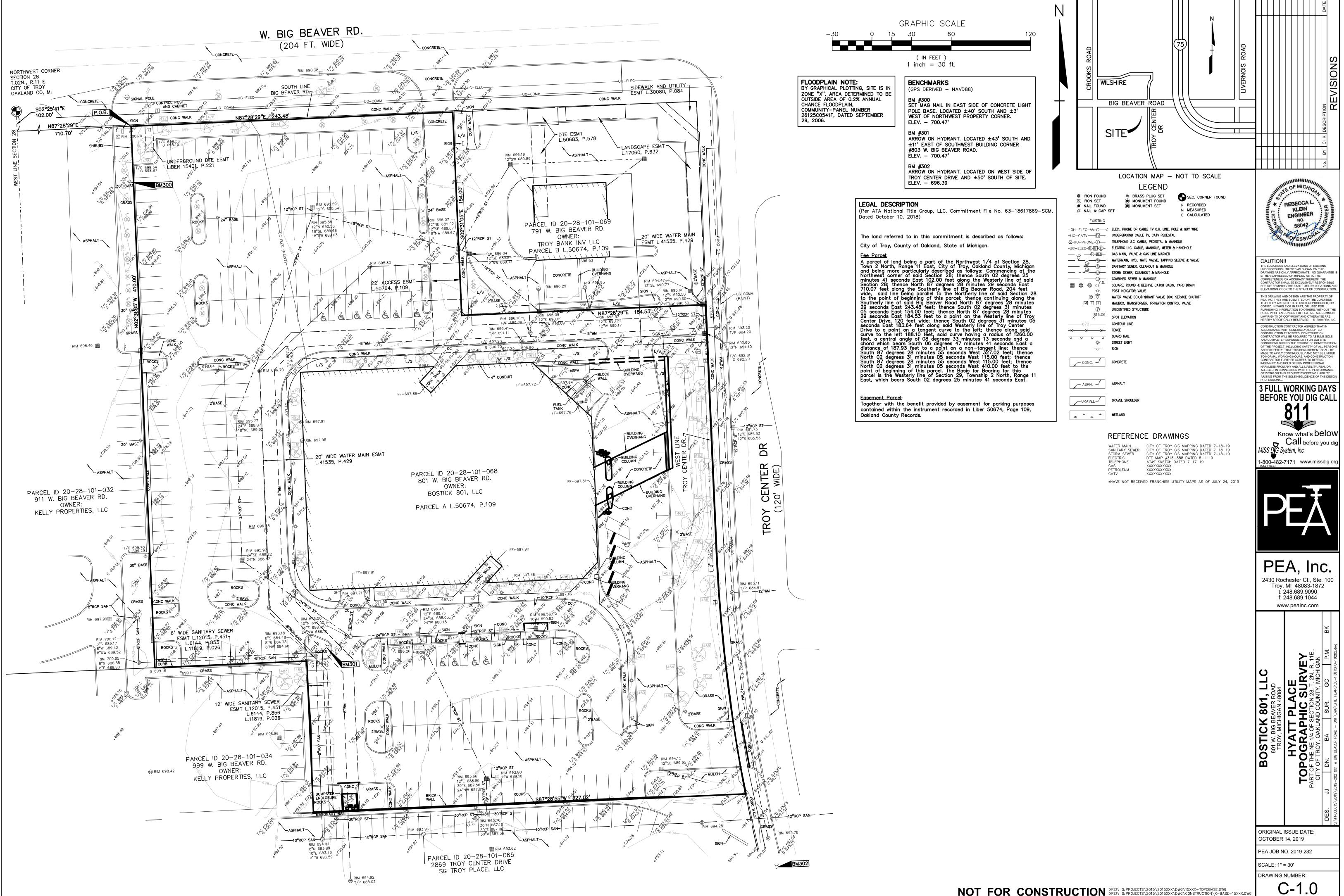
LANDSCAPE ARCHITECT:

PEA, INC. 7927 NEMCO WAY, SUITE 115 BRIGHTON, MI 48116 CONTACT: JANET EVANS, P.L.A., PHONE: (517) 546-8583 FAX: (517) 546-8973 EMAIL: JEVANS@PEAINC.COM

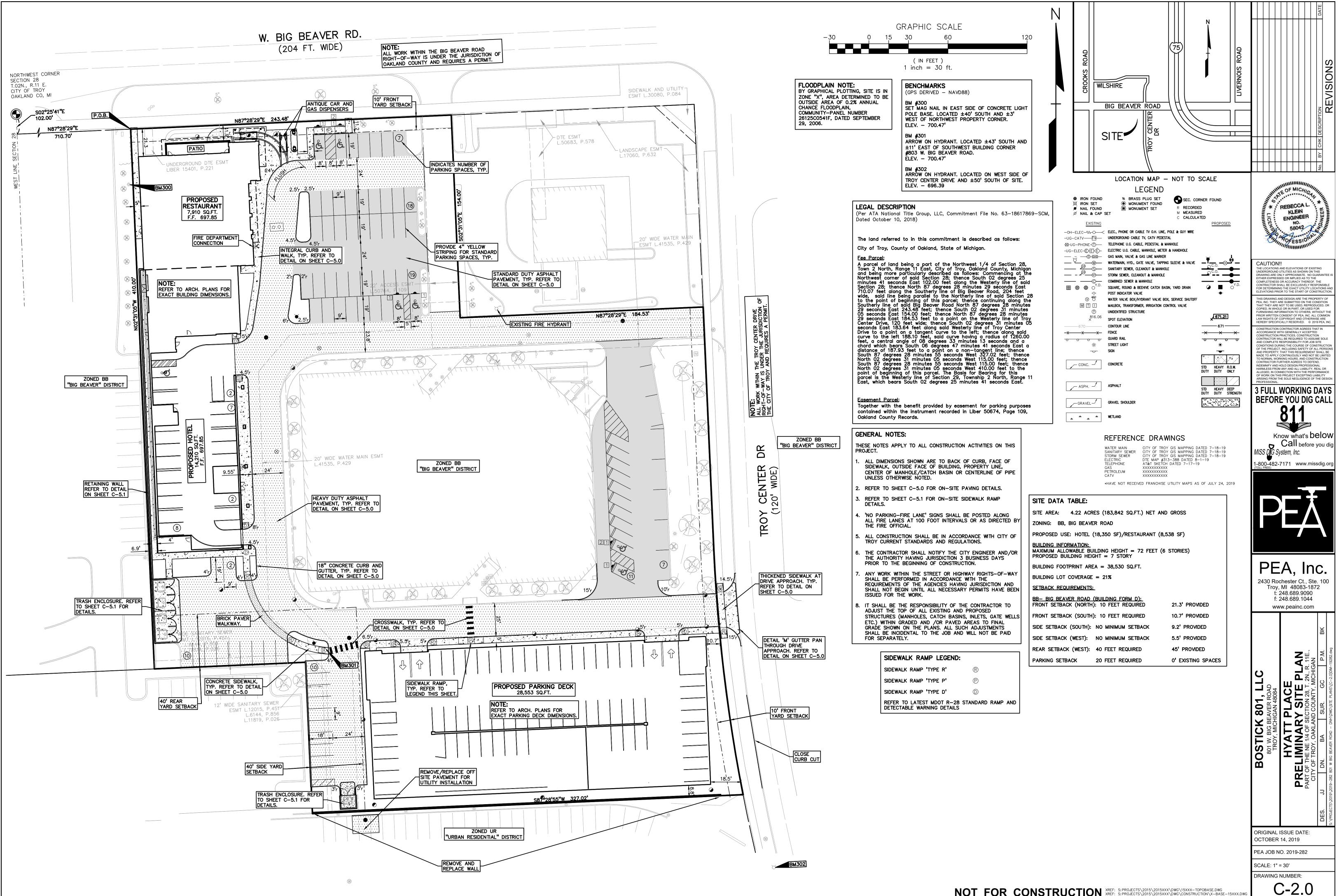


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		CAUTION!! THE LOCATIONS AND ELEVATIONS OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THIS DRAWING ARE ONLY APPROXIMATE. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED AS TO THE COMPLETENESS OR ACCURACY THEREOF. THE COMPLETENESS OR ACCURACY THEREOF. THE CONTRACTOR SHALL BE EXCLUSIVELY RESPONSIBLE FOR DETERMINING THE EXACT UTILITY LOCATIONS AND ELEVATIONS PRIOR TO THE START OF CONSTRUCTION. THIS DRAWING AND DESIGN ARE THE PROPERTY OF PEA, INC. THEY ARE SUBMITTED ON THE CONDITION THAT THEY ARE NOT TO BE USED. REPRODUCED, OR CODIED. IN WHOLE OF DIN DRAFT OR USED. EDEN
INDEX	OF DRAWINGS	COPIED, IN WHOLE OR IN PART, OR USED FOR FURNISHING INFORMATION TO OTHERS, WITHOUT THE PRIOR WRITTEN CONSENT OF PEA, INC. ALL COMMON LAW RIGHTS OF COPYRIGHT AND OTHERWISE ARE HEREBY SPECIFICALLY RESERVED. © 2019 PEA, INC.
C-1.0 C-2.0 C-3.0 C-4.0	COVER SHEET TOPOGRAPHIC SURVEY PRELIMINARY SITE PLAN PRELIMINARY GRADING PLAN PRELIMINARY UTILITY PLAN	CONSTRUCTION CONTRACTOR AGREES THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONSTRUCTION CONTRACTOR WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY, THAT THIS REQUIREMENT SHALL BE MADE TO APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS, AND CONSTRUCTION CONTRACTOR FURTHER AGREES TO DEFEND, INDEMNIFY AND HOLD DESIGN PROFESSIONAL HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT EXCEPTING LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE DESIGN PROFESSIONAL.
C-5.0 L-1.0 L-1.1 T-1.0	NOTES AND DETAILS LANDSCAPE PLAN LANDSCAPE DETAILS TREE PRESERVATION PLAN	BEFORE YOU DIG CALL 811 Know what's below Call before you dig
	PRELIMINARY PHOTOMETRIC PLAN PRELIMINARY PHOTOMETRIC DETAILS	MISS DG System, Inc. 1-800-482-7171 www.missdig.org
	TECTURAL DRAWINGS	
A001 A101 A101B A102 A103 A104 A105 A106	ARCHITECTURAL SITE PLAN FIRST FLOOR PLAN BUILDING 2 FLOOR PLANS SECOND FLOOR PLAN THIRD FLOOR PLAN FOURTH FLOOR PLAN FIFTH FLOOR PLAN SIXTH FLOOR PLAN	PEA, Inc. 2430 Rochester Ct., Ste. 100 Troy, MI 48083-1872 t: 248.689.9090 f: 248.689.1044 www.peainc.com
A107 A201 A202 A203	SEVENTH FLOOR PLAN EXTERIOR BUILDING ELEVATIONS EXTERIOR BUILDING ELEVATIONS EXTERIOR BUILDING ELEVATIONS	11E., N P.M. BK
A900 A901	PERSPECTIVE VIEWS PRELIMINARY RENDERINGS	C . 2N, R. 11 MICHIGAN . 2N, R. 11 . 2N,
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		ORIGINAL ISSUE DATE: OCTOBER 14, 2019 PEA JOB NO. 2019-282
		SCALE: N.T.S.
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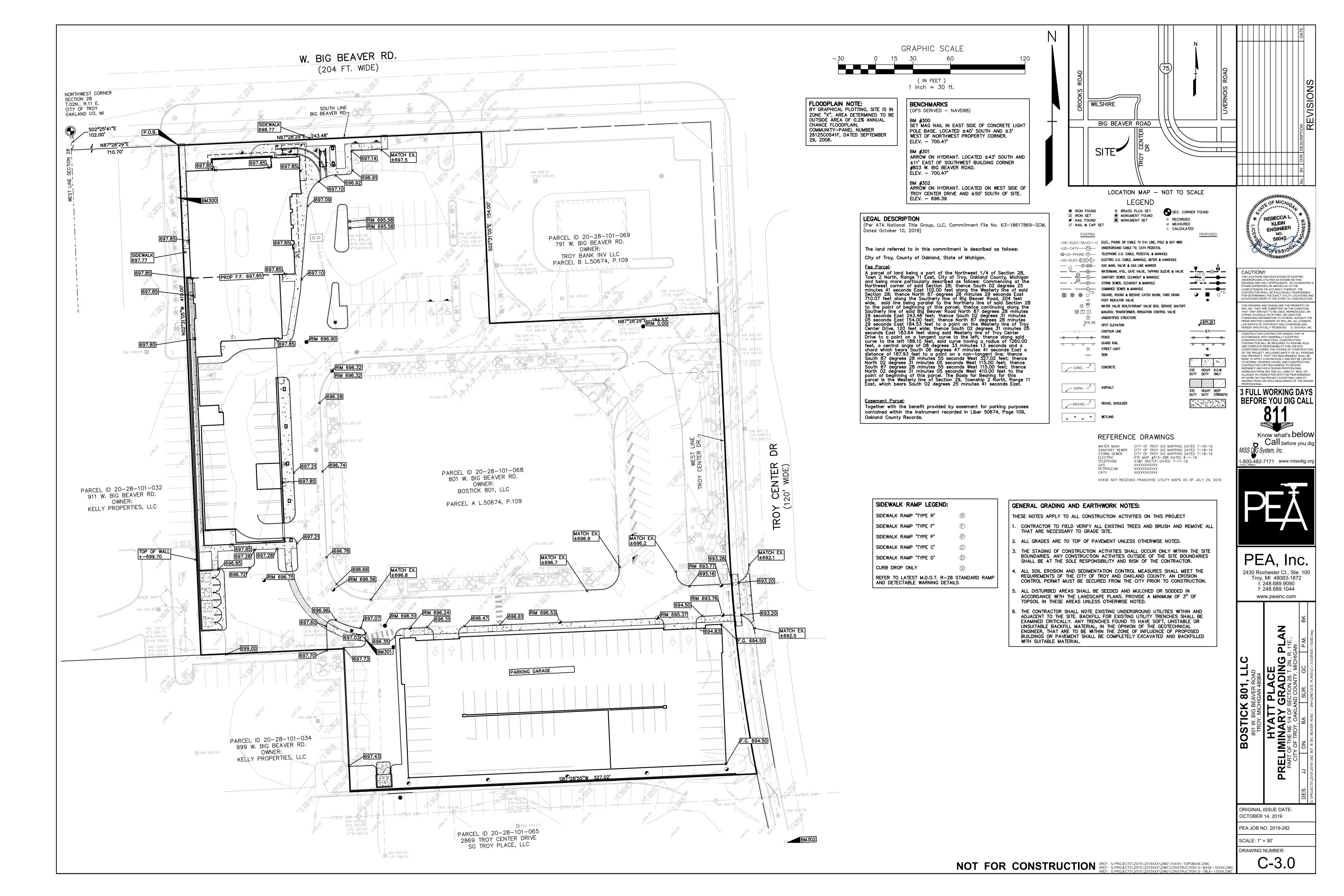
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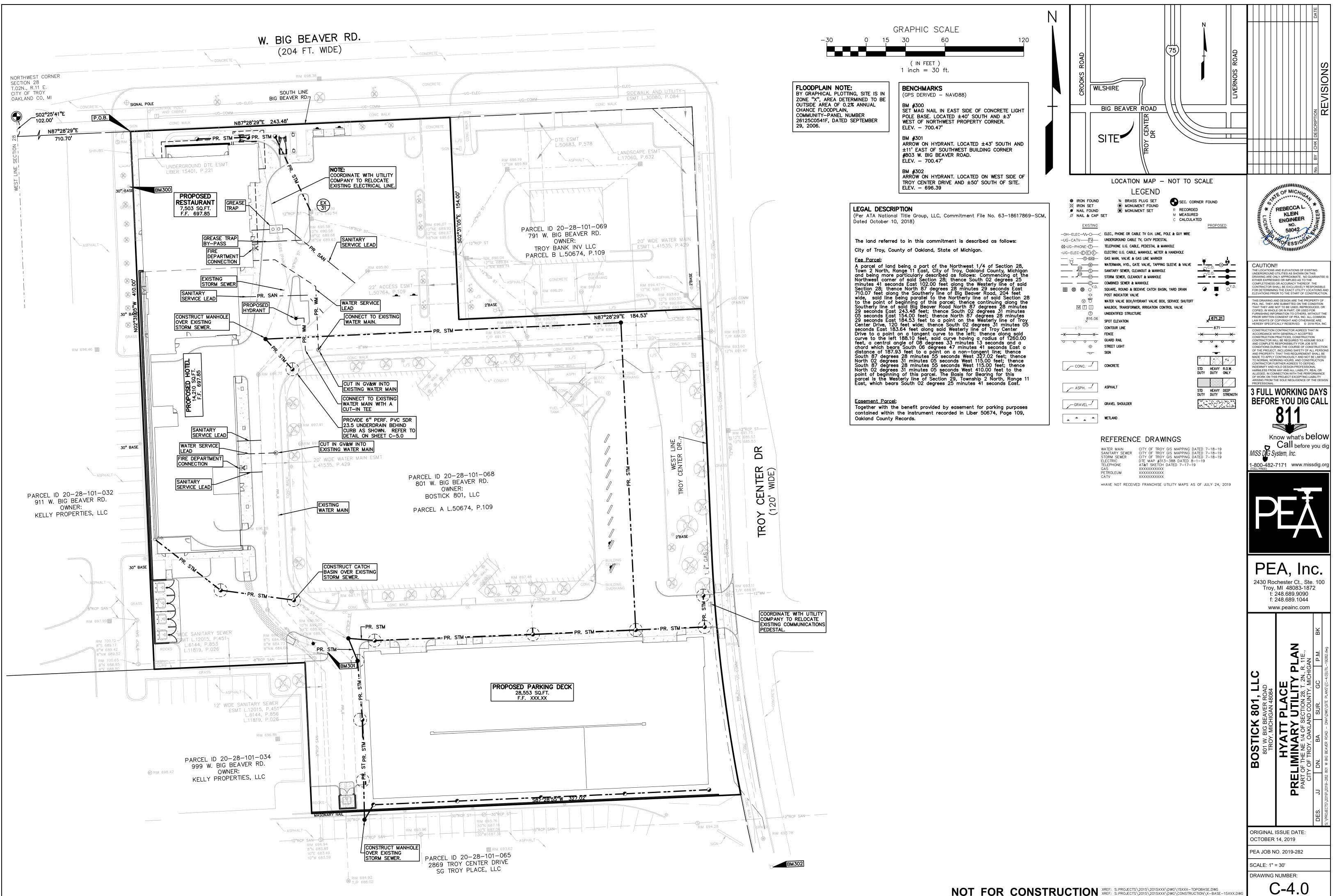


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GENERAL NOTES:

- ALL CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT STANDARDS AND SPECIFICATIONS OF THE CITY OF TROY AND OAKLAND COUNTY.
- THE CONTRACTOR MUST CONTACT THE ENGINEER SHOULD THEY ENCOUNTER ANY DESIGN ISSUES DURING CONSTRUCTION. IF THE CONTRACTOR MAKES DESIGN MODIFICATIONS WITHOUT THE WRITTEN DIRECTION OF THE DESIGN ENGINEER, THE CONTRACTOR DOES SO AT HIS OWN RISK.
- ALL NECESSARY PERMITS, TESTING, BONDS AND INSURANCES ETC., SHALL BE PAID FOR BY THE CONTRACTOR. THE OWNER SHALL PAY FOR ALL CITY INSPECTION FEES.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DUST CONTROL DURING THE PERIODS OF CONSTRUCTION. THIS SHALL BE CONSIDERED INCIDENTAL TO THE JOB.
- THE CONTRACTOR SHALL NOTIFY MISS DIG (811) AND REPRESENTATIVES OF OTHER UTILITIES IN THE VICINITY OF THE WORK A MINIMUM OF 72 HOURS PRIOR TO START OF CONSTRUCTION (EXCLUDING WEEKENDS AND HOLIDAYS) FOR LOCATION AND STAKING OF ON-SITE UTILITY LINES. IF NO NOTIFICATION IS GIVEN AND DAMAGE RESULTS, SAID DAMAGE WILL BE REPAIRED AT SOLE EXPENSE OF THE CONTRACTOR. IF EXISTING UTILITY LINES ARE ENCOUNTERED THAT CONFLICT IN LOCATION WITH NEW CONSTRUCTION, THE CONTRACTOR SHALL NOTIFY THE DESIGN ENGINEER SO THAT THE CONFLICT MAY BE RESOLVED.
- CONTRACTOR TO VERIFY THAT THE PLANS AND SPECIFICATIONS ARE THE VERY LATEST PLANS AND SPECIFICATIONS AND FURTHERMORE, VERIFY THAT THESE PLANS AND SPECIFICATIONS HAVE BEEN APPROVED. ALL ITEMS CONSTRUCTED BY THE CONTRACTOR PRIOR TO RECEIVING FINAL APPROVAL, HAVING TO BE ADJUSTED OR RE-DONE, SHALL BE AT THE CONTRACTORS EXPENSE. SHOULD THE CONTRACTOR ENCOUNTER A CONFLICT BETWEEN THESE PLANS AND/OR SPECIFICATIONS, THEY SHALL SEEK CLARIFICATION IN WRITING FROM THE ENGINEER BEFORE COMMENCEMENT OF CONSTRUCTION. FAILURE TO DO SO SHALL BE AT SOLE EXPENSE TO THE CONTRACTOR.
- ALL PROPERTIES OR FACILITIES IN THE SURROUNDING AREAS, PUBLIC OR PRIVATE, DESTROYED OR OTHERWISE DISTURBED DUE TO CONSTRUCTION, SHALL BE REPLACED AND/OR RESTORED TO THE ORIGINAL CONDITION BY THE CONTRACTOR.
- 8. MANHOLE, CATCH BASIN, GATE VALVES AND HYDRANT FINISH GRADES MUST BE CLOSELY CHECKED AND APPROVED BY THE ENGINEER BEFORE THE CONTRACTOR'S WORK IS CONSIDERED COMPLETE.
- CONTRACTOR SHALL REMOVE AND DISPOSE OF OFF-SITE ANY TREES, BRUSH, STUMPS, TRASH OR OTHER UNWANTED DEBRIS AT THE OWNER'S DIRECTION, INCLUDING OLD BUILDING FOUNDATIONS AND FLOORS. BURNING OF TRASH, STUMPS OR OTHER DEBRIS SHALL NOT BE PERMITTED.
- 10. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY BARRICADING, SIGNAGE, LIGHTS AND TRAFFIC CONTRO DEVICES TO PROTECT THE WORK AND SAFELY MAINTAIN TRAFFIC IN ACCORDANCE WITH LOCAL REQUIREMENTS AND THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (LATEST EDITION). THE DESIGN ENGINEER, OWNER, CITY AND STATE SHALL NOT BE HELD LIABLE FOR ANY CLAIMS RESULTING FROM ACCIDENTS OR DAMAGES CAUSED BY THE CONTRACTOR'S FAILURE TO COMPLY WITH TRAFFIC AND PUBLIC SAFETY REGULATIONS DURING THE CONSTRUCTION PERIOD.
- ALL EXCAVATIONS SHALL BE SLOPED, SHORED OR BRACED IN ACCORDANCE WITH MI-OSHA REQUIREMENTS. THE CONTRACTOR SHALL PROVIDE AN ADEQUATELY CONSTRUCTED AND BRACED SHORING SYSTEM FOR EMPLOYEES WORKING IN AN EXCAVATION THAT MAY EXPOSE EMPLOYEES TO THE DANGER OF MOVING GROUND.

PAVING NOTES:

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT STANDARDS AND SPECIFICATIONS OF THE CITY OF TROY AND M.D.O.T.
- 2. IN AREAS WHERE NEW PAVEMENTS ARE BEING CONSTRUCTED, THE TOPSOIL AND SOIL CONTAINING ORGANIC MATTER SHALL BE REMOVED PRIOR TO PAVEMENT CONSTRUCTION.
- 3. ON-SITE FILL CAN BE USED IF THE SPECIFIED COMPACTION REQUIREMENTS CAN BE ACHIEVED. IF ON-SITE SOIL IS USED, IT SHOULD BE CLEAN AND FREE OF FROZEN SOIL, ORGANICS, OR OTHER DELETERIOUS MATERIALS.
- THE FINAL SUBGRADE/EXISTING AGGREGATE BASE SHOULD BE THOROUGHLY PROOFROLLED USING A FULLY LOADED TANDEM AXLE TRUCK OR FRONT END LOADER UNDER THE OBSERVATION OF A GEOTECHNICAL/PAVEMENT ENGINEER. LOOSE OR YIELDING AREAS THAT CANNOT BE MECHANICALLY STABILIZED SHOULD BE REINFORCED USING GEOGRIDS OR REMOVED AND REPLACED WITH ENGINEERED FILL OR AS DICTATED BY FIELD CONDITIONS.
- SUBGRADE UNDERCUTTING, INCLUDING BACKFILLING SHALL BE PERFORMED TO REPLACE MATERIALS SUSCEPTIBLE TO FROST HEAVING AND UNSTABLE SOIL CONDITIONS. ANY EXCAVATIONS THAT MAY BE REQUIRED BELOW THE TOPSOIL IN FILL SECTIONS OR BELOW SUBGRADE IN CUT SECTIONS, WILL BE CLASSIFIED AS SUBGRADE UNDERCUTTING.
- SUBGRADE UNDERCUTTING SHALL BE PERFORMED WHERE NECESSARY AND THE EXCAVATED MATERIAL SHALL BECOME THE PROPERTY OF THE CONTRACTOR. ANY SUBGRADE UNDERCUTTING SHALL BE BACKFILLED WITH SAND OR OTHER SIMILAR APPROVED MATERIAL. BACKFILL SHALL BE COMPACTED TO 95% OF THE MAXIMUM UNIT WEIGHT (PER ASTM D-1557) UNLESS OTHERWISE SPECIFIED.
- 7. BACKFILL UNDER PAVED AREAS SHALL BE AS SPECIFIED ON DETAILS.
- 8. ANY SUB-GRADE WATERING REQUIRED TO ACHIEVE REQUIRED DENSITY SHALL BE CONSIDERED INCIDENTAL TO THE JOB.
- 9. FINAL PAVEMENT ELEVATIONS SHOULD BE SO DESIGNED TO PROVIDE POSITIVE SURFACE DRAINAGE. A MINIMUM SURFACE SLOPE OF 1.0 PERCENT IS RECOMMENDED.
- 10. CONSTRUCTION TRAFFIC SHOULD BE MINIMIZED ON THE NEW PAVEMENT. IF CONSTRUCTION TRAFFIC IS ANTICIPATED ON THE PAVEMENT STRUCTURE, THE INITIAL LIFT THICKNESS COULD BE INCREASED AND PLACEMENT OF THE FINAL LIFT COULD BE DELAYED UNTIL THE MAJORITY OF THE CONSTRUCTION ACTIVITIES HAVE BEEN COMPLETED. THIS ACTION WILL ALLOW REPAIR OF LOCALIZED FAILURE, IF ANY DOES OCCUR, AS WELL AS REDUCE LOAD DAMAGE ON THE PAVEMENT SYSTEM.

GENERAL UTILITY NOTES:

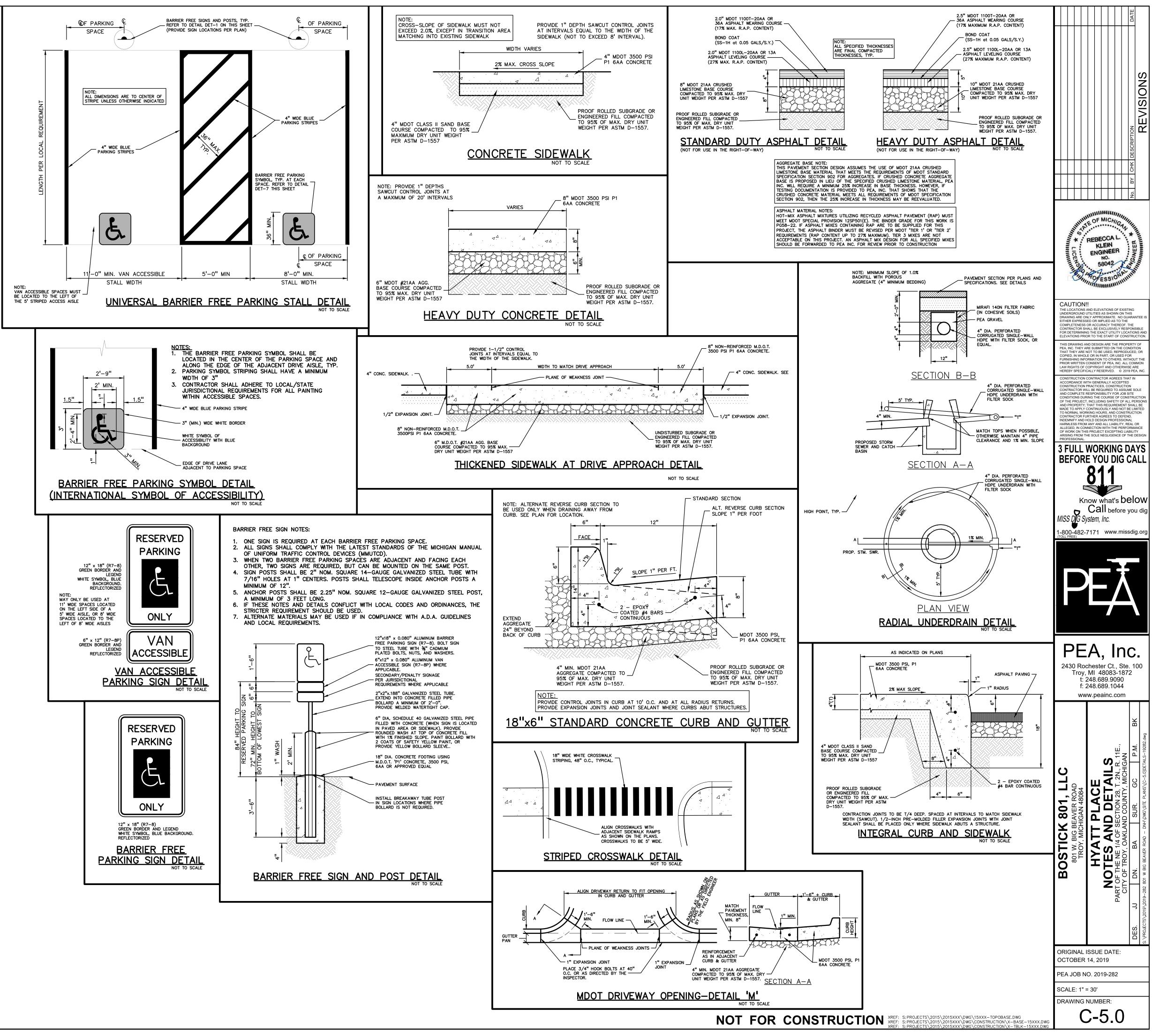
- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE CITY OF TROY AND M.D.O.T.
- 2. ALL TRENCHES UNDER OR WITHIN FIVE (5) FEET OR THE FORTY-FIVE (45) DEGREE ZONE OF INFLUENCE LINE OF EXISTING AND/OR PROPOSED PAVEMENT, BUILDING PAD OR DRIVE APPROACH SHALL BE BACKFILLED WITH SAND COMPACTED TO AT LEAST NINETY-FIVE (95) PERCENT OF MAXIMUM UNIT WEIGHT (ASTM D-1557). ALL OTHER TRENCHES TO BE COMPACTED TO 90% OR BETTER.
- WHENEVER EXISTING MANHOLES OR SEWER PIPE ARE TO BE TAPPED, DRILL HOLES 4" CENTER TO CENTER, AROUND PERIPHERY OF OPENING TO CREATE A PLANE OF WEAKNESS JOINT BEFORE BREAKING SECTION OUT.
- THE LOCATIONS AND DIMENSIONS SHOWN ON THE PLANS FOR EXISTING UTILITIES ARE IN ACCORDANCE WITH AVAILABLE INFORMATION WITHOUT UNCOVERING AND MEASURING. THE DESIGN ENGINEER DOES NOT GUARANTEE THE ACCURACY OF THIS INFORMATION OR THAT ALL EXISTING UNDERGROUND FACILITIES ARE SHOWN. CONTRACTOR TO FIELD VERIFY UTILITIES.
- THE CONTRACTOR MUST COORDINATE TO ENSURE ALL REQUIRED PIPES, CONDUITS, CABLES AND SLEEVES ARE PROPERTLY PLACED FOR THE INSTALLATION OF GAS, ELECTRIC, PHONE, CABLE, IRRIGATION, ETC. IN SUCH A MANNER THAT WILL FACILITATE THEIR PROPER INSTALLATION PRIOR TO THE PLACEMENT OF THE PROPOSED PAVEMENT AND LANDSCAPING.
- 6. REFER TO CITY OF TROY STANDARD DETAILS FOR PIPE BEDDING DETAILS.
- 7. REFER TO CITY OF TROY STANDARD DETAIL SHEETS FOR ADDITIONAL INFORMATION.

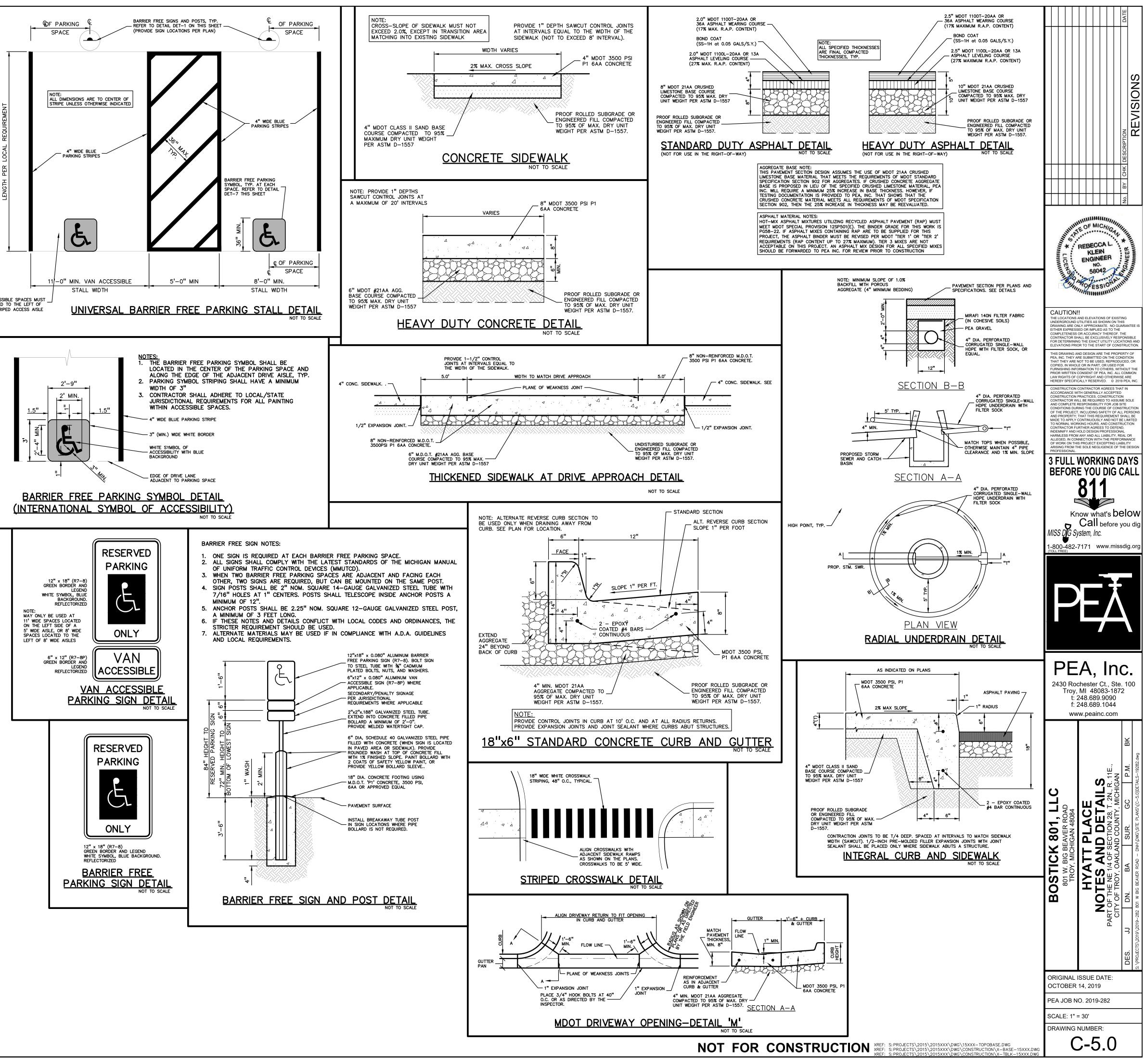
STORM SEWER NOTES:

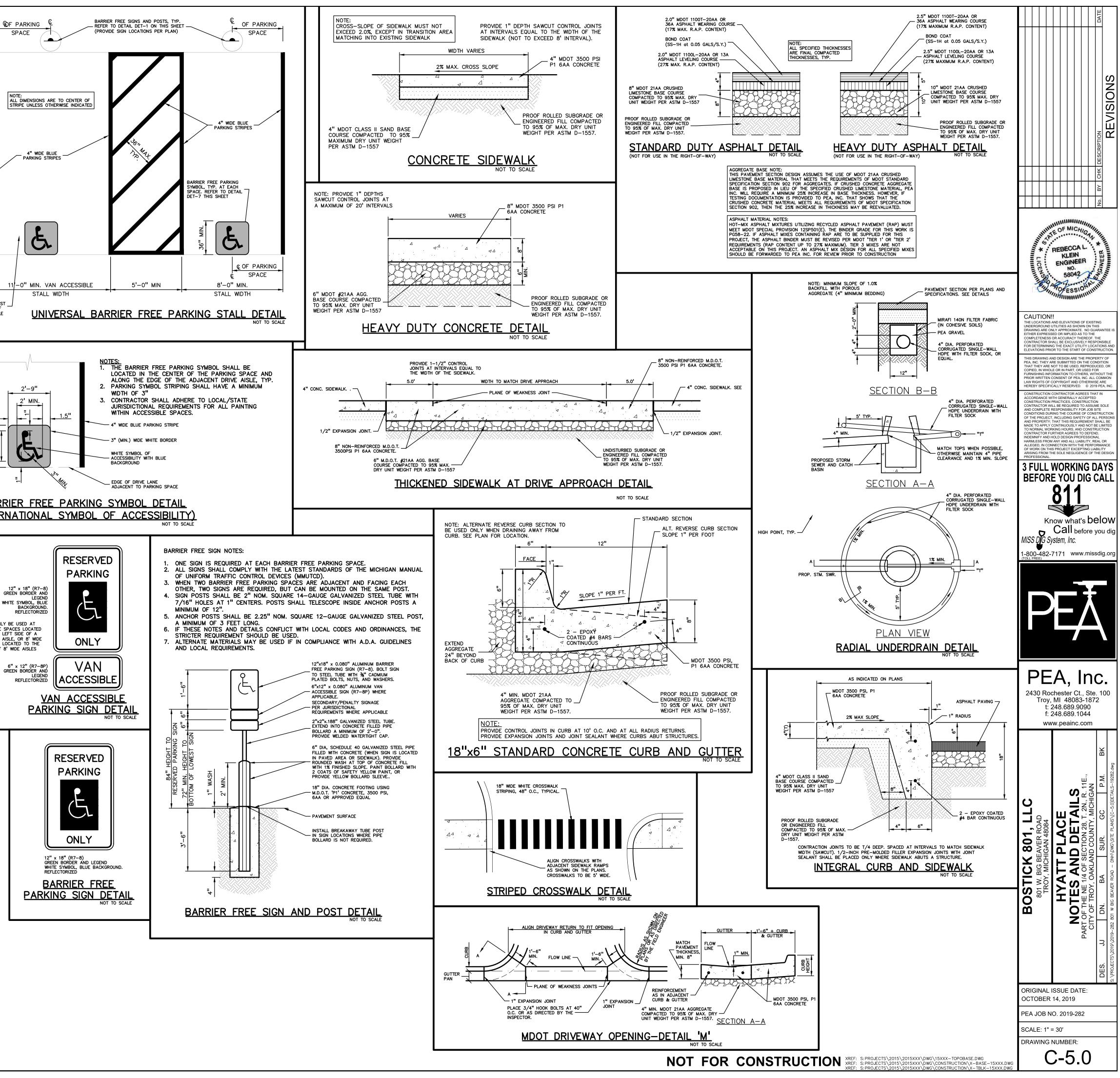
- ALL STORM SEWER 12" AND LARGER SHALL BE RCP CLASS IV UNLESS OTHERWISE NOTED. REFER TO CITY STANDARD DETAILS SHEETS FOR STANDARD BEDDING DETAILS.
- 2. JOINTS FOR ALL STORM SEWER 12" AND LARGER SHALL BE MODIFIED TONGUE AND GROOVE JOINT WITH RUBBER GASKETS UNLESS SPECIFIED OTHERWISE (ASTM C-443)

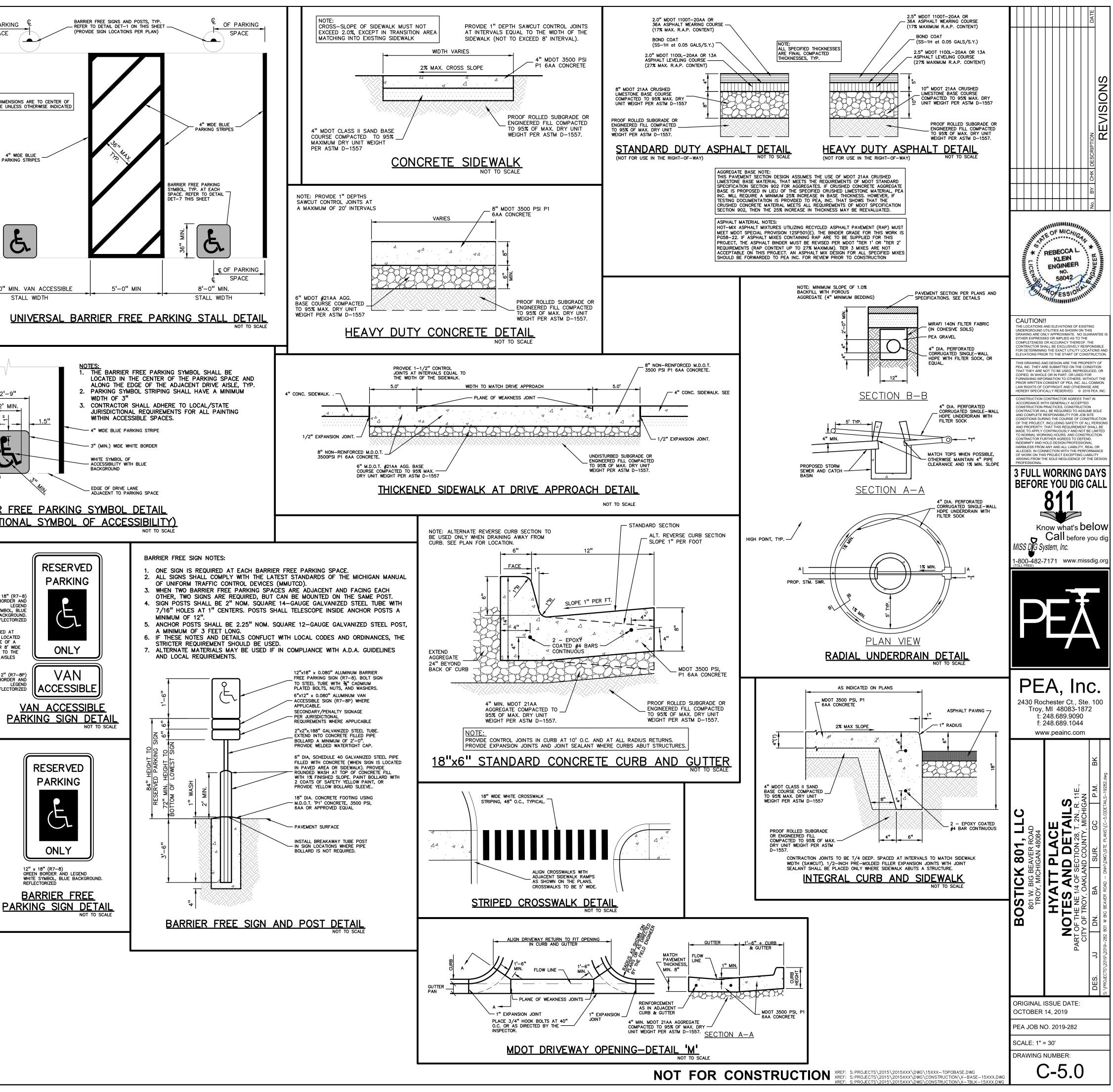
WATER MAIN NOTES:

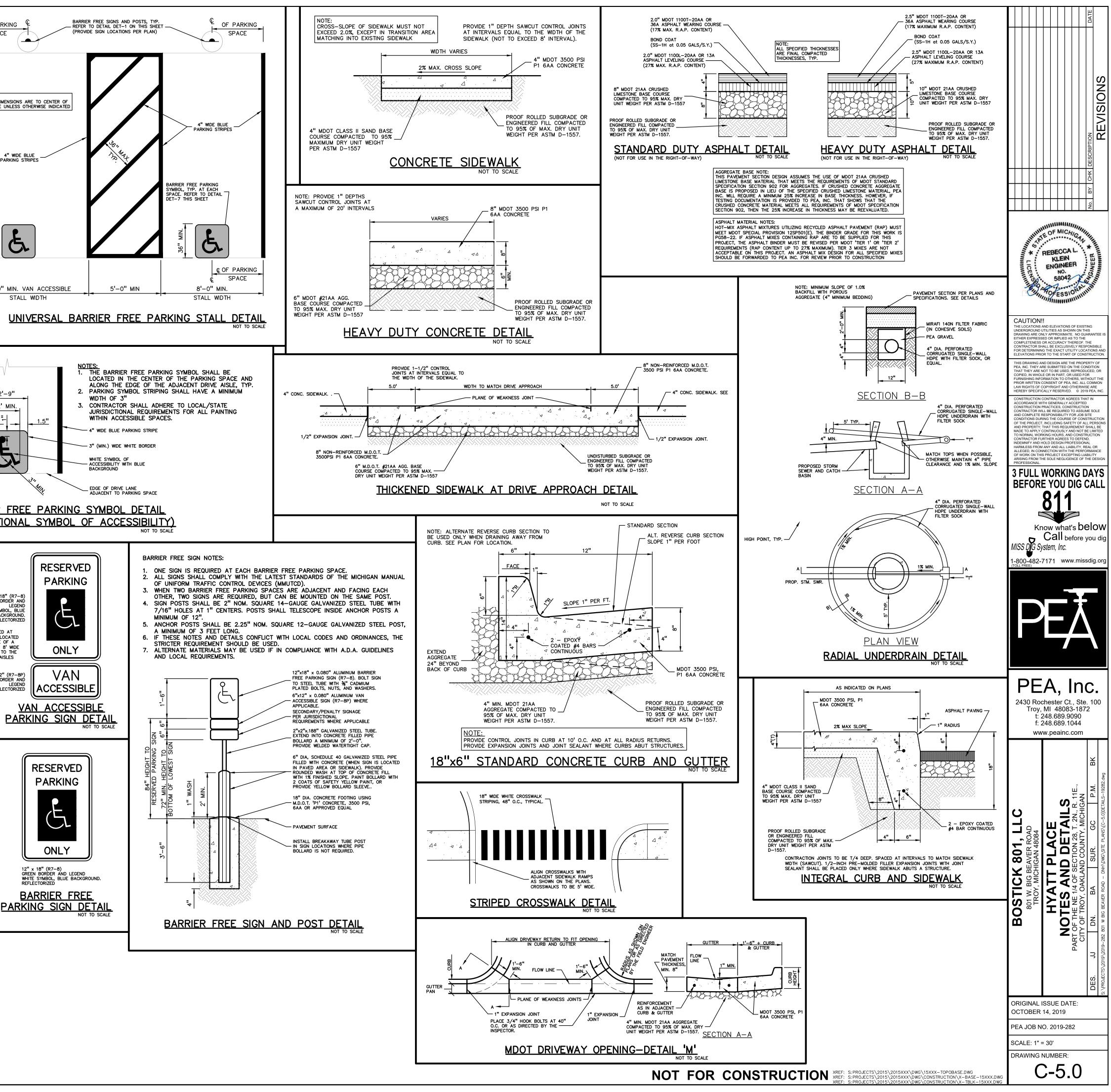
- ALL WATER MAIN SHALL BE INSTALLED WITH A MINIMUM COVER OF 6.0' BELOW FINISH GRADE. WHEN WATER MAINS MUST DIP TO PASS UNDER A STORM SEWER OR SANITARY SEWER, THE SECTIONS WHICH ARE DEEPER THAN NORMAL SHALL BE KEPT TO A MINIMUM LENGTH BY THE USE OF VERTICAL TWENTY TWO AND A HALF (22.5°) DEGREE BENDS, PROPERLY ANCHORED.
- 2. ALL TEE'S, BENDS, CONNECTIONS, ETC. ARE CONSIDERED INCIDENTAL TO THE JOB.
- 3. PHYSICAL CONNECTIONS SHALL NOT BE MADE BETWEEN EXISTING AND NEW WATER MAINS UNTIL TESTING IS SATISFACTORILY COMPLETED.
- . MAINTAIN 10' HORIZONTAL CLEARANCE BETWEEN OUTER EDGE OF WATERMAIN AND ANY SANITARY SEWER OR STRUCTURE. 5. ALL WATER MAIN SHALL BE DUCTILE IRON CLASS 54 WITH POLYETHYLENE WRAP.
- SANITARY SEWER NOTES:
- DOWNSPOUTS, WEEP TILE, FOOTING DRAINS OR ANY CONDUIT THAT CARRIES STORM OR GROUND WATER SHALL NOT BE ALLOWED TO DISCHARGE INTO A SANITARY SEWER.
- 2. ALL SANITARY SEWER SHALL BE CONSTRUCTED OF RCP CLASS IV PIPE WITH A MINIMUM DIAMETER OF 12" PER CITY REQUIREMENTS.
- 3. JOINTS FOR ALL CONCRETE SANITARY SEWER 12" AND LARGER SHALL BE MODIFIED TONGUE AND GROOVE JOINT WITH RUBBER GASKETS UNLESS SPECIFIED OTHERWISE.

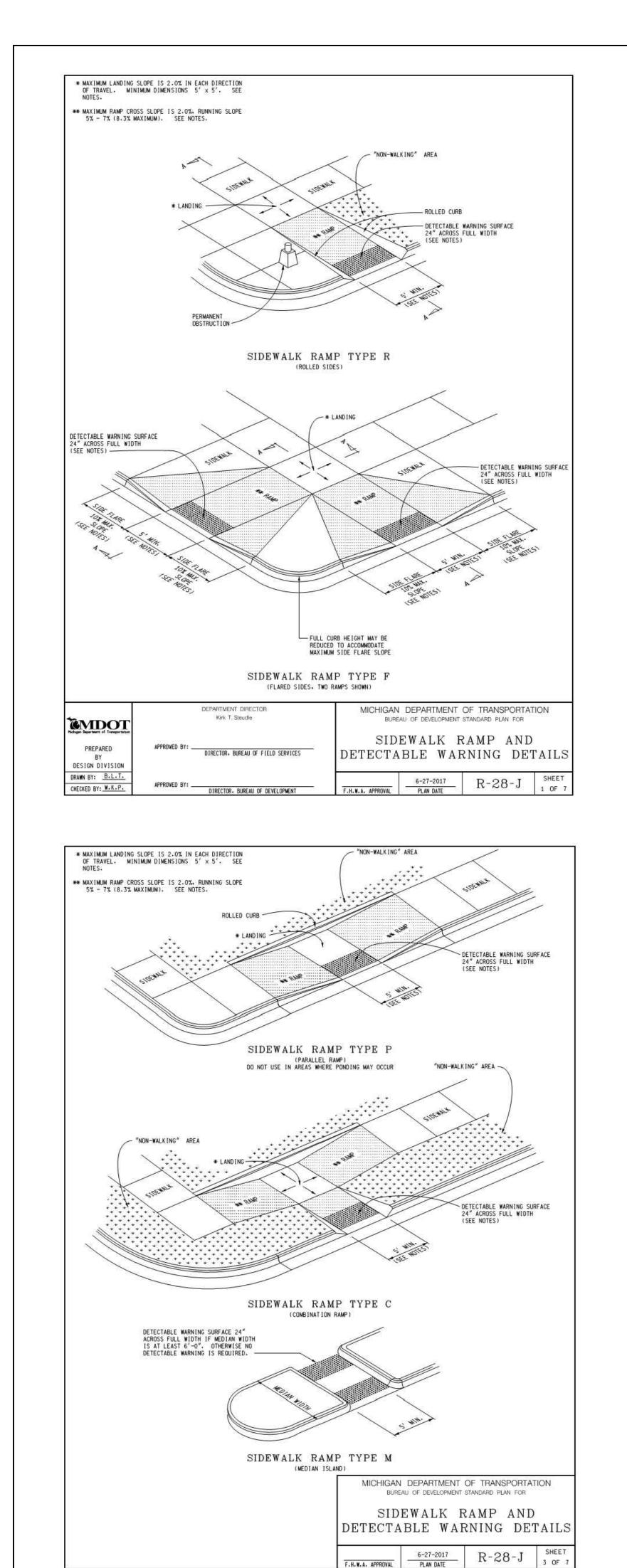




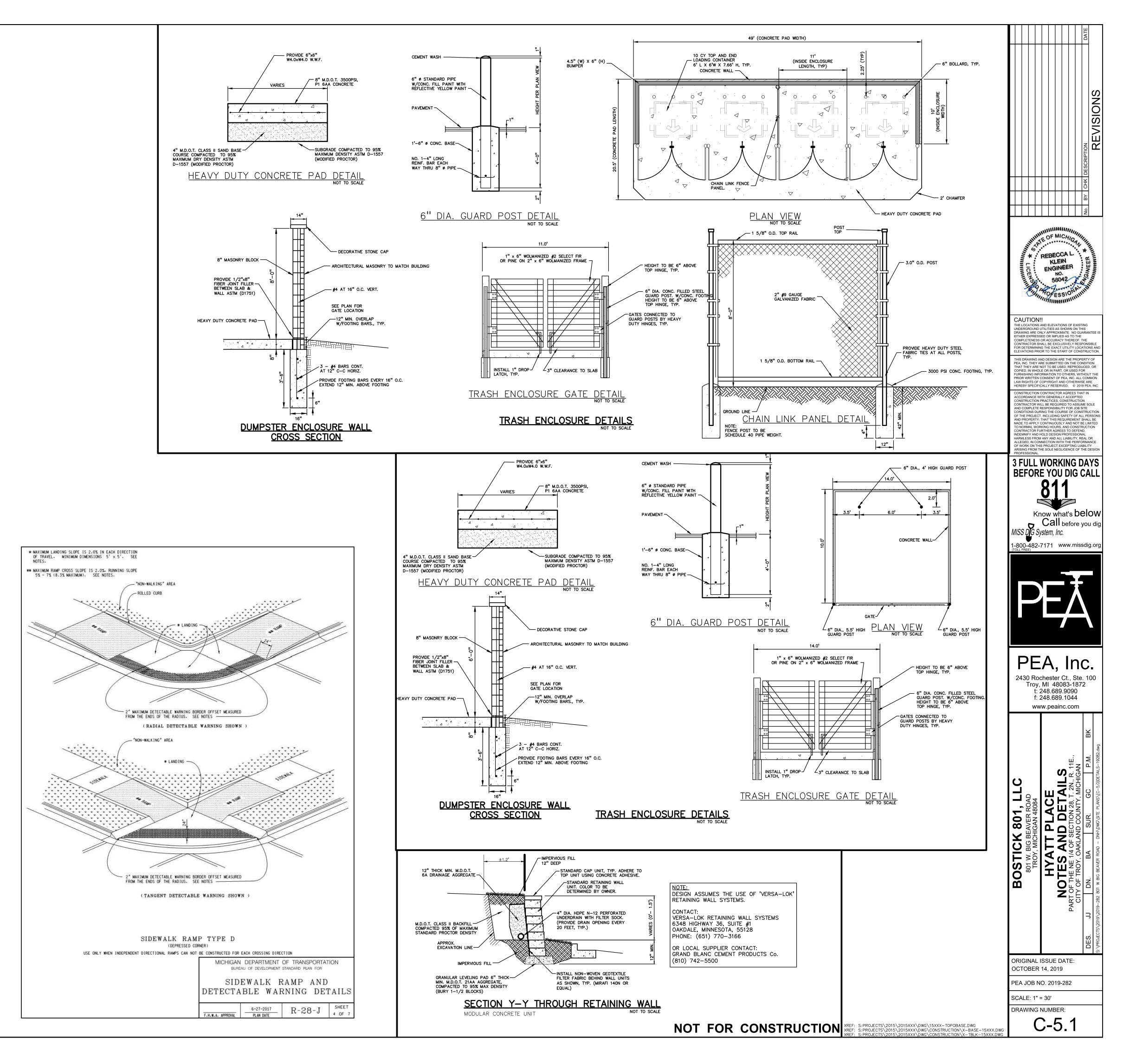


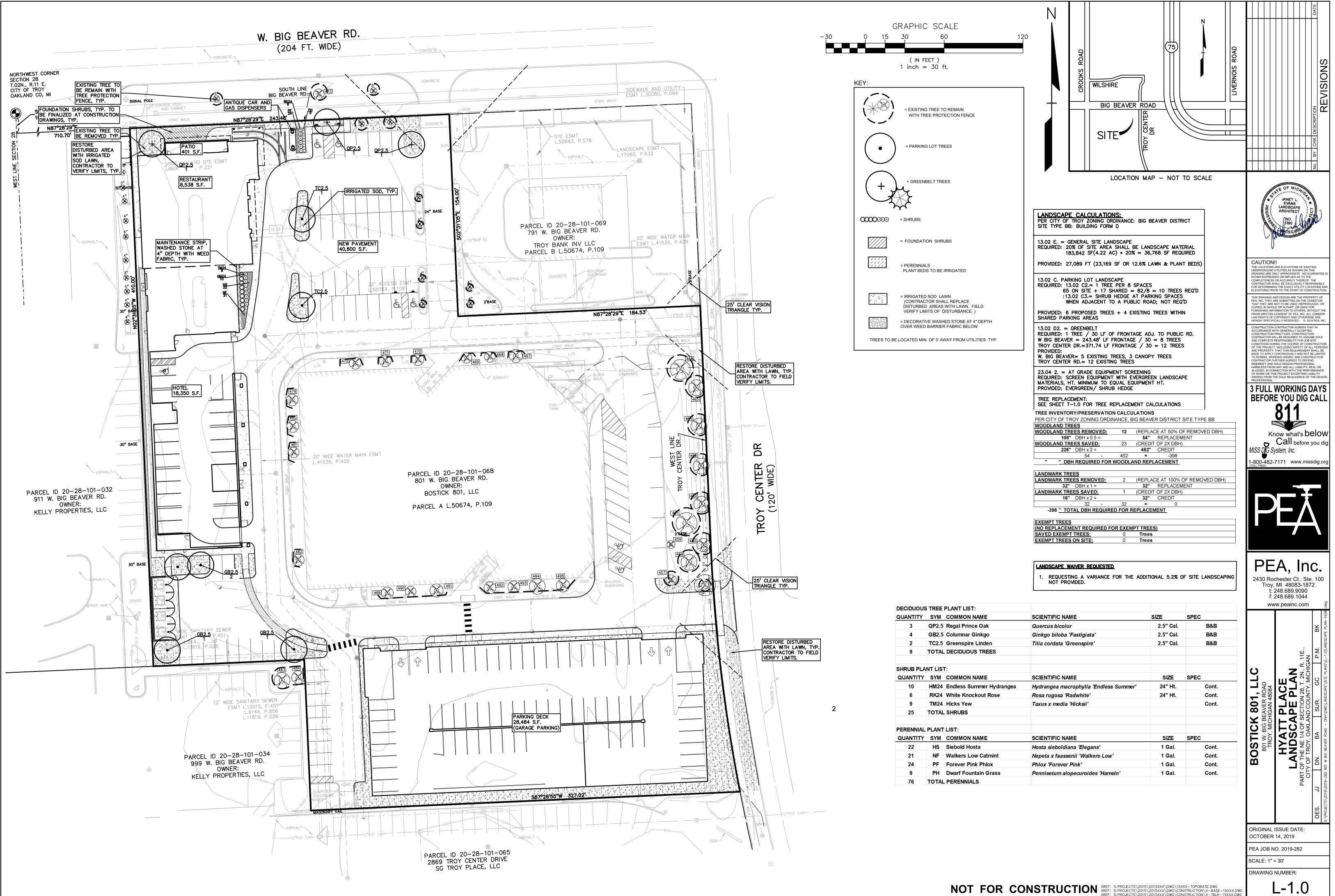


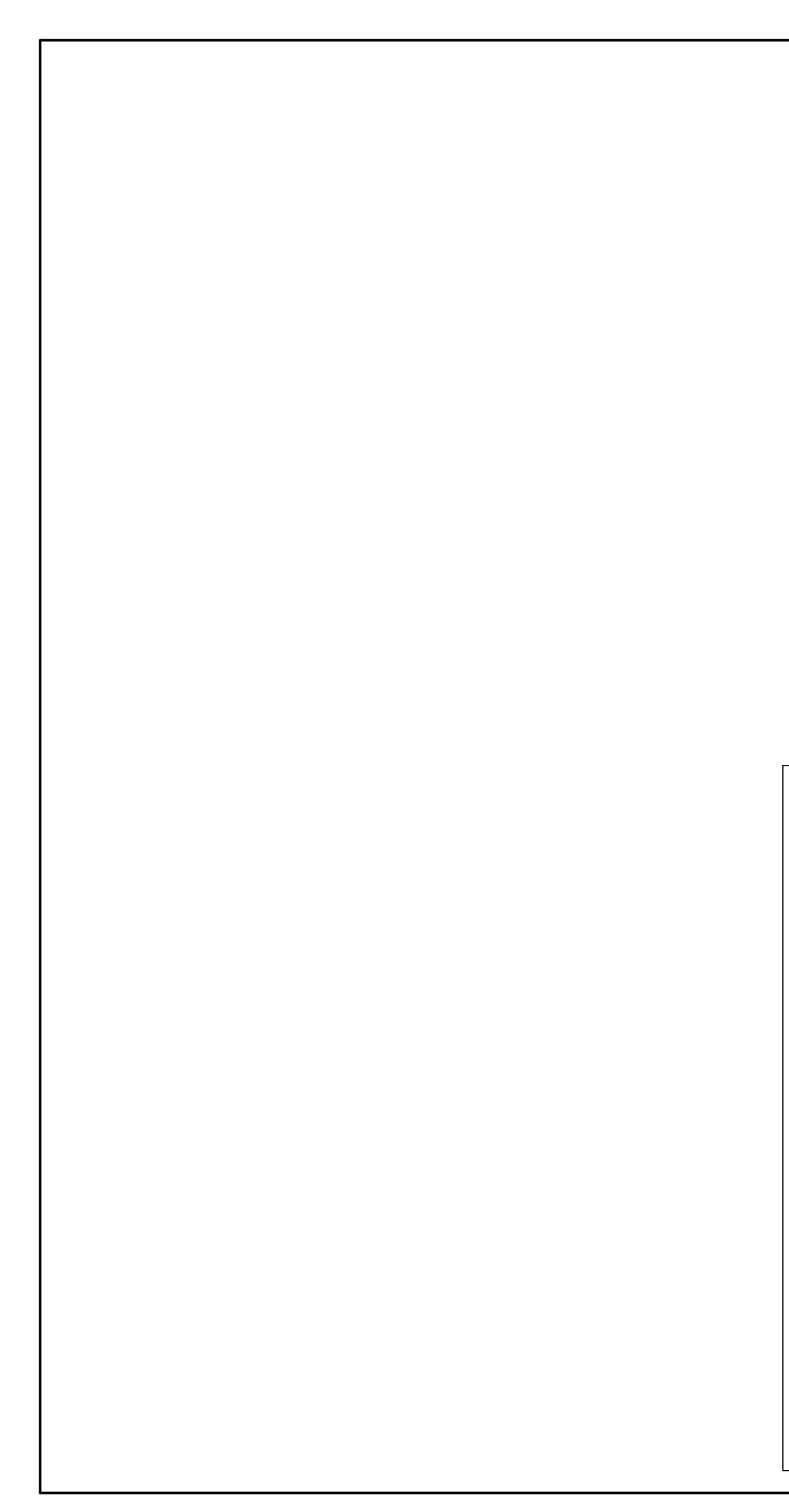




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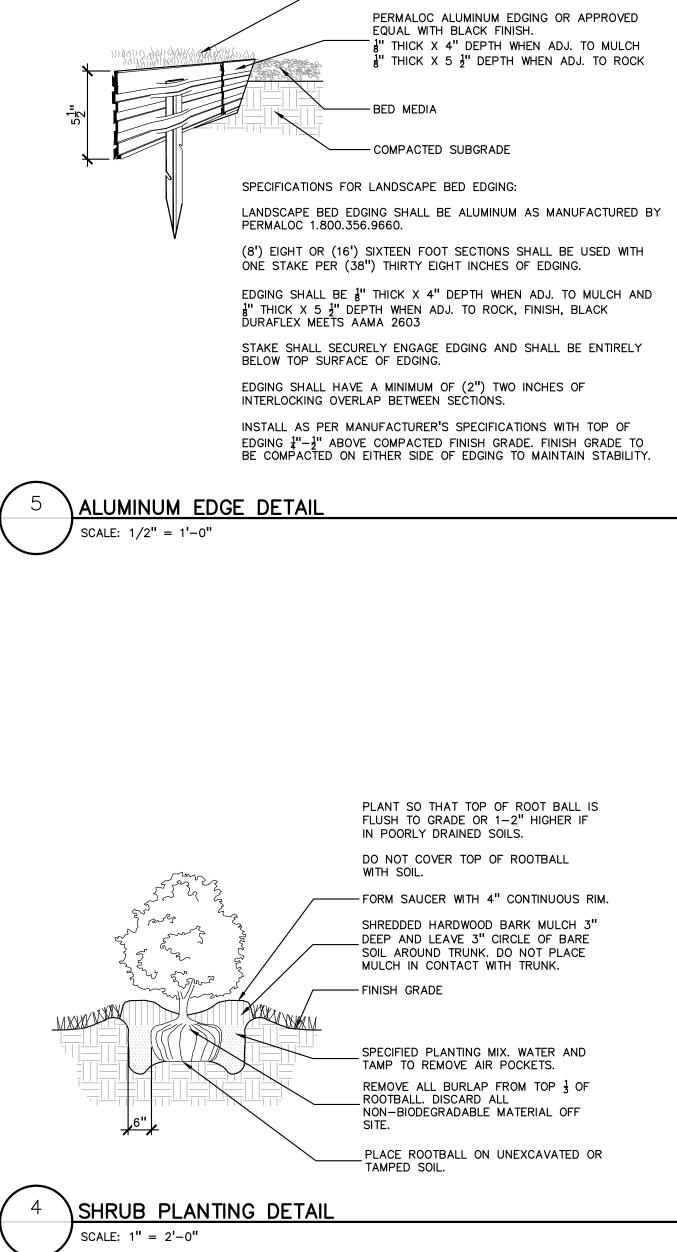


GENERAL LANDSCAPE NOTES: 1. LANDSCAPE CONTRACTOR SHALL VISIT SITE, INSPECT EXISTING SITE CONDITIONS AND REVIEW PROPOSED PLANTING AND RELATED WORK. IN CASE OF DISCREPANCY BETWEEN PLAN AND PLANT LIST, PLAN SHALL GOVERN QUANTITIES. CONTACT LANDSCAPE ARCHITECT WITH ANY CONCERNS. 2. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL ON SITE UTILITIES PRIOR TO BEGINNING CONSTRUCTION ON HIS/HER PHASE OF WORK. ELECTRIC, GAS, TELEPHONE,

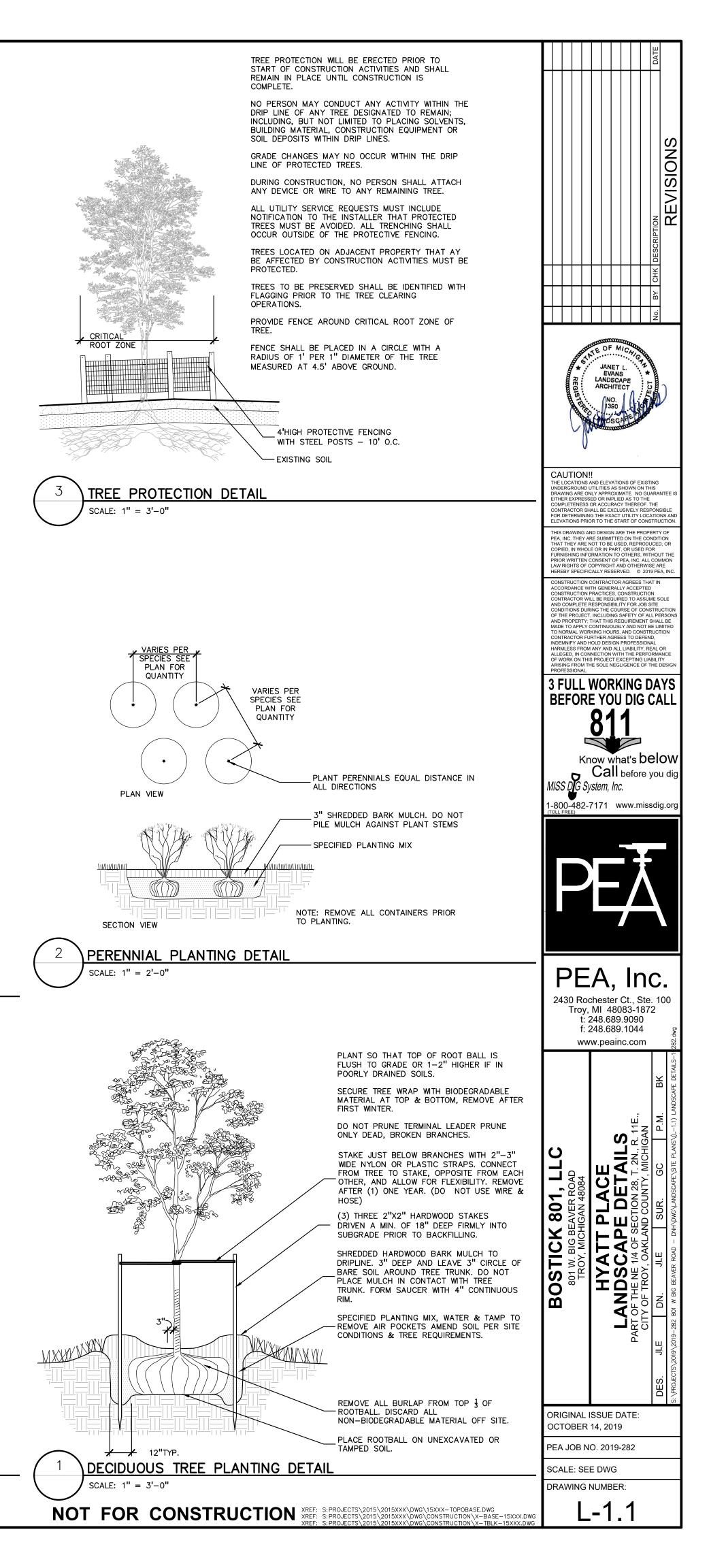
HIS/HER PHASE OF WORK. ELECTRIC, GAS, TELEPHONE, CABLE TELEVISION MAY BE LOCATED BY CALLING MISS DIG 1-800-482-7171. ANY DAMAGE OR INTERRUPTION OF SERVICES SHALL BE THE RESPONSIBILITY OF CONTRACTOR. CONTRACTOR SHALL COORDINATE ALL RELATED ACTIVITIES WITH OTHER TRADES ON THE JOB AND SHALL REPORT ANY UNACCEPTABLE JOB CONDITIONS TO OWNER'S REPRESENTATIVE PRIOR TO COMMENCING.

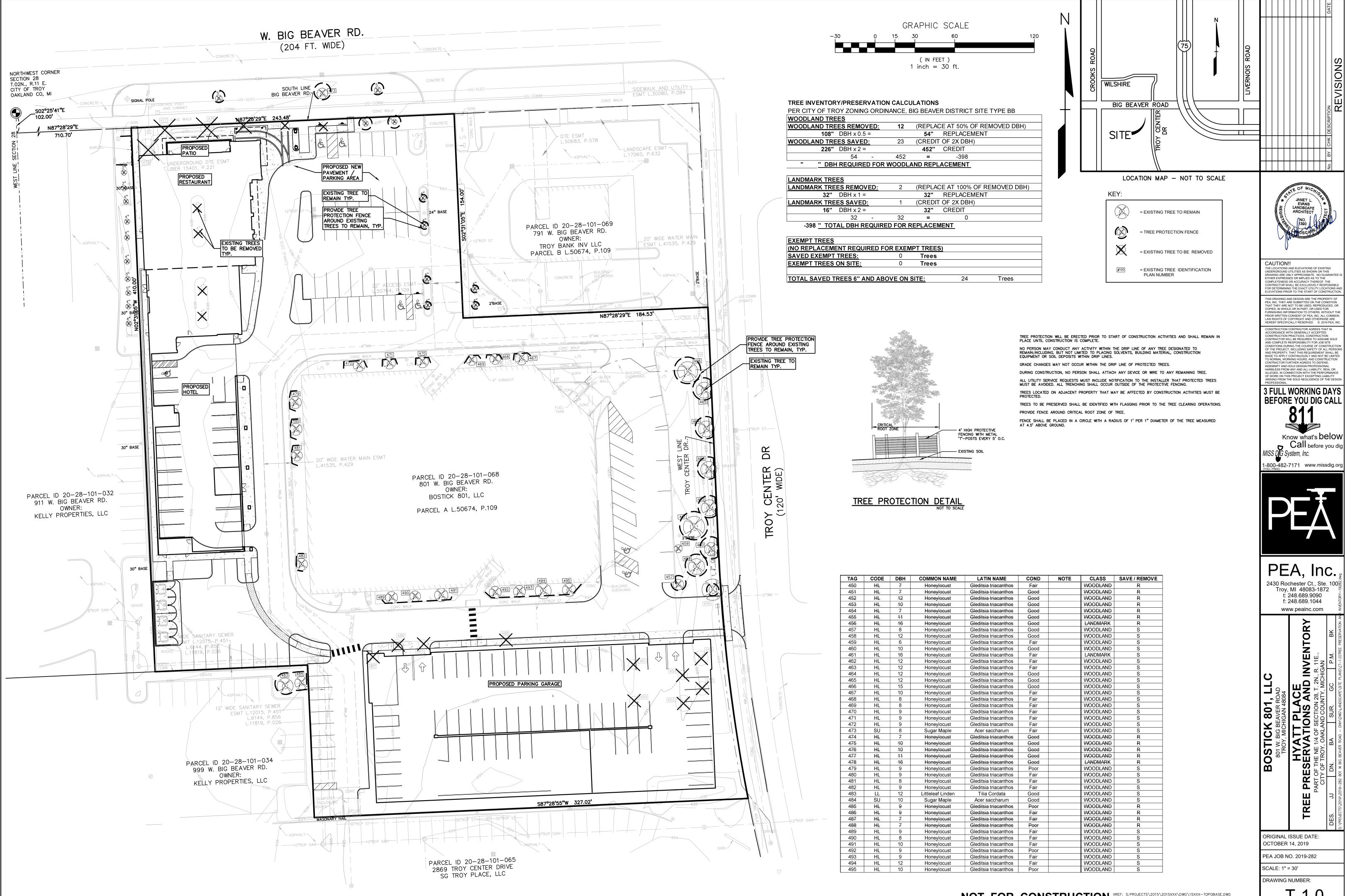
- 3. ALL PLANT MATERIAL TO BE PREMIUM GRADE NURSERY STOCK AND SHALL SATISFY AMERICAN ASSOCIATION OF NURSERYMEN STANDARD FOR NURSERY STOCK. ALL LANDSCAPE MATERIAL SHALL BE NORTHERN GROWN, NO. 1. GRADE.
- 4. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL QUANTITIES SHOWN ON LANDSCAPE PLAN PRIOR TO PRICING THE WORK.
- 5. THE OWNER'S REPRESENTATIVE RESERVES THE RIGHT TO REJECT ANY PLANT MATERIAL NOT MEETING SPECIFICATIONS.
- 6. ALL SINGLE STEM SHADE TREES TO HAVE STRAIGHT TRUNKS AND SYMMETRICAL CROWNS.
- 7. ALL SINGLE TRUNK SHADE TREES TO HAVE A CENTRAL LEADER; TREES WITH FORKED OR IRREGULAR TRUNKS WILL NOT BE ACCEPTED.
- 8. ALL MULTI STEM TREES SHALL BE HEAVILY BRANCHED AND HAVE SYMMETRICAL CROWNS. ONE SIDED TREES OR THOSE WITH THIN OR OPEN CROWNS SHALL NOT BE ACCEPTED.
- 9. ALL EVERGREEN TREES SHALL BE HEAVILY BRANCHED AND FULL TO THE GROUND, SYMMETRICAL IN SHAPE AND NOT SHEARED FOR THE LAST FIVE GROWING SEASONS.
- 10.ALL TREES TO HAVE CLAY OR CLAY LOAM BALLS, TREES WITH SAND BALLS WILL BE REJECTED.
- 11.NO MACHINERY IS TO BE USED WITHIN THE DRIP LINE OF EXISTING TREES; HAND GRADE ALL LAWN AREAS WITHIN THE DRIP LINE OF EXISTING TREES.
- 12.ALL TREE LOCATIONS SHALL BE STAKED BY LANDSCAPE CONTRACTOR AND ARE SUBJECT TO THE APPROVAL OF THE LANDSCAPE ARCHITECT PRIOR TO INSTALLATION OF THE PLANT MATERIAL.
- 13.IT IS MANDATORY THAT POSITIVE DRAINAGE IS PROVIDED AWAY FROM ALL BUILDINGS.
- 14.ALL PLANTING BEDS SHALL RECEIVE 3" SHREDDED HARDWOOD BARK MULCH WITH PRE EMERGENT, SEE SPECIFICATIONS. SHREDDED PALETTE AND DYED MULCH WILL NOT BE ACCEPTED.
- 15.ALL LANDSCAPED AREAS SHALL RECEIVE 3" COMPACTED TOPSOIL.
- 16.SEE SPECIFICATIONS FOR ADDITIONAL COMMENTS, REQUIREMENTS, PLANTING PROCEDURES AND WARRANTY STANDARDS.
- 17.FOR NON-LAWN SEED MIX AREAS, AS NOTED ON PLAN, BRUSH MOW ONCE SEASONALLY FOR INVASIVE SPECIES

CONTROL.



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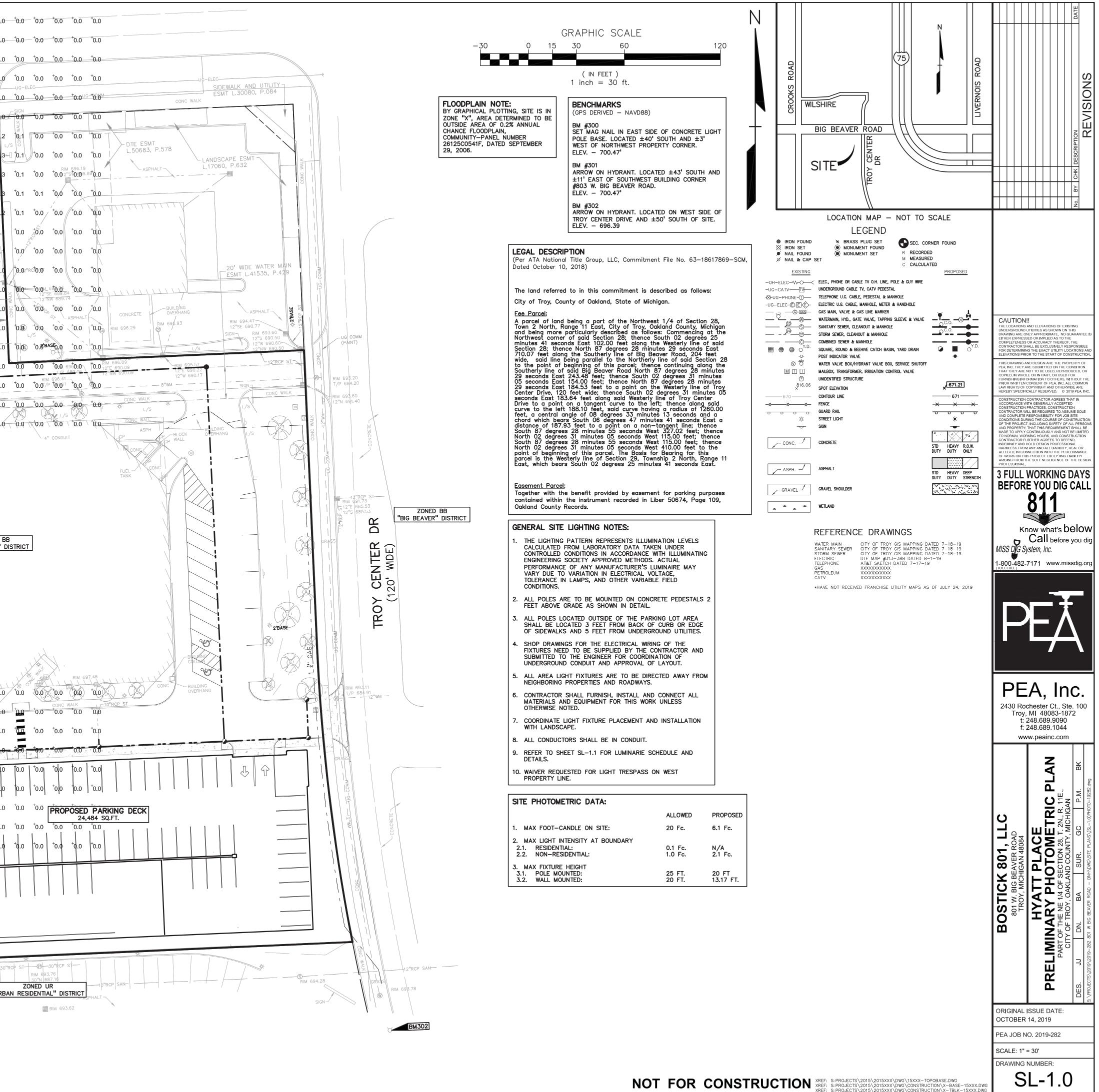


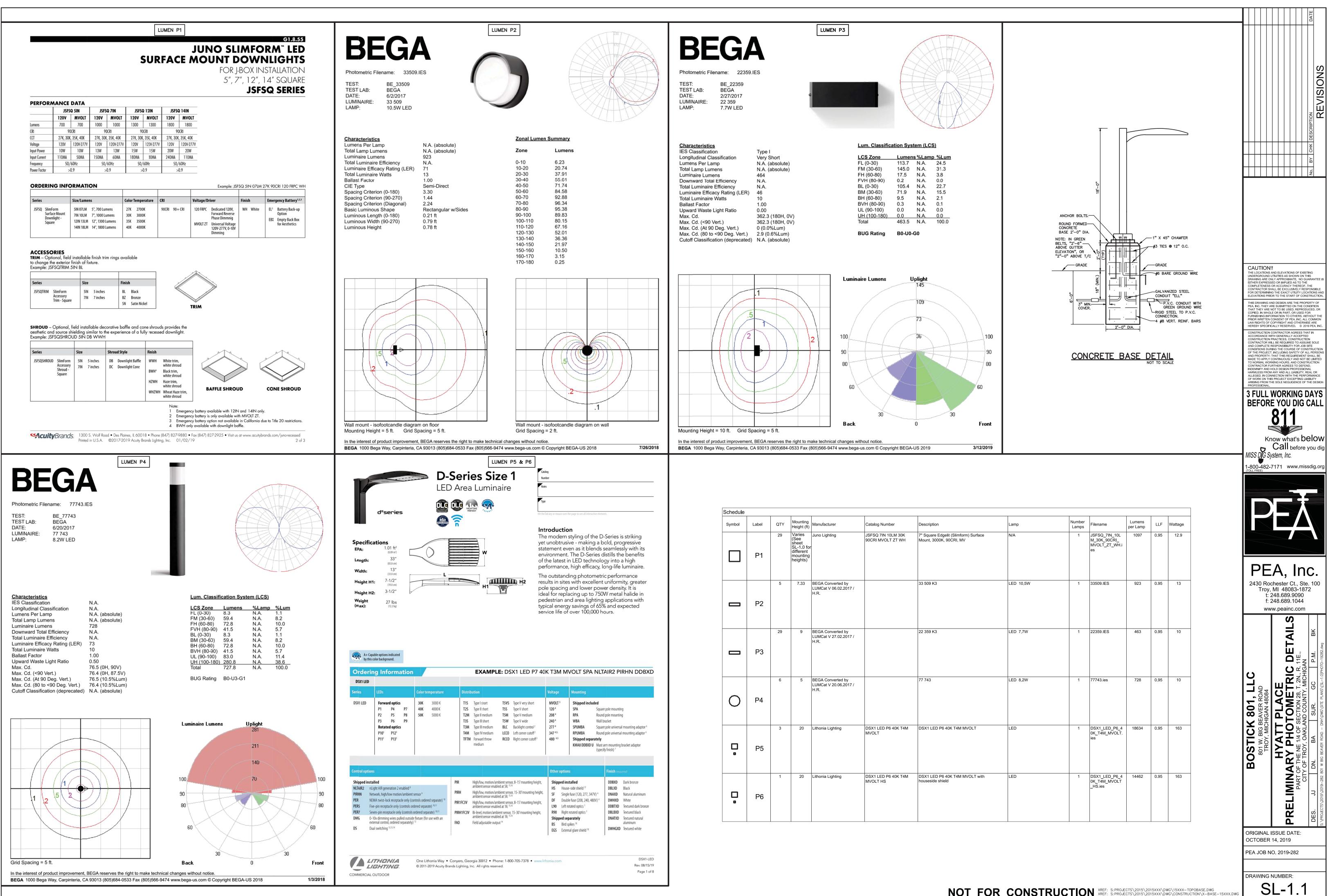


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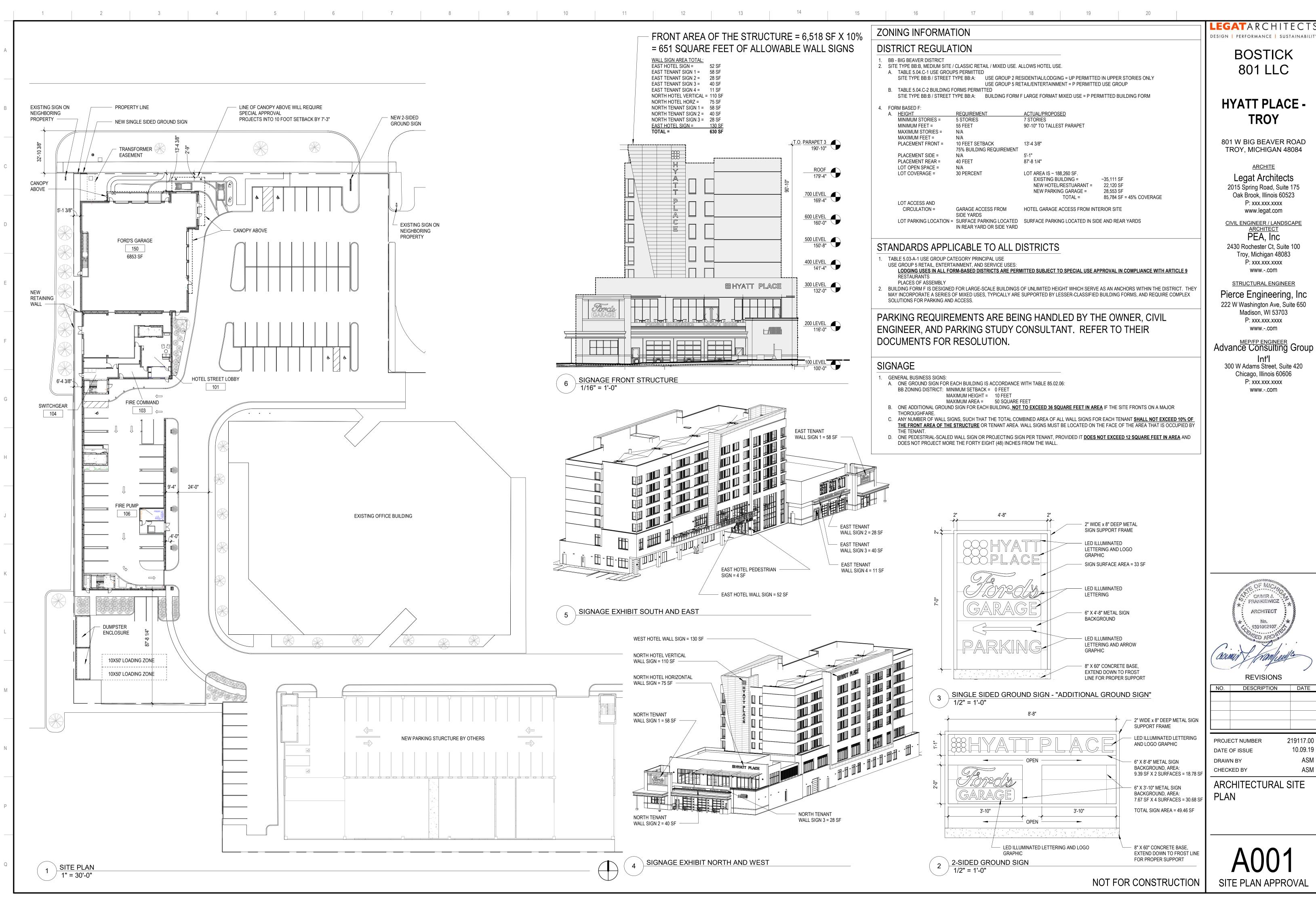
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	⁺ 0.0	⁺ 0.0	+0.0	⁺ 0.0	_{RUBS} 0.0	⁺ 0.0	[†] 0.2	+0.4				P1	P1 P1	54	⁺ 28		2.0	5-3.0	CA-3	⁺ 6.1	[‡] 5.5	+3.4	⁺ 2.1	⁺ 1.1	⁺ 0.6 sid	G⊼C.3−
	⁺ 0.0		1.0[23	P1 P1 UNDERGR LIBER 54	P1 OUND D 491, P.2	TELESN				+3.0	± <u>1.3</u>	2.5	+ - 3.5	^{4.5}	⁺ 4.9	⁺ 4.5	⁺ 3.4	⁺ 2.0	+1.1	⁺ 0.6	⁺ 0.3				
	+0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0	⁺ 0.1	1* 9		1700	MOUNT HEIGHT			P1			+1.8	⁺ 3.1	⁺ 3.9	⁺ 4.3	⁺ 3.9	⁺ 3.3	⁺ 2.6	⁺ 1.6	⁺ 0.9	⁺ 0.5	⁺ 0.3
	+0.0	⁺ 0.0	+0.0	⁺ 0.0	⁺ 0.0	0.1	ASE 2 1		1300	HEIG	NTING HT:		<u>ם</u> כ	⁺ 3.5	⁺ 1.7	+1.4	3.5	4.4	⁺ 4.0	⁺ 2.9	⁺ 1.9	+1.3	+ 0 .9	⁺ 0.5	⁺ 0.3	+0.2
	⁺ 0.0	+ GRA	SS 1.7	P3		13'-	·2"	1		+3.4	+ 1.6 ₁₂ "		and the second se	M+695.59 2"6 5 6 5 0.5	9 + 54 4.0	⁺ 2.5	+1.4	+0.8	+0.5	0.3 ☆ 24	"BASE	⁺ 0.				
	⁺ 0.0	⁺ 0.0	0.0	⁺ 0.0	⁺ 0.0	†0.0 ²	€ + +0.9	P3			JNTING		P1	⁺ 3.3	+ 1.6	P5 ⁺ 2.0		69 558 690.58	⁻⁺ 4.2	18 ⁺ 2.7	1.5	⁺ 0.8	+0.4	102	v+696.07 .''N 0: 1 689. .''SE 689.	.9 0
	+0.0	⁺ 0.0	+0.0	- ASP 19AL		0.0				HEI0 13'-	GHT: -2"	K		1 13.0	-+ 1-3	+1.5	/ 18"S 18"S	689.68 689.68 4.6	⁸ 4.0	⁺ 2.9	1.8	† - ↓ ⁺0.9			3"NW 689	.67
	⁺ 0.0	⁺ 0.0	°0.0	[PROPOSE	D BUIL	DING		<u></u> "C - " - ►	- 4	€ (P) (P)	+1.6	⁺ 3.7	+4.2	⁺ 3.6	⁺ 2.6	⁺ 1.7	+0.9	⁺ 0.5	⁺ 0.2	+0.1	+0.0				
	⁺ 0.0	⁺ 0.0	+0.0	⁺ 0.0	⁺ 0.0	0.0	° ⁺0.2					P4		⁺ 1.4	+0.8	+1	L.	⁺ 4.5	⁺ 3.7	⁺ 2.4⁄	RIM 65	95.80	+0.4	+0.2	+0.1	+0.0
	+0.0	⁺ 0.0		⁺ 0.0	⁺ 0.0	Q	9	P3				P4	• • • • • • • • • •	1.6) ⁺1.3	2.6	+5.1	+4.9	+3.3	÷2.1			S ESMT .10 03	<u> </u>		+ 0.0
	+0.0	+0.0		+0.0		+0.0 +0.0		РЗ				P4 P4	O O ₆	+1.4	/	V 1		+4.6		+1.8	+1.0			₽ ₽ ₽ 0.0		+0.0
	+0.0	⁺ 0.0	+0.0			30"	BASE	P 3				P4		+1.1	, , ⁺ 0.8		ן∘יי ר ₃₃ /		⁺ 2.6	+1.7	⁺ 1.0	+0.5	+0.3			+ 0.0
	0.0 ⁺	0.0 ⁺	0.0 ⁺		0.0 ⁺0.0		⁺ 0.3 ⁻ 0.3	РЗ	[)			o ^{+2.0}	. /	/		T/									+0.0
			DIM			Шь	0.		7		1	P1		+0.0				1			/M	+0.5				
	+0.0					[†] 0.0	+-			JNTING	א ר			⁺ 0.8	†03 +	⁺ 0.5	¥ -			52/*		+0.4	0.2 * + L/S 0.2	\mathcal{O}		^t 0.0
	+0.0	⁺ 0.0		ZONE	D BB		+0.2	P3		GHT: 12	-' //	P1 P	⁺ 3.5	Ĭ	+ 0.3 696.70	0.3			+0.8	0.6	+0.4	~ ~0.3	0.2	[*] 0.1	-70.1	+ <u>0.</u> 0
	⁺ 0.0	⁺ 0.0			<u>R" DIS</u>		⁺ 0.2					4	412 P2	[™] 1.4		0.2	\backslash									
	+0.0			+0.0			⁺ 0.2		23			P1		1.3	⁺ 0.4	0 2	\backslash							\		
	⁺ 0.0	⁺ 0.0	⁺ 0.1	1.9	3		\Box	P1	-3.1 P2	1.2	[†] 0.4	⁺ 0.2	RIM 697	7.91												
	⁺ 0.0		⁺ 0.1	[‡] 1.6— ₽		INTING GHT: 12	K	P1	4.3	1.5	0.4	⁺ 0.2	RIM 697	7.95												
	⁺ 0.0	+0 30"	BABET	[‡] 2:3	23				⁺ 3.9	+1.3	+0.4	+0.2		o' wide	WATE	R MAI	N ESM	Т								
	⁺ 0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0			⁺ 0.1	1.8				P1	⁺ 3.9	- 1	+0.4	0.2	L.	41535,	P.429							ED BB
																11								"BIG	DLAVL	
	⁺ 0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0	- ASPHAL 0.0	+ 0.0	+0.1						3.9	1.6	0.5 XIV	0.2								"BIG		
	⁺ 0.0 +0.0	+0.0 +0.0	+0.0 +0.0				+0.1 +0.1	2.0	P3			P1	3.9 □ 1 +4.0	1.6 ⁺ 1.5	W									"BIG		
			⁺ 0.0	⁺ 0.0				20	P3	DUNTING		P1 P1	3.9 7 4.0 ₽ 4.3	1.6 ⁺ 1.5	7/1/									"BIG	BLAVE	
	⁺ 0.0	+0.0 +0.0	+0.1	2.0	P3 P3 P3 HE	DUNTING IGHT: 1		P1 P1	3.9 7 4.0 1 4.3 0	1.6 +1.5	+0.5	0.2								"BIG						
	+0.0 +0.0	+0.0 +0.0	+0.0 +0.0	+0.0 +0.0 +0.0	+0.0 +0.0	+0.0 +0.0 +0.1	+0.1 +0.1	2.0	P3 P3 P3 P3 HE P3 P3		2'	P1 P1 P1	3.9 4.0 4.3 3.9	⁺ 1.5 ⁺ 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.8	+0.5									"BIG		
	+0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0	+0.0 +0.0 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 	P3 P3 P3 P3 P3 P3	IGHT: 1	2'	P1 P1 P1 P1 P1	3.9 4.0 4.3 3.9	⁺ 1.5 ⁺ 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.8	+0.5 +0.5		0.1							"BIG		
	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1	2.0 1.7 	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1	2'	P1 P1 P1 P1 P1	3.9 4.0 4.3 3.4 3.4 1 D P	⁺ 1.5 2.3 ⁺ 1.5 2.3 ⁺ 1.5 ⁺ 1.8 2.1			21	LIGHT	POLE,					"BIG		
	+0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.1 +0.0 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 .6 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P		2' +4.1 -6.0	P1 P1 P1 P1 -3.3	3.9 4.0 4.3 3.4 3.4 1.3 2.4	⁺ 1.5 ⁺ 2.3 ⁺ 1.5 ⁺ 1.8 ⁺ 1.8 ⁺ 1.5 ⁺ 1.5		+0.2 +0.2 +0.2	21			TYP.				"BIG		*0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.1 +0.0 +0.0 +0.0 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 .6 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	1GHT: 1 P3 ↓ +4.4 P5 ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓	2 P3 F 4.1 6.0 +5.0	P1 P1 P1 P1 P1 -3.3 -3.6	3.9 4.0 4.3 3.4 3.4 1.3 2.4	⁺ 1.5 ⁺ 2.3 ⁺ 1.5 ⁺ 1.8 ⁺ 1.8 ⁺ 1.5 ⁺ 1.5		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5	⁺ 0.3	<u> </u>			+0.0	*0:010	*0.0	*0.0	6.0	<u>.</u>
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 PHALT - +0.1	+0.0 +0.0 +0.1 +0.0 +0.0 +0.1 +0.3 +0.3	⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.1 ⁺ 0.2 ⁺ 0.2 ⁺ 0.6	2.0 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1	2 P3 F +4.1 -6.0 +5.0 +3.2	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 3.4 3.4 4.3 3.4 4.3 4.3 4.3 4.3	⁺ 1.5 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.8 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.5 +0.6	+0.3 +0.3 +0.3 *0.3 *0.3 *0.3 *0.3 *0.3 *0.3 *0.3	+0.1 0.1		TYP.		*0.0		+0.0	+0.0	<u>.</u>
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 PHALT +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.1 +0.0 +0.0 +0.1 +0.3 +0.3	+0.1 +0.1 +0.1 +0.1 +0.1 = *0.1 = *0.1 = *0.1 = *0.1 = *0.1	2.0 1.7 .6 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1 P3 P3 +4.4 P5 +4.3 +3.2 +2.0	2 P3 F 4.1 +6.0 +5.0 +3.2 +1.9	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1	3.9 +4.0 +4.0 -4.3 -3.9 -3.4 -1.3 +2.7 +2.7 +2.2	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.8 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 (% 0.3 5 0'N 698 40'F 698	+ 0.1 0.1 0.1 0.1 0.1 0.1	0.1 CC LS CC	TYP. 397.71 0.0	⁺ 0.0	⁺ 0.0	⁺ 0.0	+0.0	+0.0 +0.0	+0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 PHALT +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.1 +0.0 +0.0 +0.0 +0.1 +0.3 +0.3 +0.3 +0.2 12	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1 P3 +4.4 +4.4 +4.4 +4.3 +3.2 +2.0 SANITA L.12011 61.44, P5 12012	2 P3 F 4.1 f 6.0 f 3.2 f 1.9 RY SEV 5, 1-045 .85B	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1	3.9 4.0 4.3 3.9 3.4 1.3 +2.7 +2.7 +2.2 +1.6	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.8 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 1.5		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	*0.1 0.0 0.0 0.1 0.0 0.1	0.1 CC LS CC	TYP. 397.71 0.0	+0.0	⁺ 0.0	+0.0 +0.0	+0.0 +0.0	+0.0 +0.0	+0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 CP \$A1 CP \$	+0.0 +0.0 +0.1 +0.0 +0.0 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.2 12 17 10.2 17 10.2 11 17 10.2 11 17 10.2 11 10 10 10 10 10 10 10 10 10 10 10 10	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1 P3 +4.4 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 118495 F	2 P3 F3 F4.1 F6.0 F5.0 F3.2 F1.9 RY SEV 5, 1-045 85/8 P3 04 F3.2	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.3 3.4 4.3 3.4 4.3 4.3 4.3 4.3	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	*0.1 0.0 0.0 0.1 0.0 0.1	+0.1 00 +0.1 00 +0.0 00 +0.000 +0.000 +0.000 +0.00000 +0.00000 +0.0000 +0.0000 +0.00000 +0.00000 +0.00000 +0.0000 +0.0	TYP. 997.71 0.0 +0.0 +0.0 0.0	+0.0	+0.0 +0.0	+0.0 +0.0	+0.0 +0.0 +0.0	+0.0 +0.0 	+0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 -0.1 -0.1 -0.1 -0.1 -0.1 -0.1 -0.1 -	+0.0 +0.0 +0.1 +0.0 +0.0 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.2 12 17 10.2 17 10.2 11 17 10.2 11 17 10.2 11 10 10 10 10 10 10 10 10 10 10 10 10	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P3 P	IGHT: 1 P3 +4.4 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 118495 F	2 P3 F 4.1 f 6.0 f 3.2 f 1.9 RY SEV 5, 1-045 .85B	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 +4.0 +4.0 -4.0 -4.0 -4.0 -3.4 -4.0 -	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 1.5		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.6 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 0°% 6392 0°% 688.68 0°% 689.68 0°% 689.68 000000000000000000000000000000000000	+0.1 0 000 0 000 0 0.1 0 0.1 +0.1 +0.1 +0.1	+0.1 +0.1 +0.1 +0.1 +0.4	TYP. 997.71 0.0 +0.0 +0.0 0.0	+0.0 +0.0 0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +-0.0	+0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.2 12 -17 17 142 99.50.2 0.65 -8.85 -8.80.1	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	2.0 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 P3 +4.4 +4.4 +4.3 +3.2 +2.0 SANITA L.12011 6144, P 1180, F 6144, P 1180, F 1180, F 1080, F 10	2 P3 +4.1 +4.1 +4.1 +5.0 +3.2 +1.9 RY SEV -0.4 -0.4 -0.2 -0.1	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 +4.0 +4.0 -4.3 -	¹ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 1.		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.4 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 0 000 0 000 0 0.1 0 0.1 +0.1 +0.1 +0.1	+0.1 +0.1 +0.1 +0.1 +0.1	TYP. 397.71 00 00 00 00 00 00 00 00 00 0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 -4.4 -4.3 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F -0.2 GRAS +0.1 -0.0	2 P3 +4.1 +4.1 +4.1 +5.0 +3.2 +1.9 RY SEV -0.4 -0.4 -0.2 -0.1	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 4.3 3.4 4.3 3.4 4.3 3.4 4.3 3.4 4.3 4.3	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 0.7 ⁺ 0.7 ⁺ 0.5 ⁺ 0.		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.4 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 0°N 698 0°N 698 0 0°N 698 0°N 698 0 0°N 698 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1	TYP. 397.71 00 00 00 00 00 00 00 00 00 0	+0.0 +0.0 0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 -4.4 -4.3 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F -0.2 GRAS +0.1 -0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 397.71 40.0 40.0 101 0.0 101 0.0 100 0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	⁺ 1.6 ⁺ 1.5 ² 2.3 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.5 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.7 ⁺ 1.5 ⁺ 0.7 ⁺ 0.7 ⁺ 0.5 ⁺ 0.		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 397.71 40.0 40.0 101 0.0 101 0.0 100 0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 397.71 40.0 40.0 101 0.0 101 0.0 100 0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 397.71 40.0 40.0 101 0.0 101 0.0 100 0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3		+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 397.71 0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3		+0.1 C +0.1 C	TYP. 397.71 0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.5 +0.5 +0.6 +0.4 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3		+0.1 CC + 0.1 CC + 0.	TYP. 397.71 0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3	+0.1 +	+0.1 CC + 0.1 CC + 0.	TYP. 97.71 0.0 10.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.5 +0.5 +0.5 +0.2 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.1 +	+0.1 CC + 0.1 CC + 0.	TYP. 97.71 0.0 10.0	⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0 ⁺ 0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.5 +0.5 +0.5 +0.2 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.1 +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 097.71 00 00 00 00 00 00 00 00 00 0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.5 +0.5 +0.5 +0.2 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.1 +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 97.71 0.0 10.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0
	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *0.1 *	+0.0 +0.0 +0.1 +0.0 +0.1 +0.0 +0.1 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.2 0.65 8.85 8.80_1 +0.1 +0.0	+0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.6 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.2 +0.2 +0.0	2.0 1.7 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7	P3 P3 P3 P3 P3 P3 P3 P3 P3 P3	IGHT: 1 P3 +4.4 P5 +4.4 +4.3 +3.2 +2.0 SANITA L.12013 6144, P 11849.F 0.5 +0.1 +0.1 +0.0	2 P3 +4.1 +4.1 +6.0 +3.2 +1.9 RY SEV 5, 1-045 .85B -04 -04 -04 -04 -04 -04 -04 -04	P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P1 P	3.9 4.0 4.0 3.4 4.3 3.4 4.3 3.4 4.3 4.3 4.3	1.6 +1.5 2.3 +1.5 2.3 +1.5 +1.5 +1.5 +1.7 +1.7 +1.7 +1.7 +1.7 +1.7 +0.4 +0.2 +0.4 +0.2 +0.1 +0.2 +0.1 +0.2 +0.1 +0.5 +0.7		+0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2 +0.2	+0.3 +0.3 +0.3 +0.3 +0.3 +0.3 +0.5 +0.5 +0.5 +0.5 +0.2 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1 +0.1	+0.1 +	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	TYP. 097.71 00 00 00 00 00 00 00 00 00 0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0	*0.0 *0.0 *0.0 *0.0 *0.0 *0.0	+0.0 +0.0 +0.0 +0.0 +0.0 +0.0 +0.0





NOT FOR CONSTRUCTION XREF: S: PROJECTS\2015\2015XXX\DWG\15XXX-TOPOBASE.DWG XREF: S: PROJECTS\2015\2015XXX\DWG\CONSTRUCTION\X-BASE-15XXX.DWG XREF: S: PROJECTS\2015\2015XXX\DWG\CONSTRUCTION\X-TBLK-15XXX.DWG



	REQUIREMENT	ACTUAL/PROPOSED	
ORIES =	5 STORIES	7 STORIES	
ET =	55 FEET	90'-10" TO TALLEST PARAPET	
TORIES =	N/A		
EET =	N/A		
FRONT =	10 FEET SETBACK	13'-4 3/8"	
	75% BUILDING REQUIREMENT		
SIDE =	N/A	5'-1"	
REAR =	40 FEET	87'-8 1/4"	
PACE =	N/A		
AGE =	30 PERCENT	LOT AREA IS ~ 188,260 SF.	
		EXISTING BUILDING =	~35,111 SF
		NEW HOTEL/RESTUARANT =	22,120 SF
		NEW PARKING GARAGE =	28,553 SF
		TOTAL =	85,784 SF = 45% COVERAGE
S AND			
ION =	GARAGE ACCESS FROM	HOTEL GARAGE ACCESS FROM INT	ERIOR SITE

801 W BIG BEAVER ROAD TROY, MICHIGAN 48084 <u>ARCHITE</u> Legat Architects 2015 Spring Road, Suite 175 Oak Brook, Illinois 60523 P: xxx.xxx.xxxx www.legat.com CIVIL ENGINEER / LANDSCAPE PEA, Inc 2430 Rochester Ct, Suite 100 Troy, Michigan 48083 P: xxx.xxx.xxxx www.-.com STRUCTURAL ENGINEER

BOSTICK

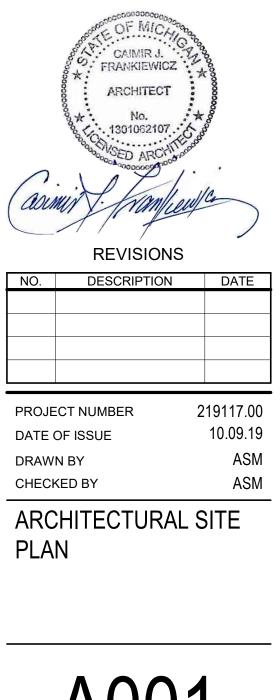
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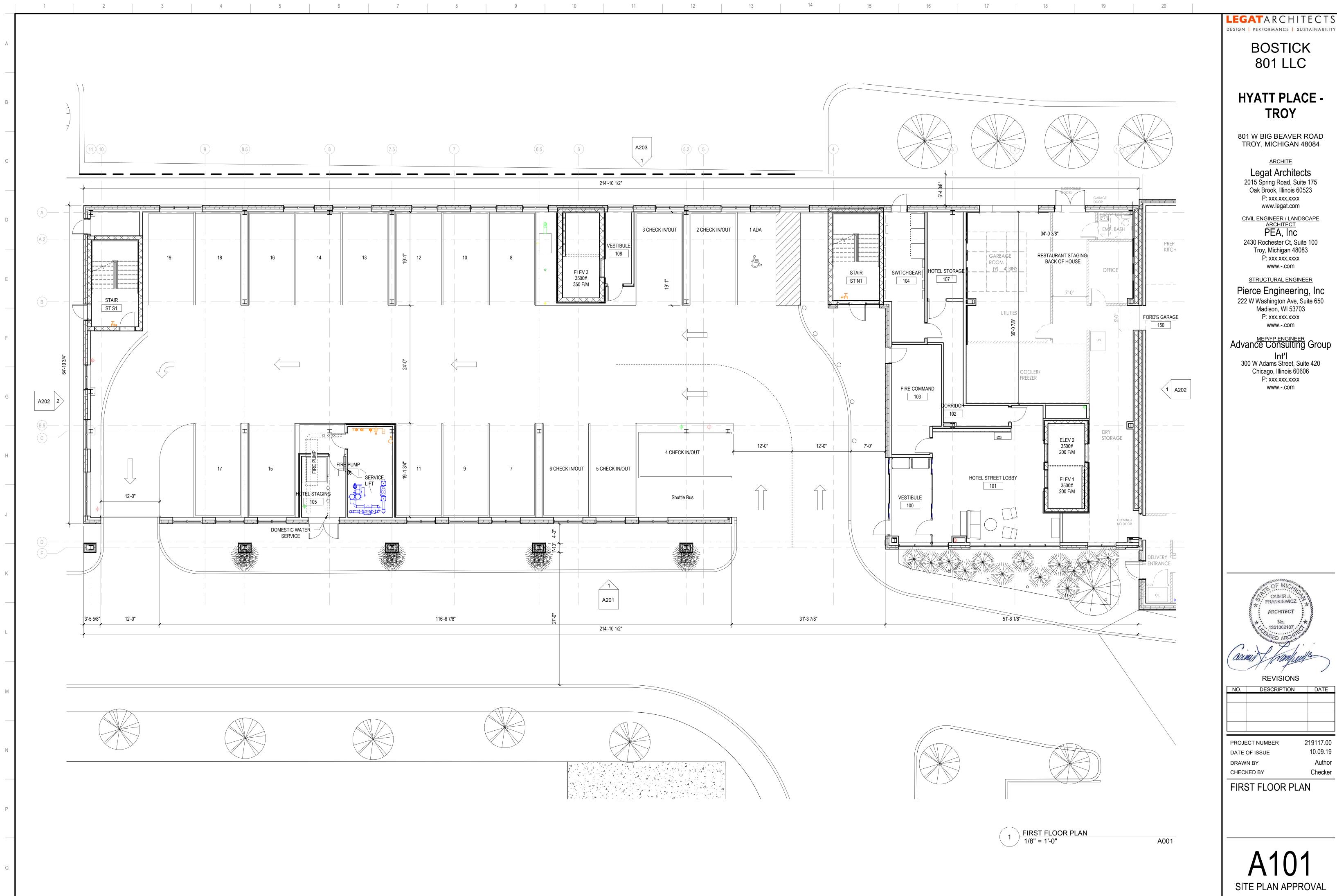
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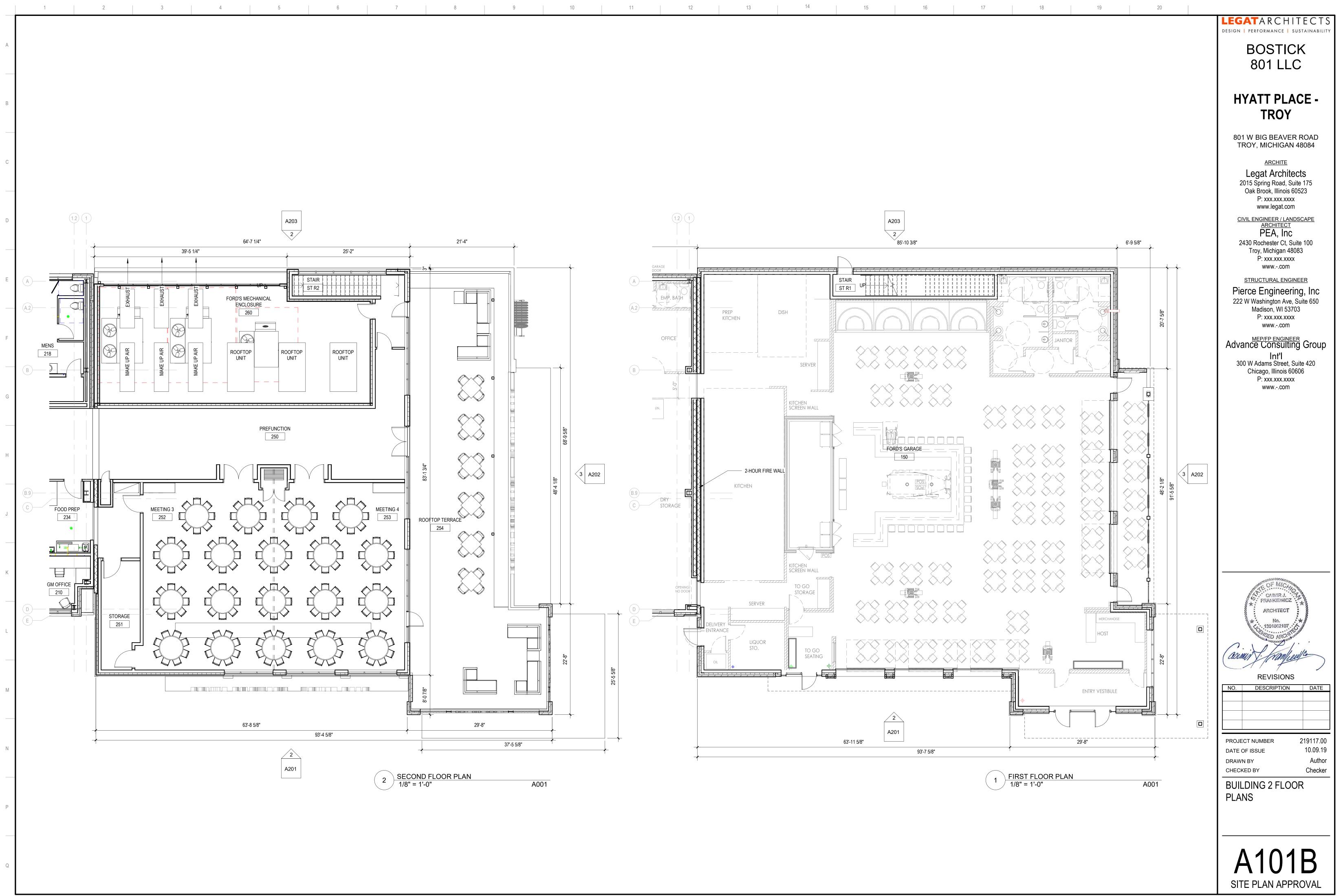


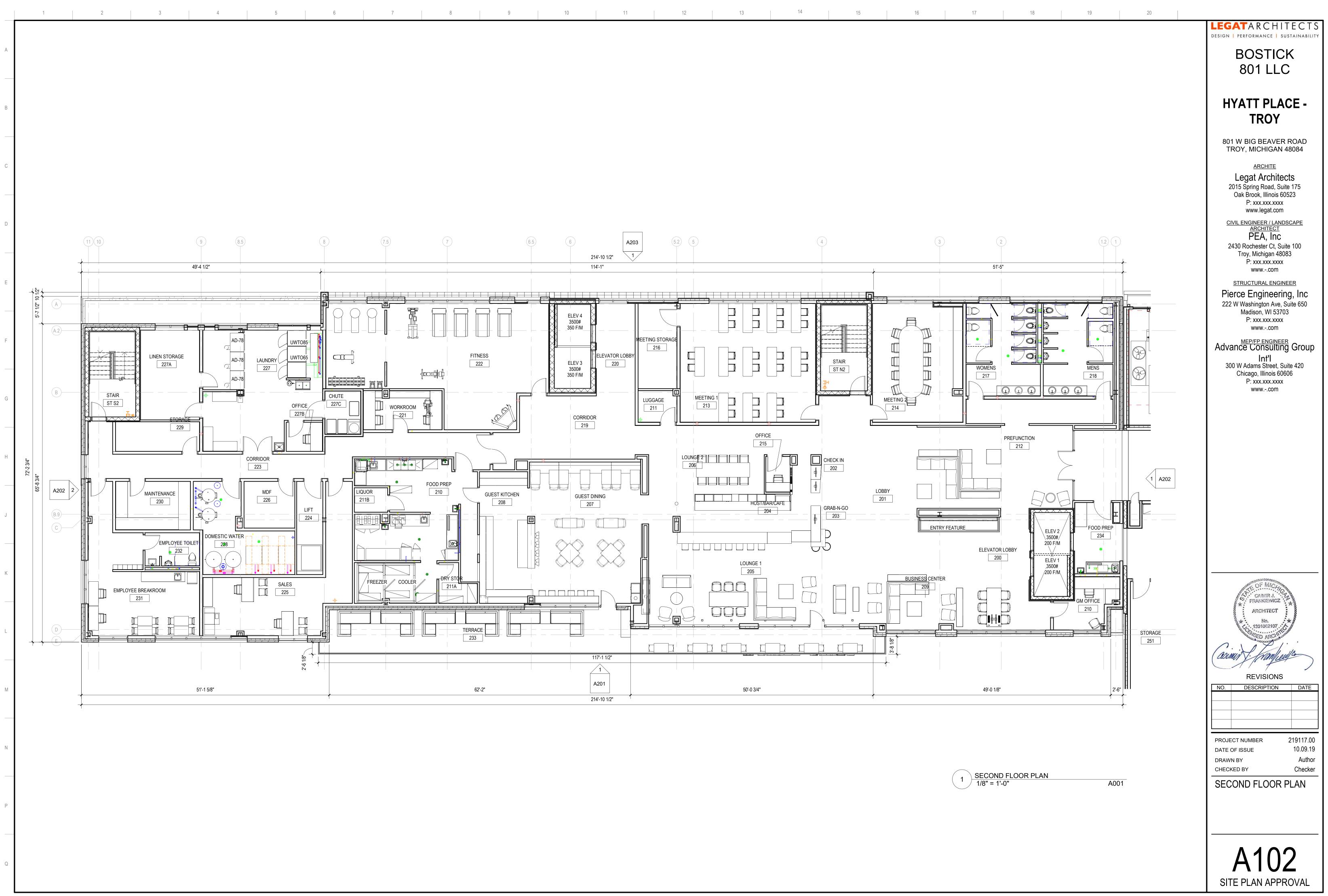


801 LLC HYATT PLACE -TROY 801 W BIG BEAVER ROAD TROY, MICHIGAN 48084 <u>ARCHITE</u> Legat Architects 2015 Spring Road, Suite 175 Oak Brook, Illinois 60523 P: xxx.xxx.xxxx www.legat.com civil engineer / landscape <u>architect</u> PEA, Inc 2430 Rochester Ct, Suite 100 Troy, Michigan 48083 P: xxx.xxx.xxxx www.-.com STRUCTURAL ENGINEER Pierce Engineering, Inc 222 W Washington Ave, Suite 650 Madison, WI 53703 P: xxx.xxx.xxxx www.-.com Advance Consulting Group Int'I 300 W Adams Street, Suite 420 Chicago, Illinois 60606 P: xxx.xxx.xxxx www.-.com CAIMIR J. ARCHITECT No. . 1301062107 . (admir V. fram REVISIONS

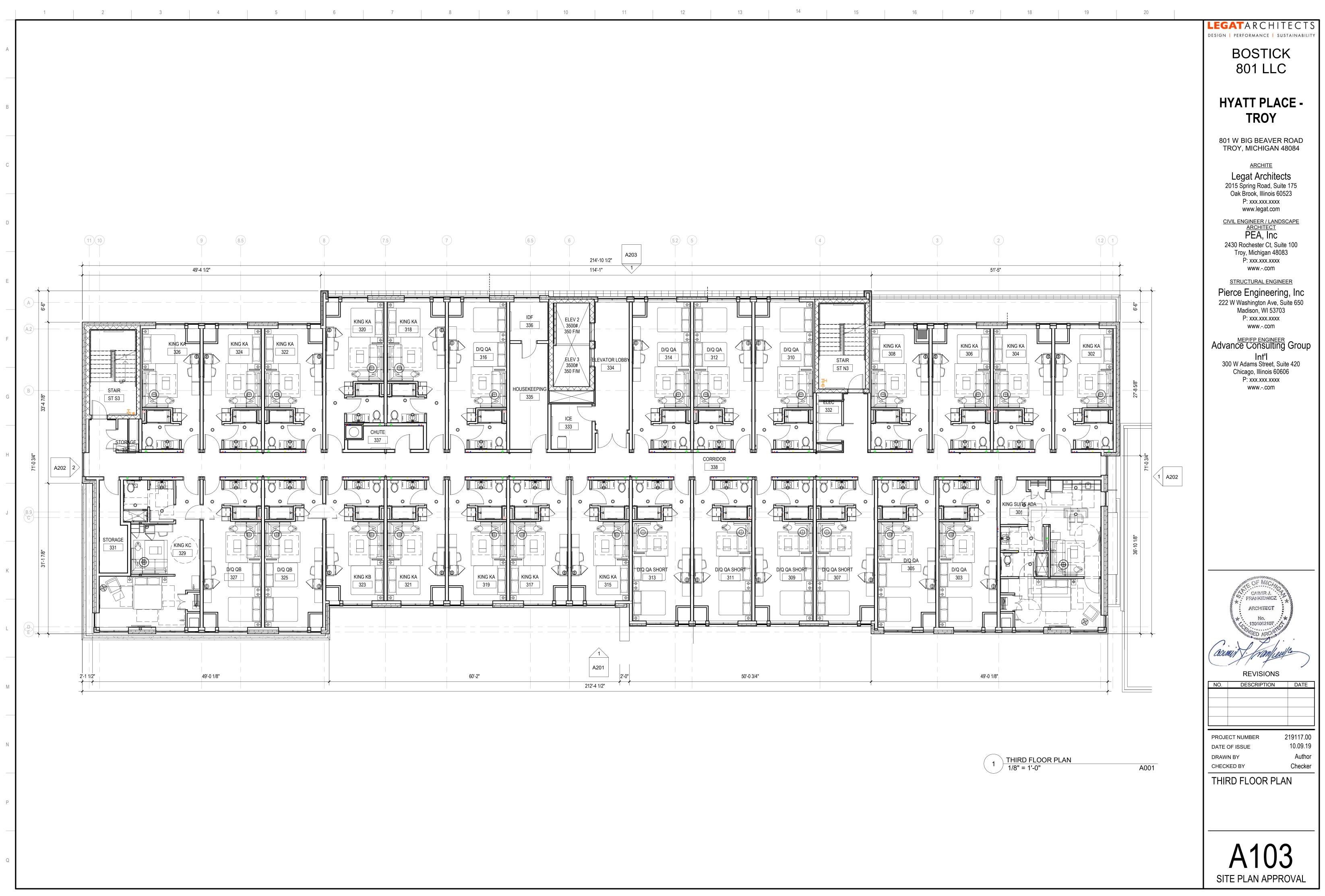
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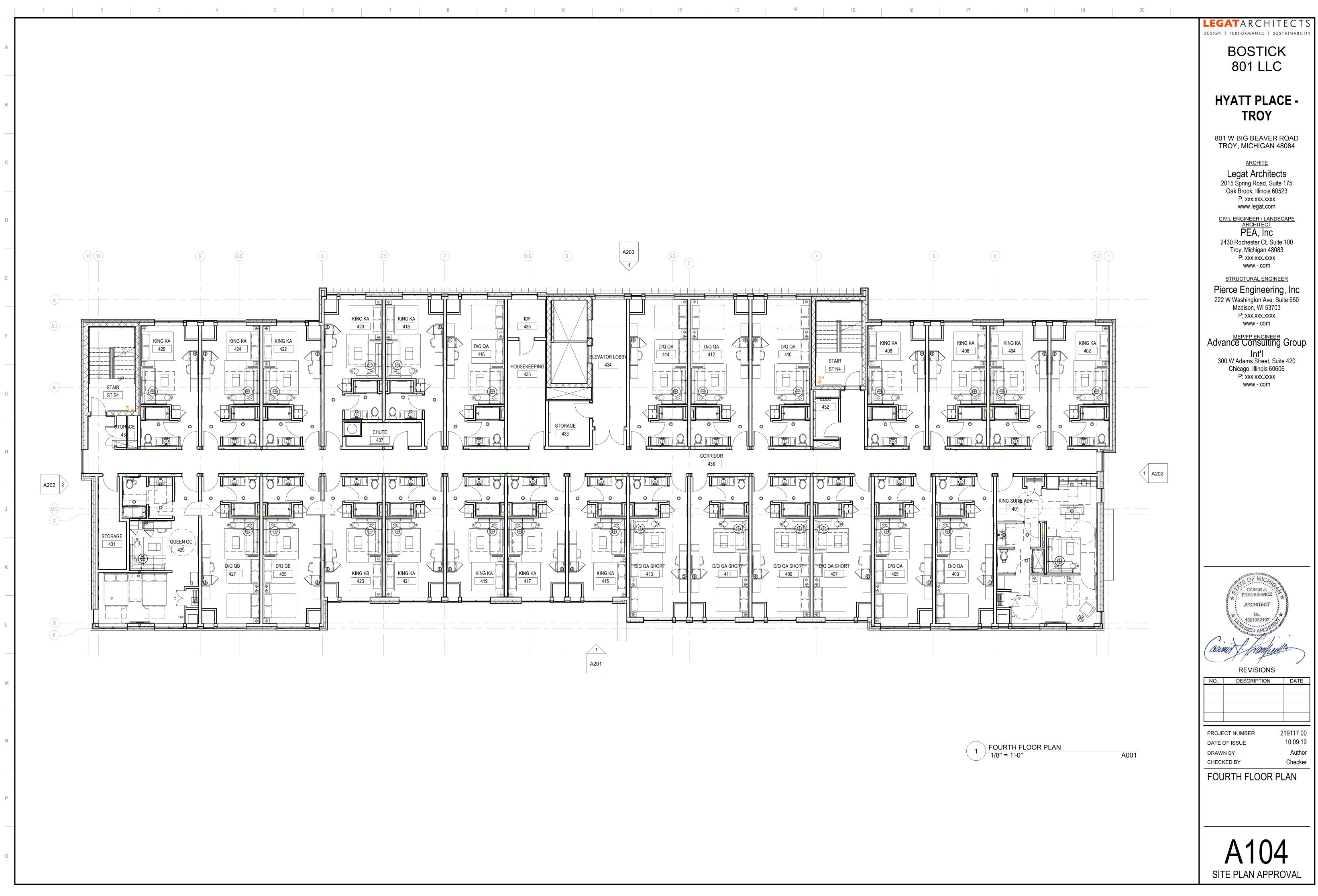
SITE PLAN APPROVAL

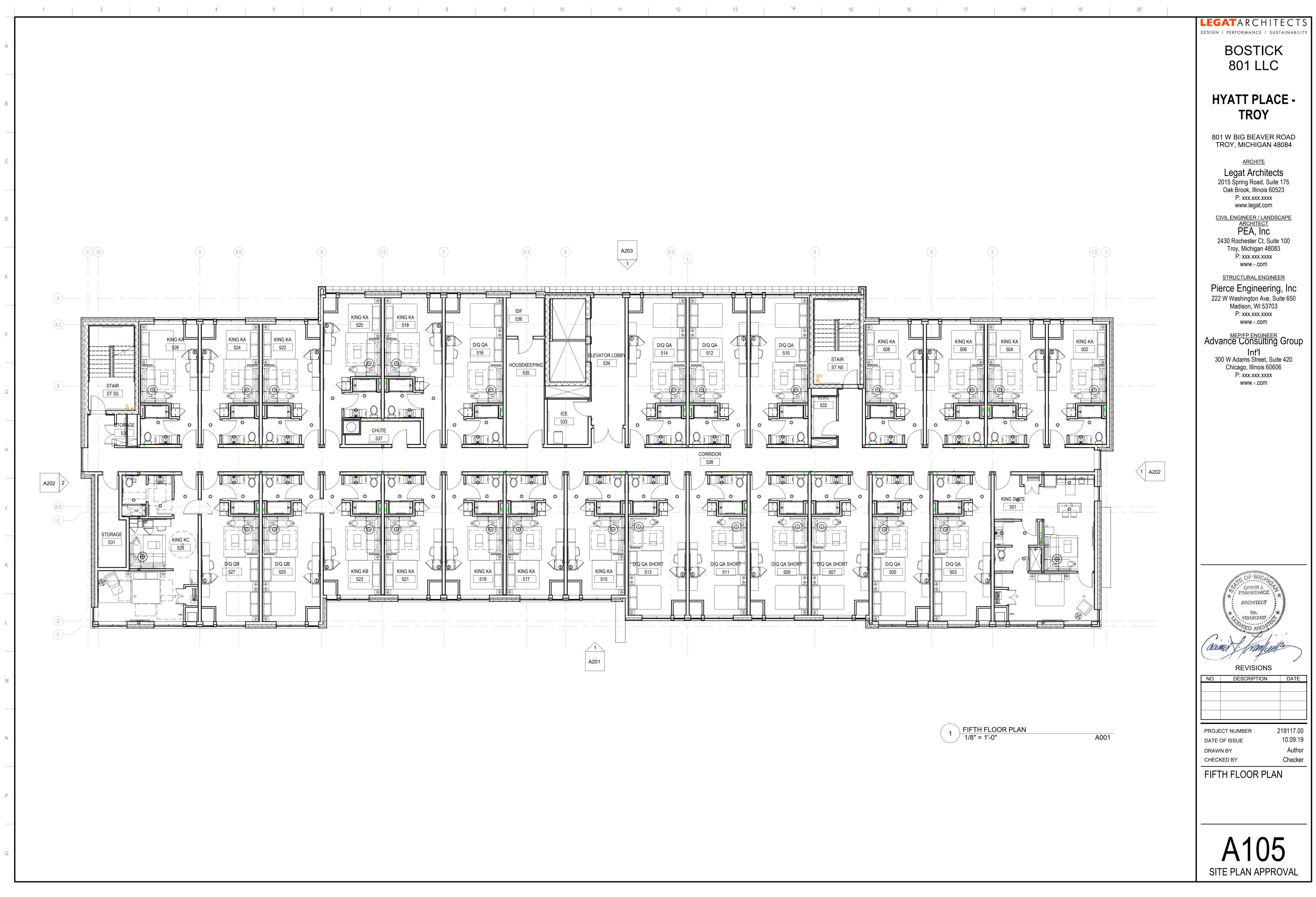


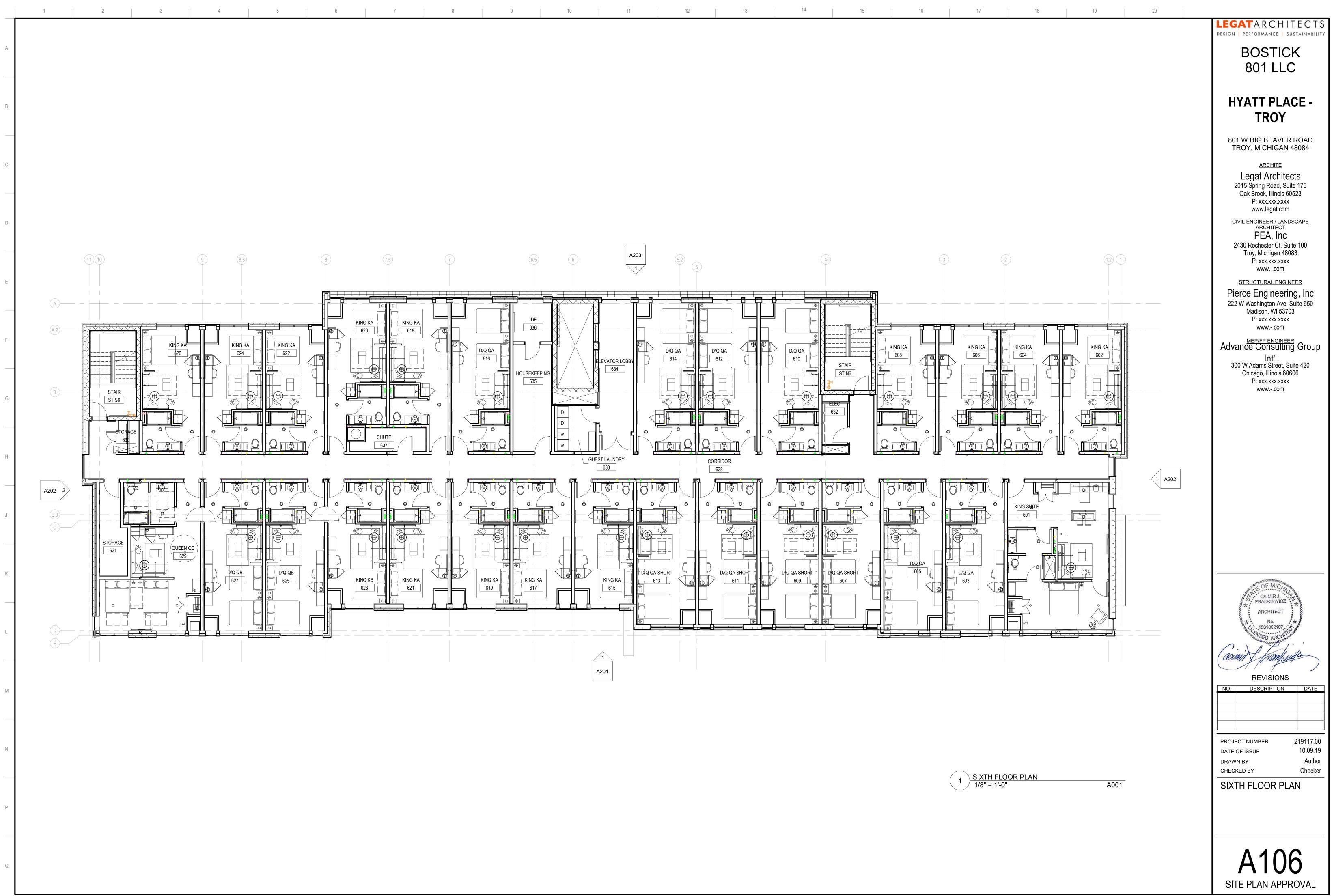


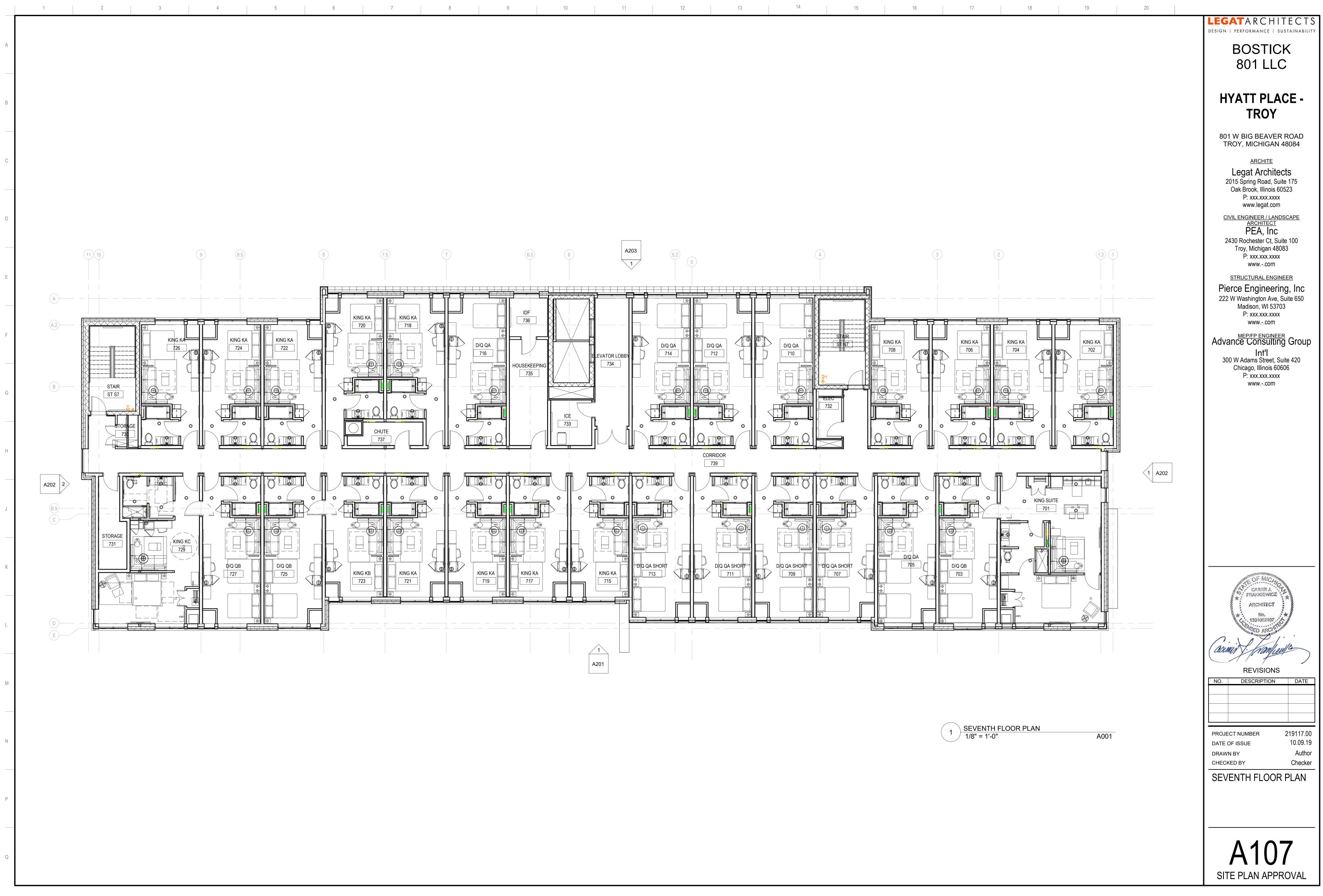
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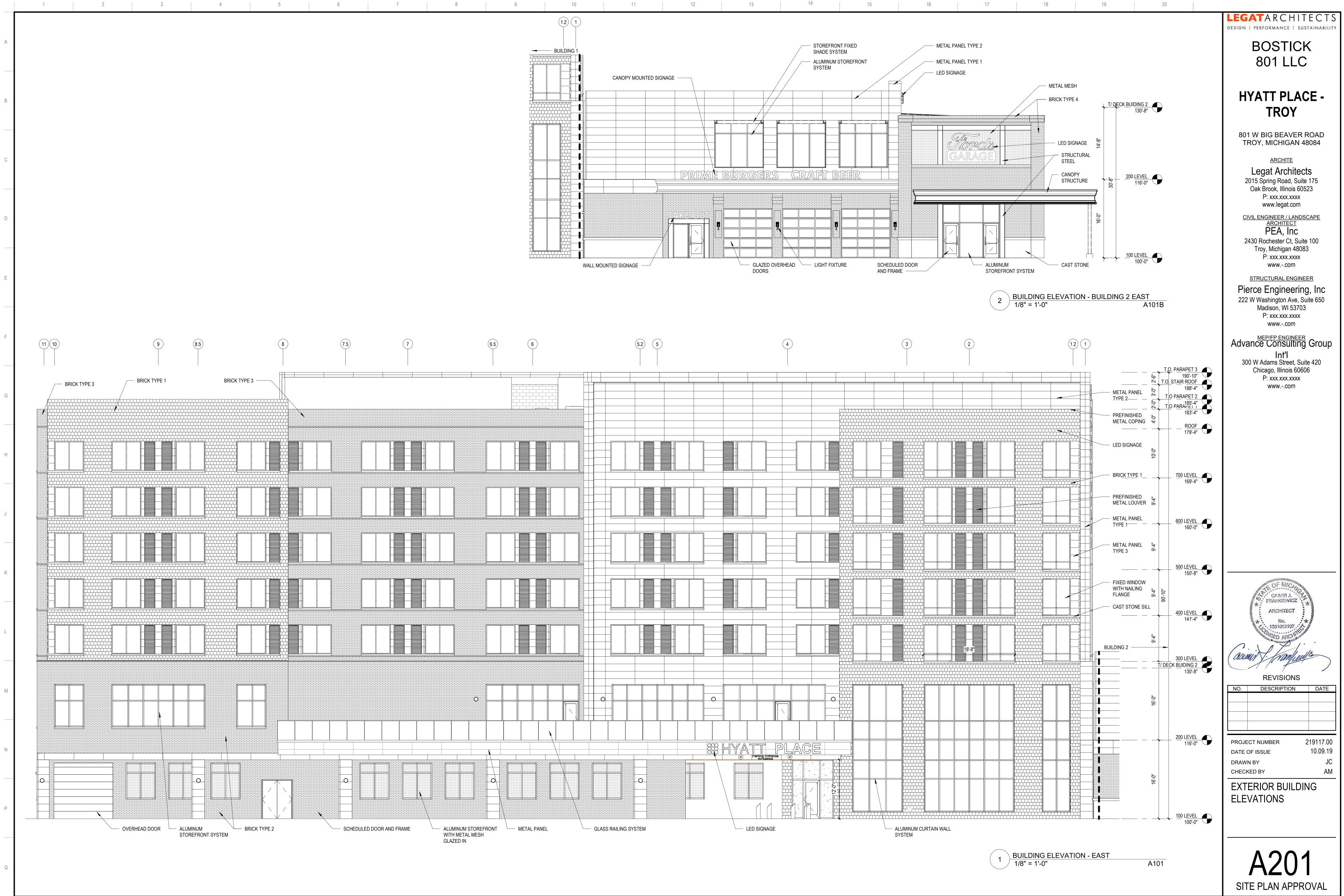


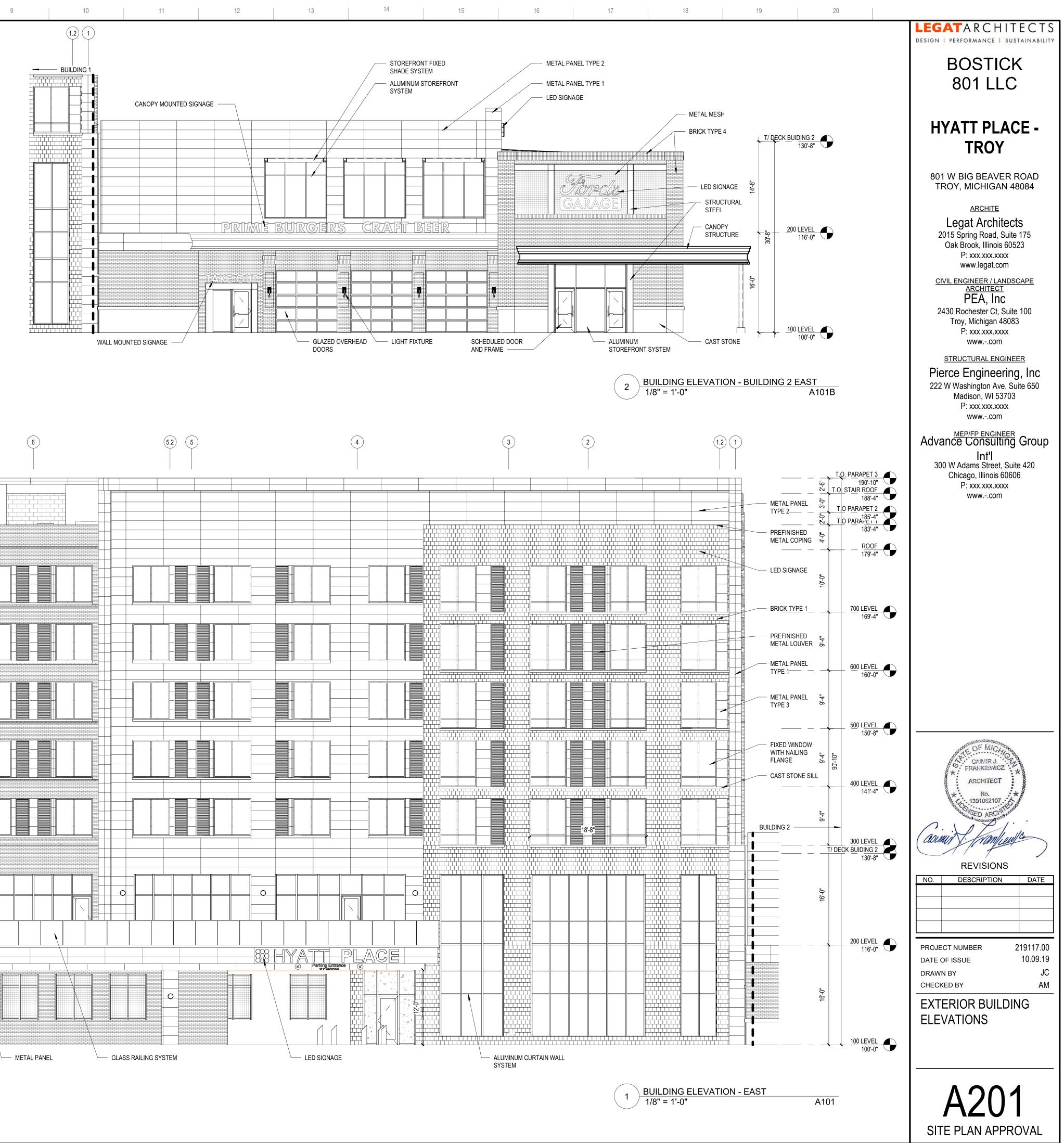


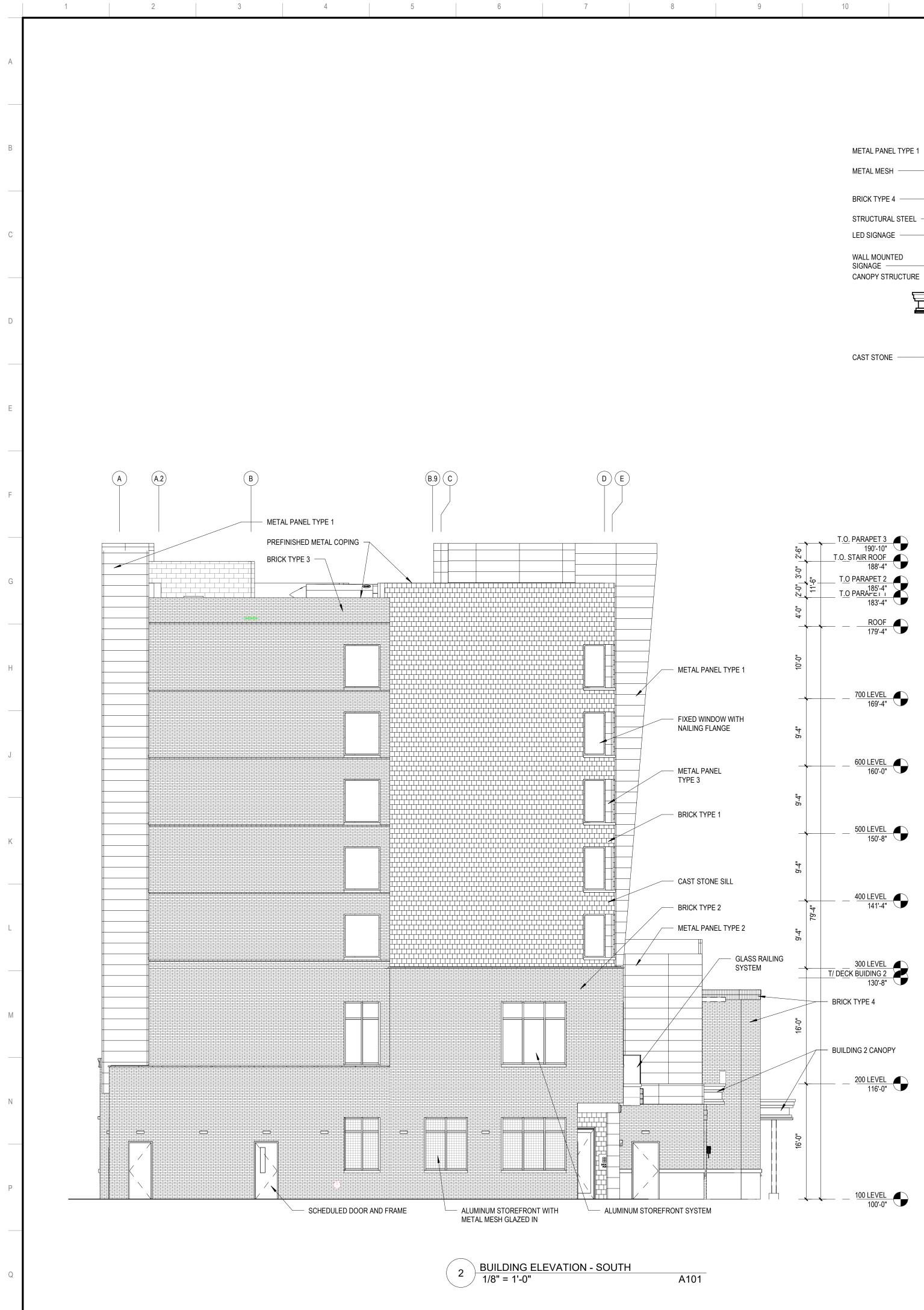


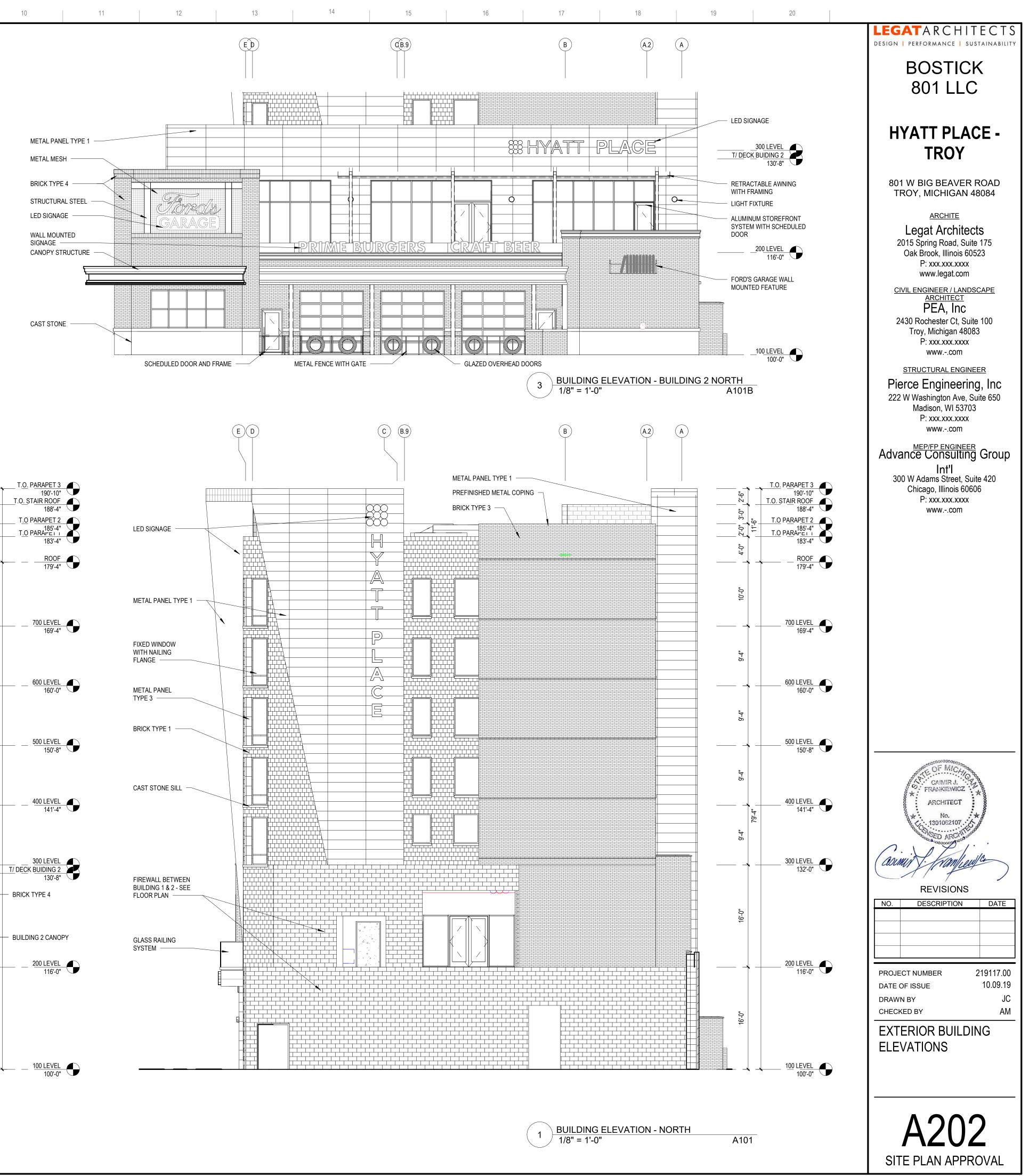


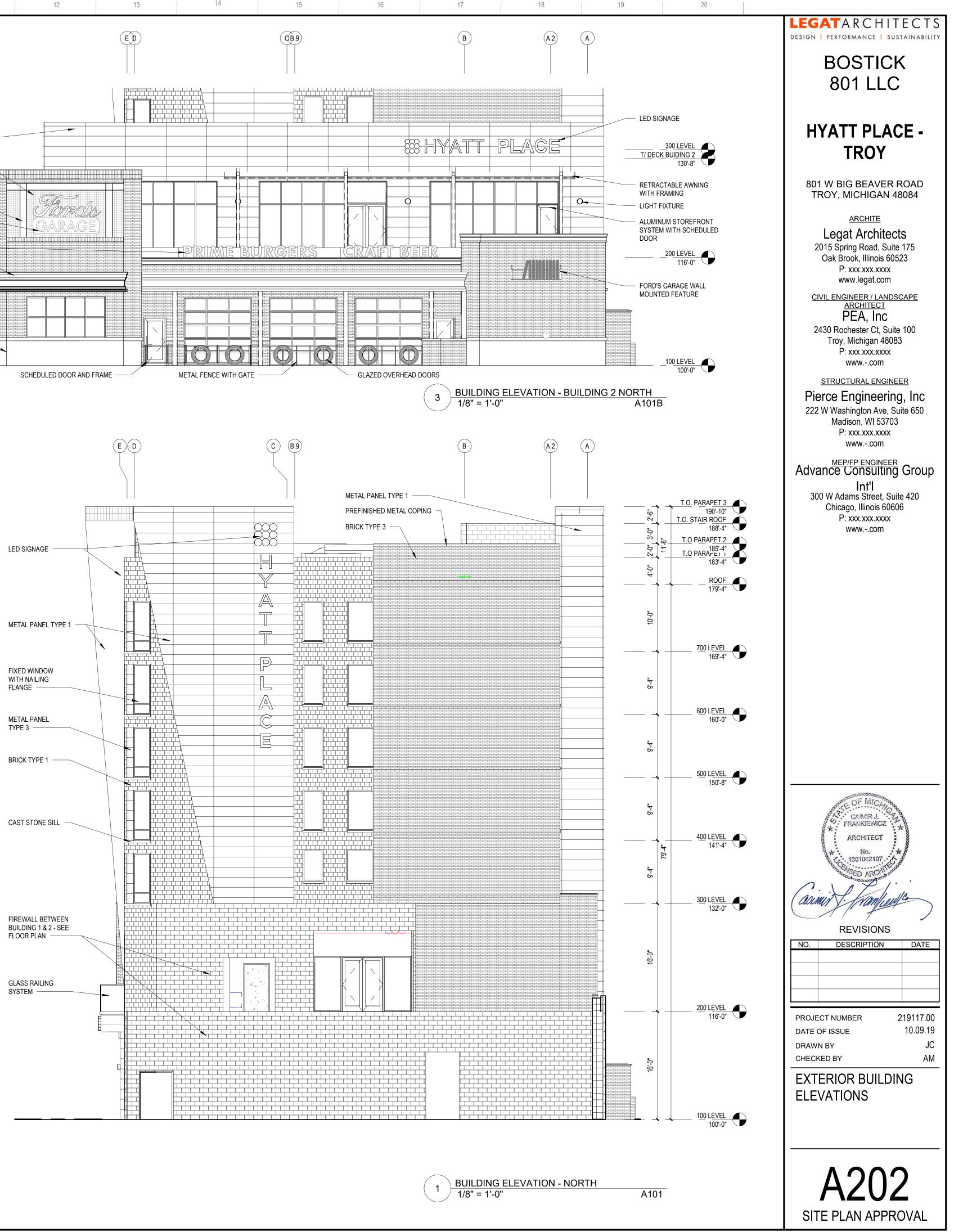




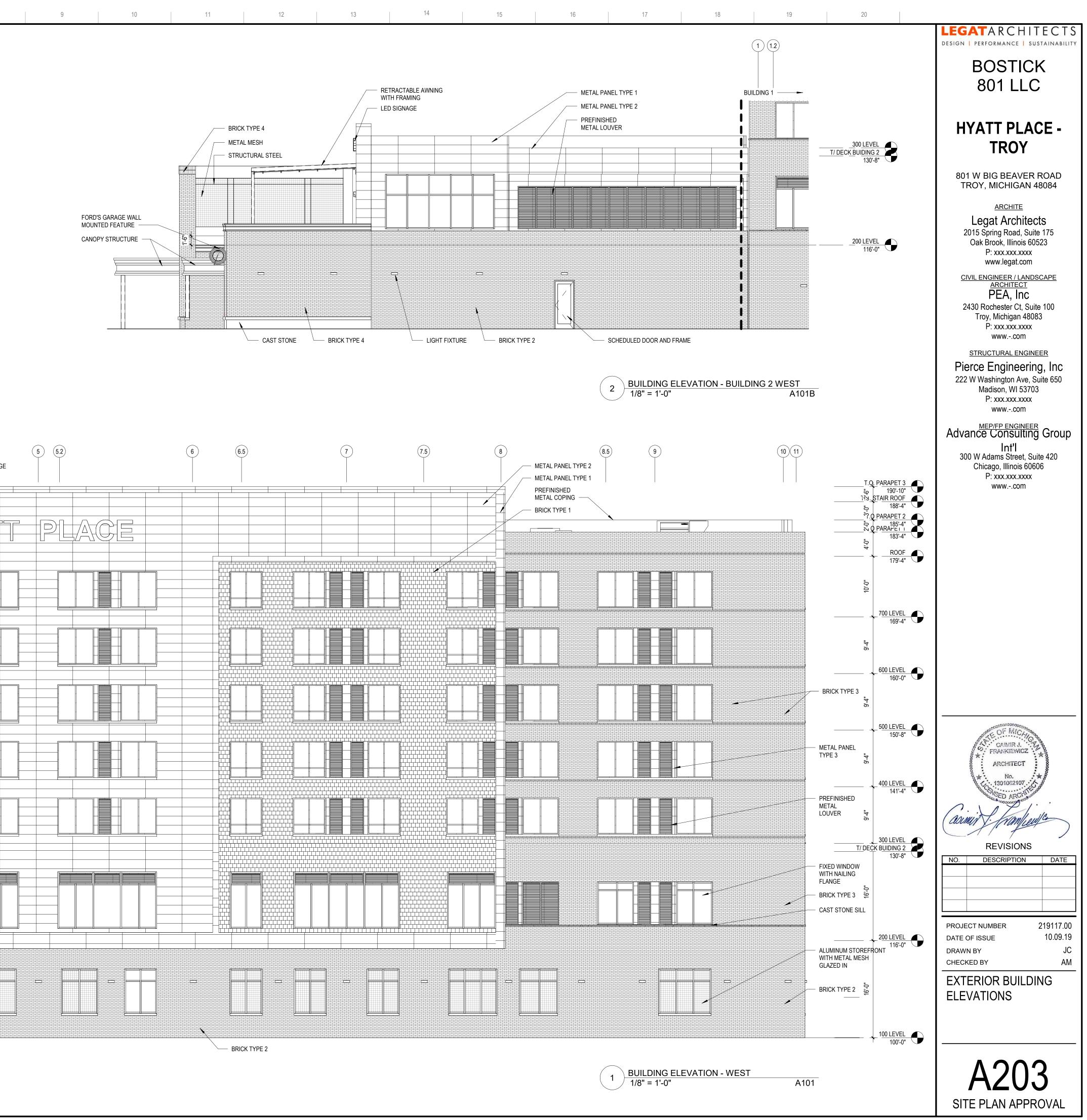


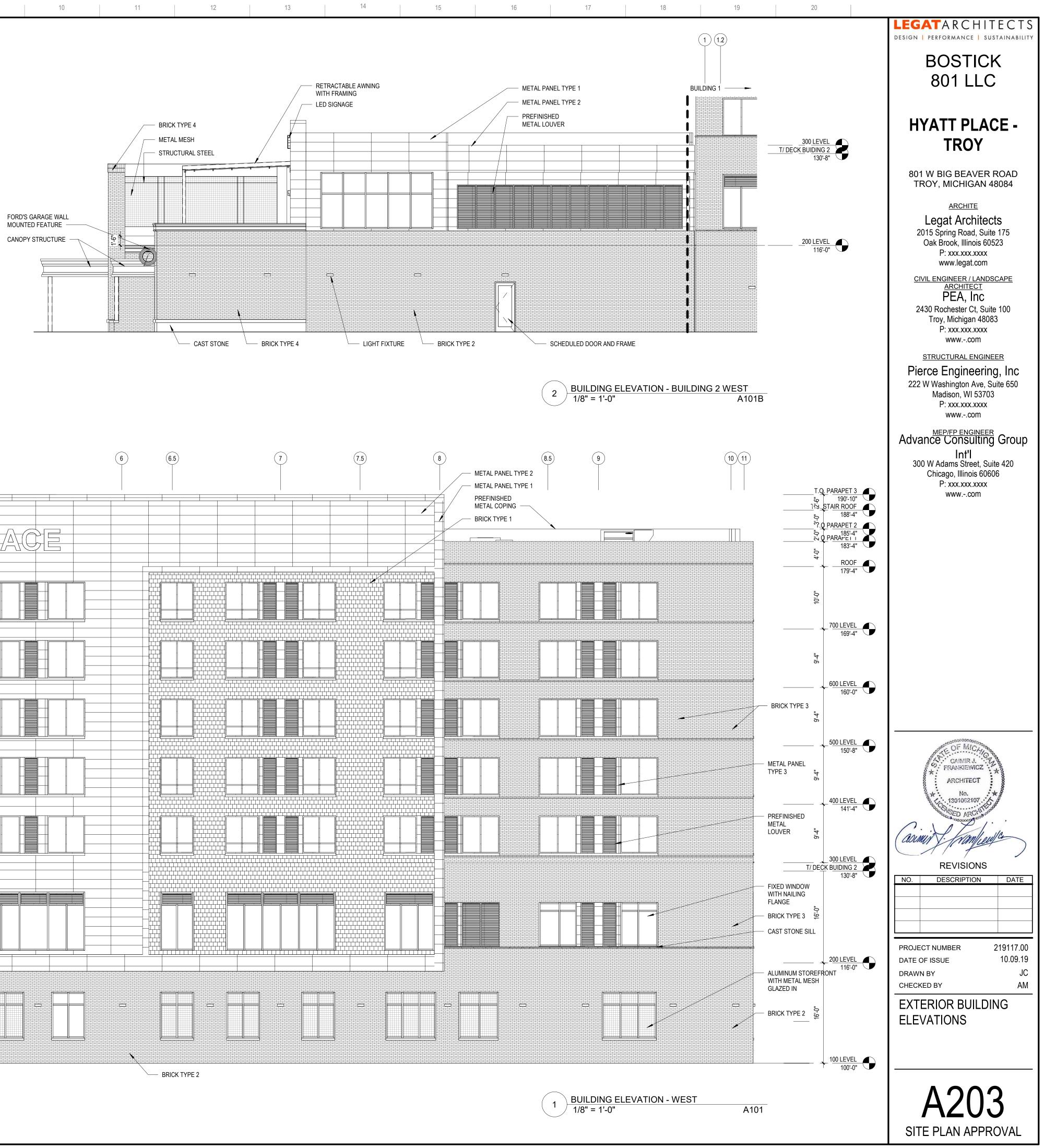


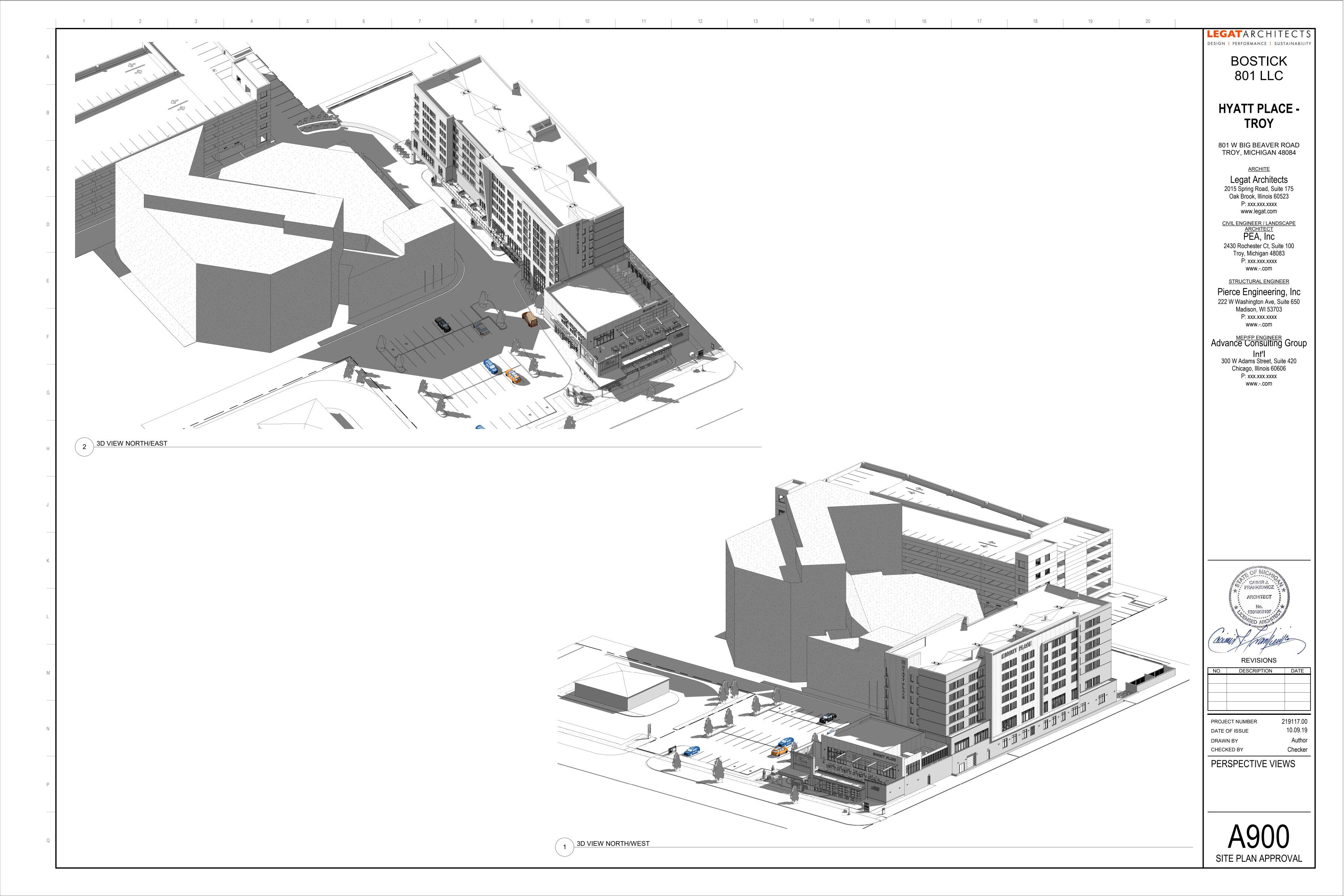














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LEGATARCHITECTS DESIGN | PERFORMANCE | SUSTAINABILITY

> BOSTICK 801 LLC

HYATT PLACE -TROY

801 W BIG BEAVER ROAD TROY, MICHIGAN 48084

ARCHITE

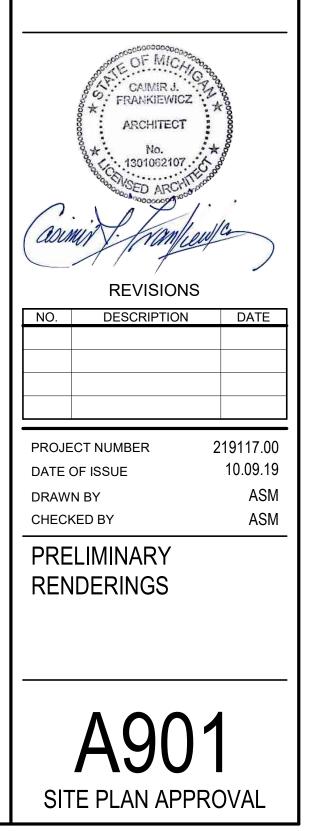
Legat Architects 2015 Spring Road, Suite 175 Oak Brook, Illinois 60523 P: xxx.xxx.xxxx www.legat.com

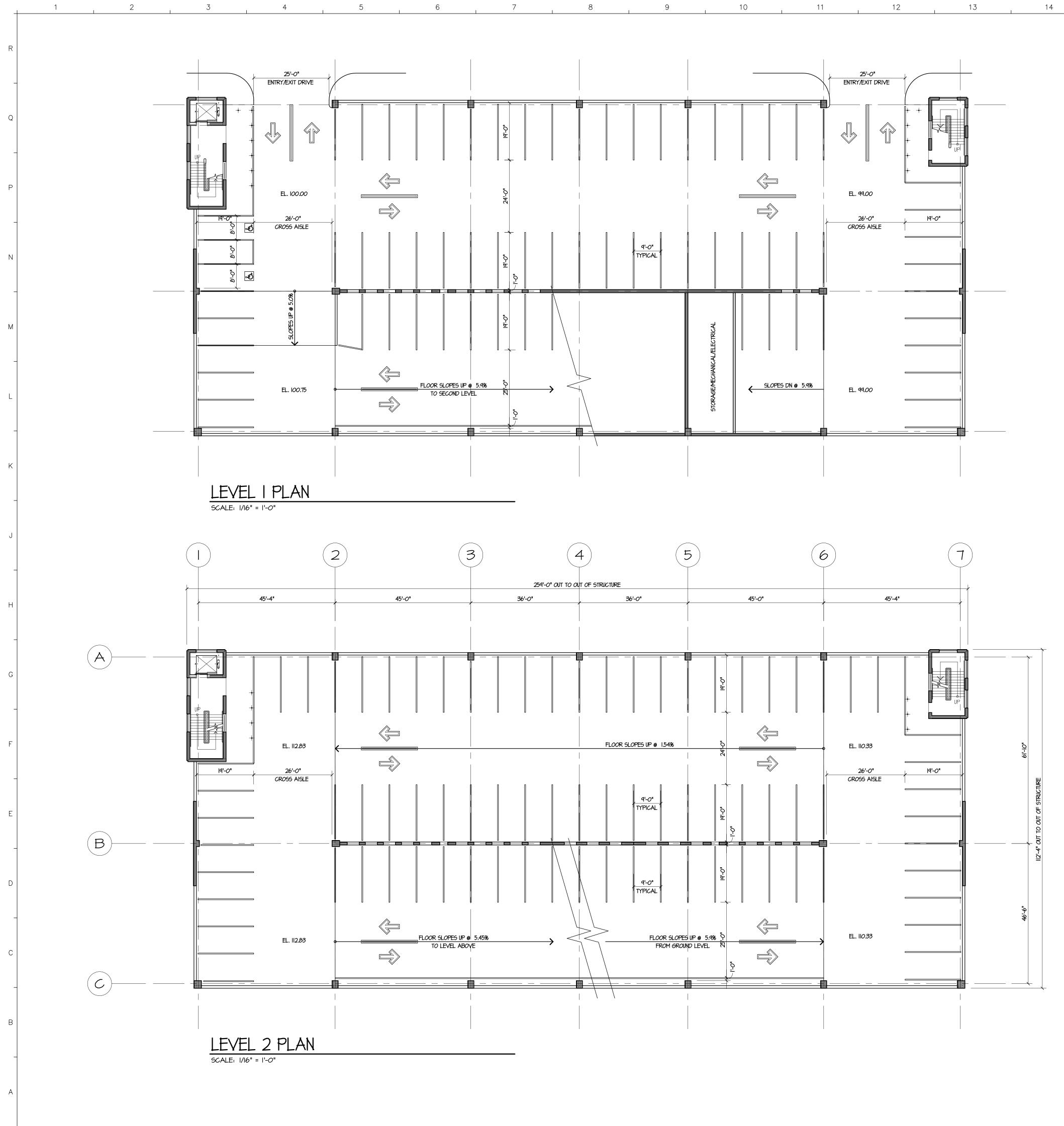
civil engineer / landscape <u>ARCHITECT</u> PEA, Inc 2430 Rochester Ct, Suite 100 Troy, Michigan 48083 P: xxx.xxx.xxx www.-.com

STRUCTURAL ENGINEER Pierce Engineering, Inc 222 W Washington Ave, Suite 650 Madison, WI 53703 P: xxx.xxx.xxxx www.-.com

Advance Consulting Group

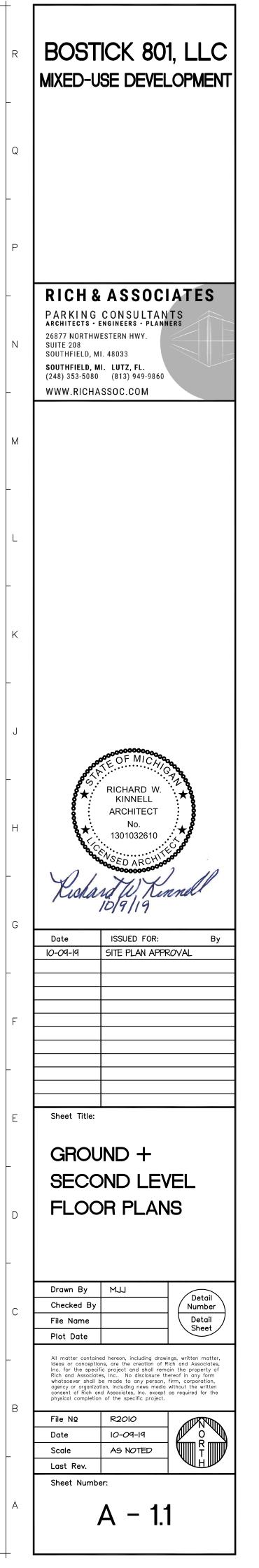
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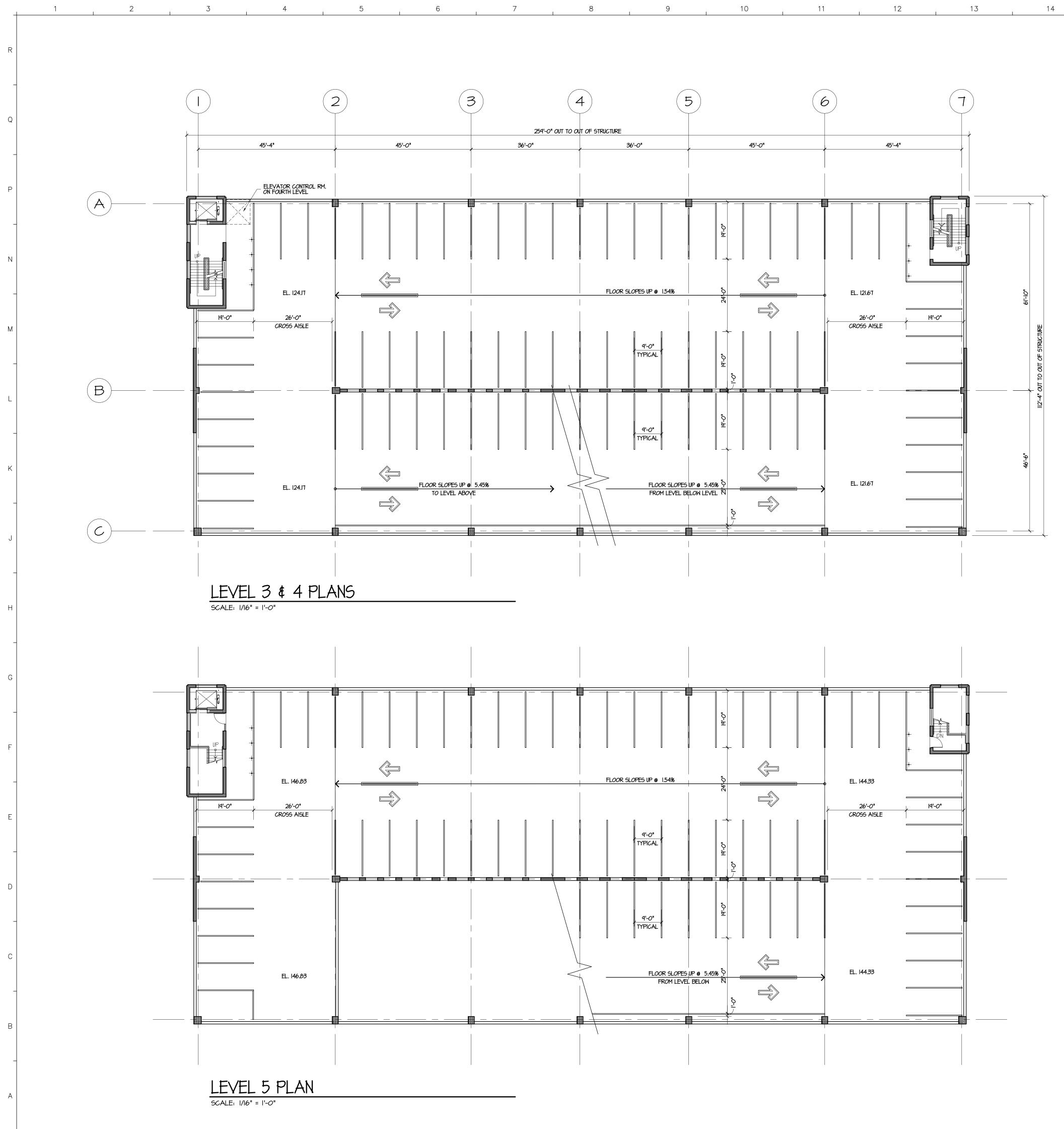






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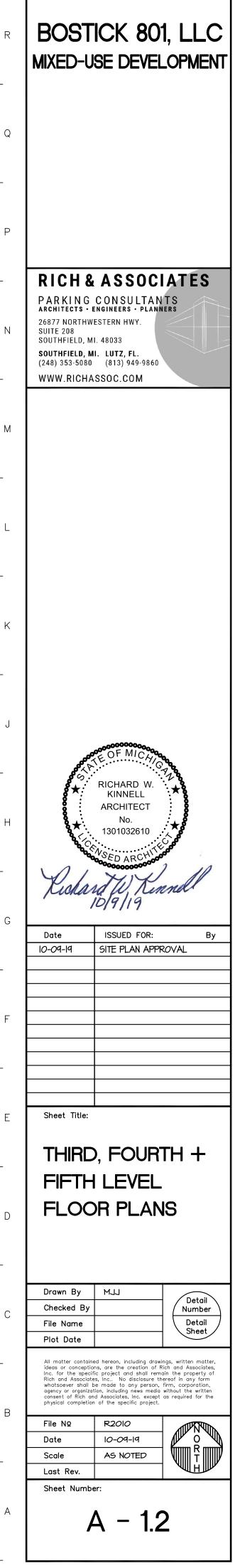


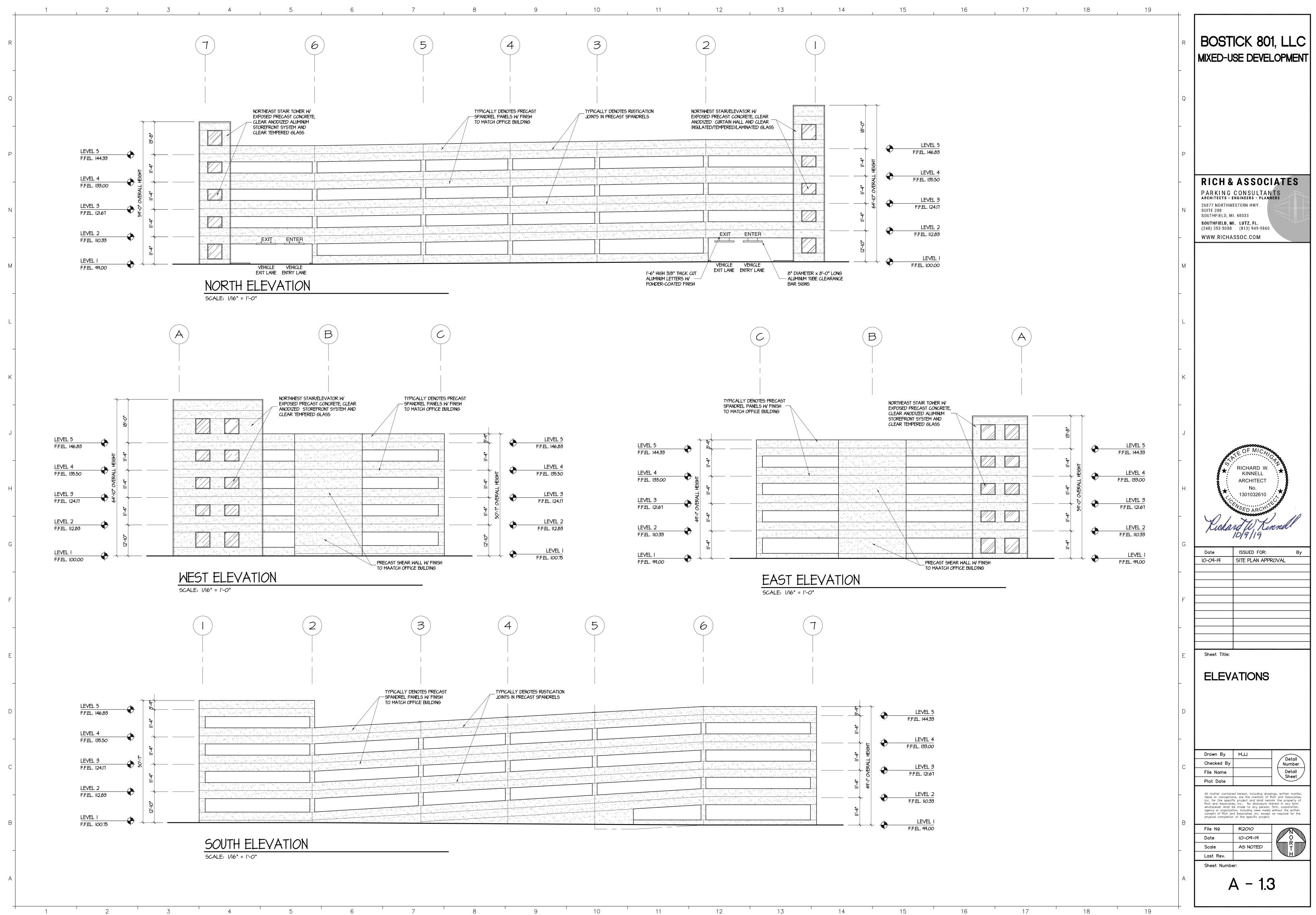






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DATE: December 5, 2019

TO: Planning Commission

- FROM: R. Brent Savidant, Community Development Director
- SUBJECT: <u>PUBLIC HEARING CONDITIONAL REZONING (CR JPLN2019-003)</u> Proposed Livernois Court, West of Livernois, North of Big Beaver, (88-20-22-301-008 and 88-20-22-301-009), Section 22, From R-1C (One Family Residential), to BB (Big Beaver Road) District.

The applicant Eureka Building Co. seeks a conditional rezoning of the subject parcel from R-1C (One Family Residential) to BB (Big Beaver) Zoning District. The applicant did not provide a complete Preliminary Site Plan application but did provide a conceptual sketch. The sketches indicate the potential for sixteen (16) units on the subject property. The applicant also voluntarily offered some conditions which are outlined in the attached report.

The site is within the Single Family Residential classification in the City of Troy Master Plan. The Planning Commission is a recommending body for this application.

The attached report prepared by Carlisle/Wortman Associates, Inc. (CWA), the City's Planning Consultant, summarizes the project. CWA prepared the report with input from various City departments including Planning, Engineering, Public Works and Fire. City Management supports the findings of fact contained in the report and the recommendations included therein.

Attachments:

- 1. Maps
- 2. Report prepared by Carlisle/Wortman Associates, Inc.
- 3. Application

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PROPOSED RESOLUTION

<u>PUBLIC HEARING – CONDITIONAL REZONING (CR JPLN2019-003)</u> – Proposed Livernois Court, West of Livernois, North of Big Beaver, (88-20-22-301-008 and 88-20-22-301-009), Section 22, From R-1C (One Family Residential), to BB (Big Beaver Road) District.

Resolution # PC-2019-12-

Moved by: Seconded by:

RESOLVED, That the Planning Commission hereby recommends to the City Council that the R-1C to BB conditional rezoning request, as per Section 16.04 of the City of Troy Zoning Ordinance, located on the west side of Livernois, north of Big Beaver, within Section 22, being approximately 7.65 acres in size, be granted for the following reasons:

- 1. The rezoning of the site to Big Beaver would allow for the potential project to have more flexibility to provide for the consideration of a range of components such as improving walkability and sense of place in the City, and even reducing emphasis on parking. Additionally, Chapter 8 of the City of Troy 2017 Master Plan indicates that here is a lack of availability of innovative housing styles due to the predominance of single-family detached homes. The rezoning of the site to Big Beaver with the creation of a multiple-family residential development would remedy this lack of housing variation, which is consistent with the Master Plan
- 2. The Form-Based District would permit greater flexibility in use and development of the property.
- 3. The conditions offered by the applicant reasonably protect the adjacent properties.
- 4. The rezoning would be compatible with surrounding zoning and land use.
- 5. The site can be adequately served with municipal water and sewer.
- 6. The development of the property should not unreasonably impact adjacent properties.

BE IT FURTHER RESOLVED, That the Planning Commission recommends the following site plan design considerations:

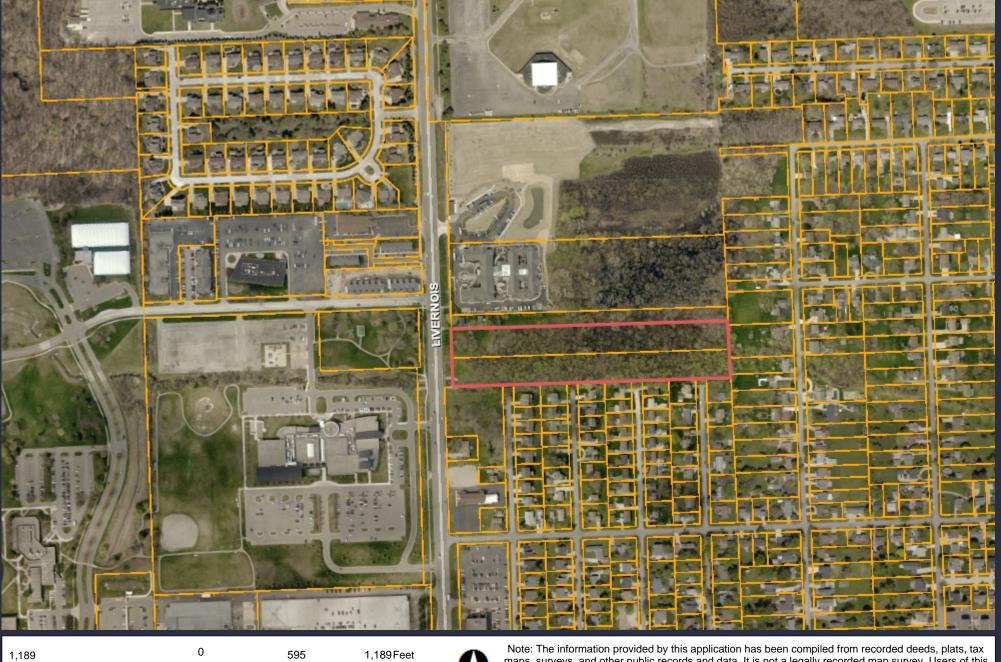
Yes: No:

MOTION PASSED / FAILED

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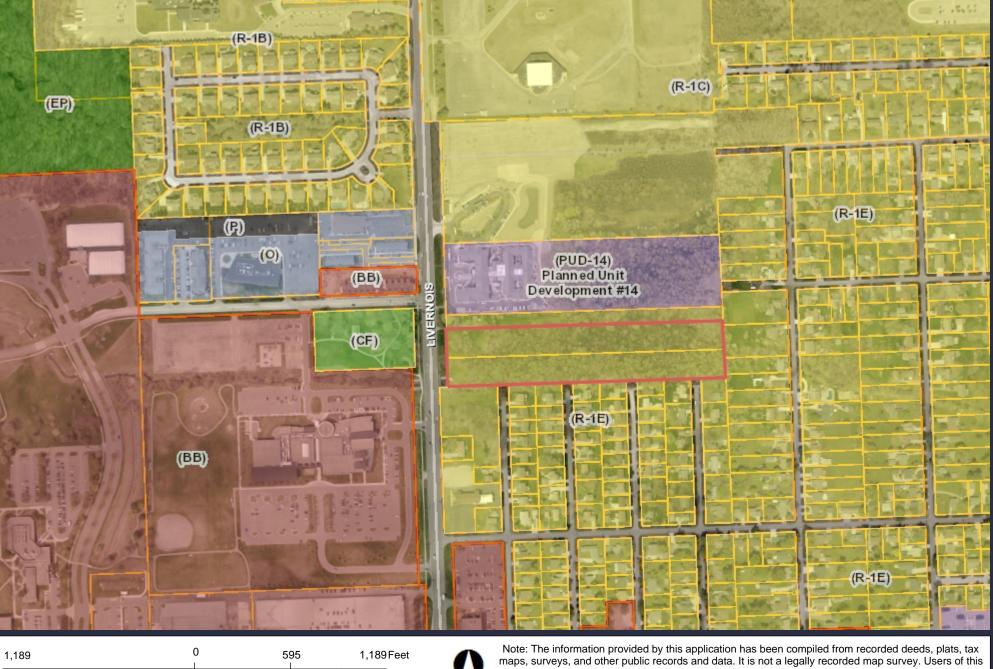
GIS Online



Note: The information provided by this application has been compiled from recorded deeds, plats, tax maps, surveys, and other public records and data. It is not a legally recorded map survey. Users of this data are hereby notified that the source information represented should be consulted for verification.



GIS Online



data are hereby notified that the source information represented should be consulted for verification.



117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

Date: November 20, 2019

Conditional Rezoning Analysis For City of Troy, Michigan

Project Name:	Livernois Road/Big Beaver Rezoning
Location:	Parcels 20-22-301-008 and 20-22-301-009 East of Livernois Road, North of Big Beaver Road
Current Zoning:	R-1C, One Family Residential
Proposed Rezoning:	Conditional Rezoning: Big Beaver Multi-Family Residential
Required Information:	As provided within this review

PROJECT AND SITE DESCRIPTION

The applicant is proposing to conditionally rezone parcels 20-22-301-008 and 20-22-301-009 from R-1C, One Family Residential to BB, Big Beaver. Conditions offered by the applicant are as follows:

- 1. Big Beaver development to occur only in the areas outside of the 100-year floodplain and wetland delineation. The 100-year floodplain and wetland areas are to be treated as conservation and are not to be developed.
- Proposed parcels to provide a 40-foot egress easement to the parcel to the north in case of future development. Easements shall satisfy ingress/egress guidelines for such development.
- 3. Big Beaver development is to include only attached multi-family unit buildings. Such buildings shall not exceed 35-feet in height and shall have a minimum setback of 35-feet from R-1C parcels.

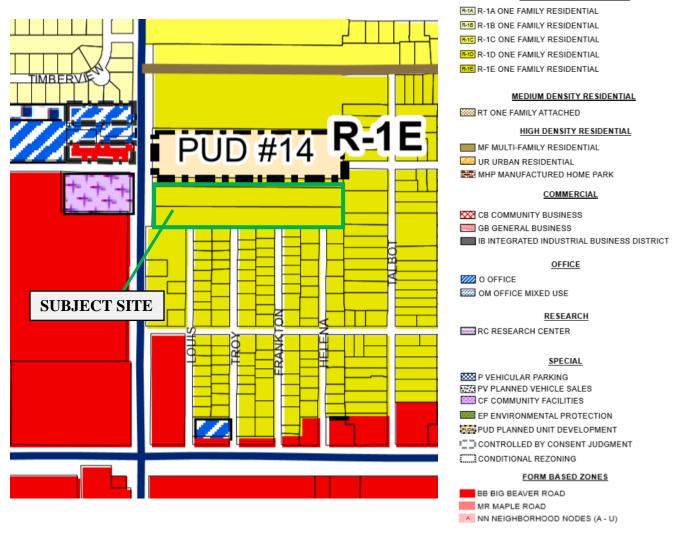
The applicant should indicate how they plan on preserving (conservation easement, land donation, or other means) the wetland and floodplain area.

Although no detailed development plans have been provided at this time, the applicant has submitted a conceptual plan for informational purposes only. If the conditional rezoning is approved, the applicant shall provide a site plan that meets the conditions voluntarily offered, and applicable standards set forth in the Zoning Ordinance including the BB Design Guidelines.

It should also be noted that the conceptual drawings provided include the parcel directly north which is currently zoned as R1-C, one family residential, as they may potentially obtain it for future development.

ZONING

The subject site is currently zoned as R-1C, One Family Residential. An excerpt from the City's zoning map is provided below.



SURROUNDING ZONING AND LAND USE

The subject parcel is currently zoned as R-1C. The Zoning and existing land uses for the subject site and surrounding parcels is listed below.

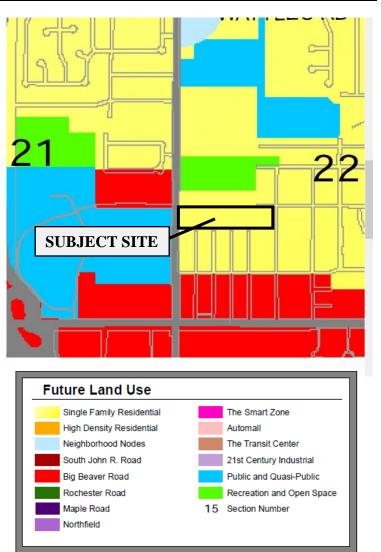
Direction	Zoning	Existing Use
North	R-1C, One Family Residential	One-Family Residential
South	R-1E, One Family Residential	One-Family Residential
East	R-1C, One Family Residential	One-Family Residential
West	CF, Community Facilities	Parks and Recreation

MASTER PLAN

According to the Future Land Use Plan (excerpt shown to the right), this area is planned as Single Family Residential. The existing zoning as One-Family Residential supports this intent and is consistent with existing zoning and future land use goals of those parcels immediately east, north and south.

The proposed land use as BB Multiple Family Residential is more intense than the uses intended, however, it is consistent with future land uses of parcels located to its west.

Due to traffic, surrounding land uses, and limited developable area due to wetland/floodplain it is unlikely that the site will develop as single-family residential.



PROPOSED DEVELOPMENT VS. BY-RIGHT

The stated intent of the BB Big Beaver Zoning District is as follows:

SECTION 5.04, Big Beaver District

Intent. The Big Beaver (BB) District is intended to implement the policies set forth in the Big Beaver Corridor Study, Big Beaver Design Guidelines, and the City's Master Plan. These regulations are intended to promote a unified vision for transforming Big Beaver Road into a world-class destination focused on mixed-use development and increased land use intensity that is oriented as much to the needs of the pedestrian as to those of the automobile. These regulations are also intended to:

- 1. Establish a development pattern in which new buildings and building modifications enhance the character of the existing built environment;
- 2. Orient building entrances and storefronts to the street to add visual interest, increase pedestrian traffic, and to reduce crime through increased surveillance;
- 3. Enhance a sense of place and contribute to the sustainability of the City.
- 4. Allow a pattern of development which will encourage transportation alternatives (walking, biking, and transit) to reduce automobile dependence and fuel consumption; and
- 5. Add value to property along the Big Beaver Corridor

The stated intent of the existing zoning district, R-1C, One Family Residential is as follows:

SECTION 4.06, One-Family Residential Districts R-1A through R-1E

Intent. The Master Plan recognizes that single-family residential neighborhoods are vital components of the City and comprise the majority of the land area within the City. The intent of the R-1A through R-1E Districts is to provide areas for single-family dwellings with the primary distinction being a range of densities, implemented through varying lot sizes. The R-1A through R-1E Districts are further intended to preserve and improve upon the quality of residential neighborhoods while permitting a limit

The table below outlines the development differences of what is currently permitted per R-1C zoning and the conditions offered by the applicant.

	By-Right Existing R-1C Zoning	Permitted and Proposed if rezoned to BB Zoning
Height	2.5 stories and 30 feet	35 feet
Setbacks	Front (Livernois): 30 feet	Minimum of 35 feet from adjacent R-1C parcels
	Sides (No sewer): 15/30 feet total	
	Sides (Sewer): 10/20 feet total Rear: 40 feet	

The applicant has also set forth two (2) additional conditions. First, the proposed parcels are to provide a 40-foot egress easement to the parcel to the north in case of future development. Easements are to satisfy egress guidelines for such development. Second, BB development is to occur only in areas outside of the 100-year floodplain and wetland delineation. Those naturally sensitive areas are to be treated as conservation and are not to be developed under the rezoning request. The applicant should indicate how they plan on preserving (conservation easement, land donation, or other means) the wetland and floodplain area.

REZONING STANDARDS

As set forth in Section 16.03.C, the Planning Commission shall consider the following standards:

- 1. The proposed rezoning is consistent with the Master Plan. If the current zoning is in material conflict with the Master Plan, such conflict is due to one of the following:
 - a. A change in City policy since the Master Plan was adopted.
 - b. A change in conditions since the Master Plan was adopted.
 - c. An error in the Master Plan.

The proposed land use as BB Multiple Family Residential is more intense than the uses intended, however, it is consistent with future land uses of parcels located to its west. The rezoning of the site to Big Beaver would allow for the potential project to have more flexibility than is currently allowed by the existing zoning district. This flexibility may allow for the consideration of a range of components such as improving walkability and sense of place in the City, and even reducing emphasis on parking. Additionally, Chapter 8 of the City of Troy 2017 Master Plan indicates that here is a lack of availability of innovative housing styles due to the predominance of single-family detached homes. The rezoning of the site to Big Beaver with the creation of a multiple-family residential development would remedy this lack of housing variation.

As noted due to traffic, surrounding land uses, and limited developable area due to wetland/floodplain it is unlikely that the site will develop as currently zoned single-family residential.

2. The proposed rezoning will not cause nor increase any non-conformity.

The applicant has stated that any new development will be designed in conformity with current City Ordinances and will undergo the site plan review process prior to approval in order to ensure that all standards have been met and conformity is achieved.

3. Public services and facilities affected by a proposed development will be capable of accommodating service and facility loads caused by use of the development.

Due to the condition that is aimed at restricting development from occurring within the wetland and floodplain areas on site, it is not likely that any improvements will overwhelm

any local public services or facilities. The site will likely be easily accommodated by public services and facilities that serve the surrounding, more intense developments.

4. The rezoning will not impact public health, safety, or welfare.

The proposed rezoning is not likely to impact public health, safety, or welfare. Conditions prohibiting development from occurring within the 100-year floodplain ensure that residentially zoned parcels to the east are minimally affected the anticipated project, and that preserved area can serve as a buffer and transition area in perpetuity. Additionally, the applicant has indicated that the site's intended use as multiple-family residential, will provide additional housing options within the City.

5. The rezoning will ensure compatibility with adjacent uses of land.

The rezoning of the site from R-1C, One Family Residential to BB Multiple Family Residential will be more intense than those parcels to the south and east as they are currently zoned and used for single family residential purposes. However, the subject site is in close proximity to the Big Beaver district, less intense than the senior facility to the north, and will act as a transition area between the more intense developments to the less intense residential areas.

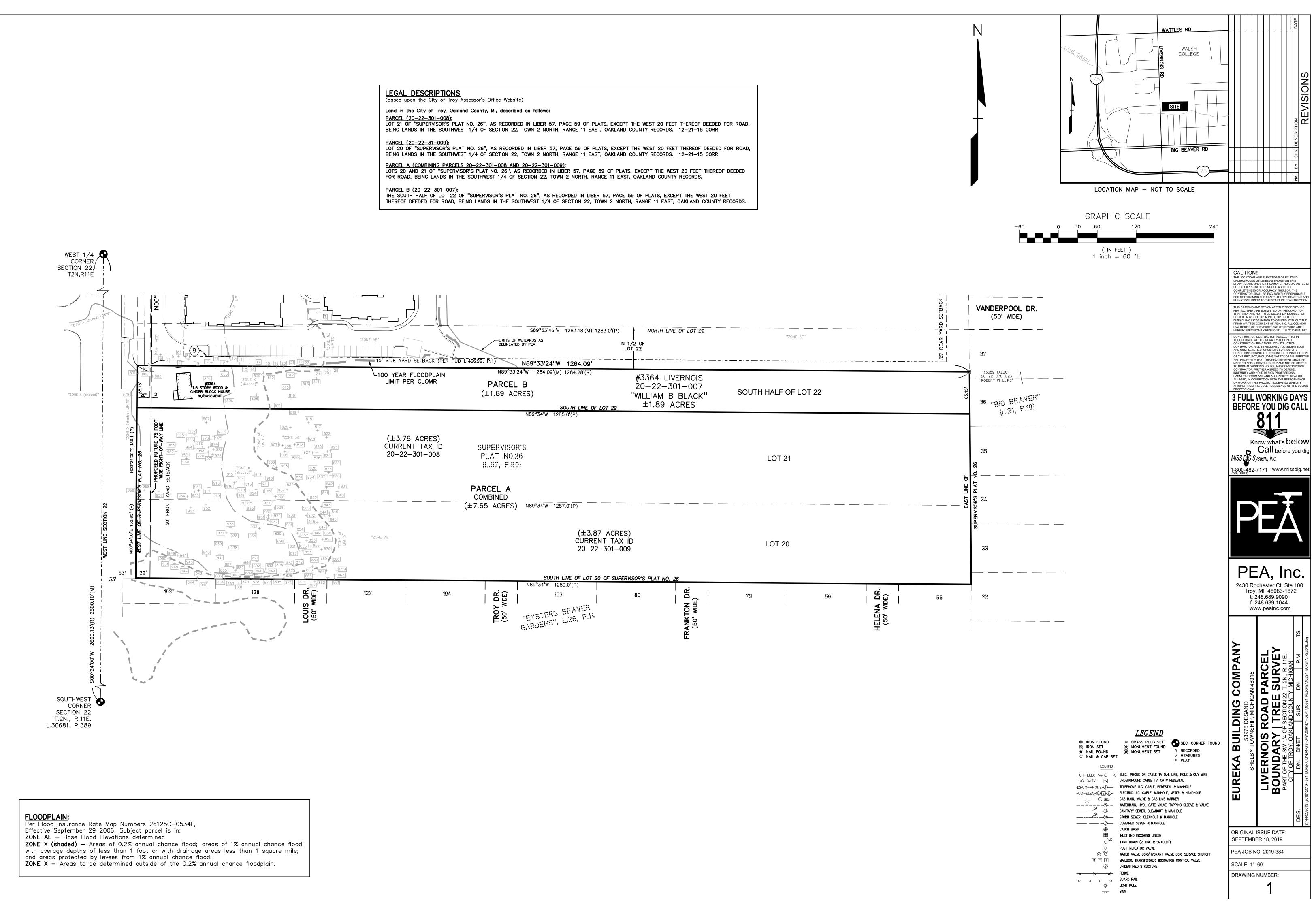
RECOMMENDATION

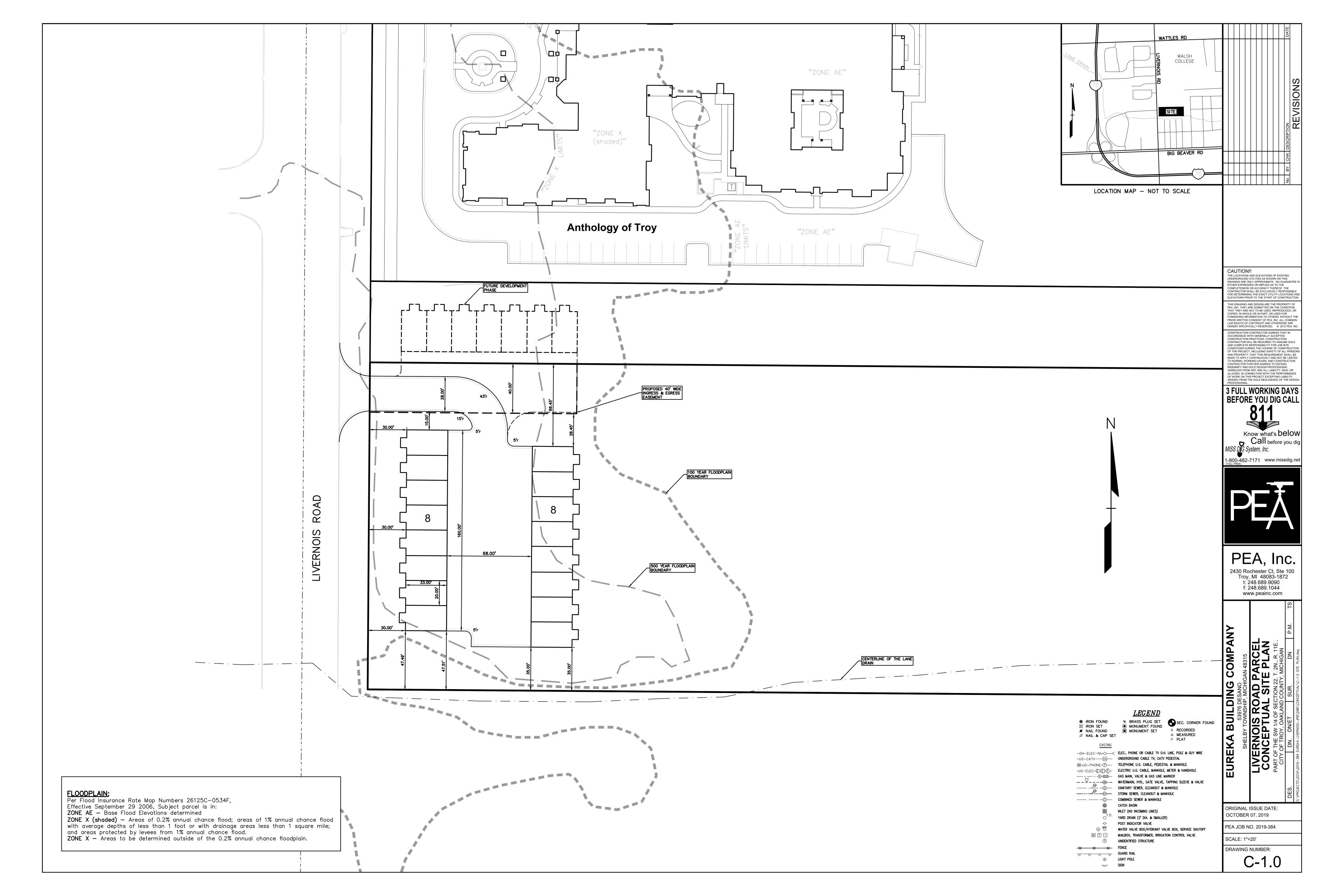
As noted due to traffic, surrounding land uses, and limited developable area due to wetland/floodplain it is unlikely that the site will develop as currently zoned single-family residential.

Multiple-family residential, with the applicant's condition of limiting height and the condition that wetlands and 100-year flood plain will be conserved to serve as a buffer to the less intense uses to the south and east, is an appropriate transitional land use. However, the applicant should indicate how they plan on preserving (conservation easement, land donation, or other means) the wetland and floodplain area.

We recommend that the Planning Commission consider the application, hold a public hearing, and consider public comments.

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP





Rezoning Statement.

ONE (1) HARD COPY OF A PROPOSED SITE PLAN INDICATING THE SUBJECT PROPERTY AND THE BUILDINGS / USES PROPOSED TO BE CONSTRUCTED

A detailed development is not proposed at this time, since the uses will be tied directly into the requirements of the Big Beaver one family attached. Once the rezoning to Big Beaver is approved, through Planning Department direction the development site plan layouts and associated requirements under the zoning ordinance will be pursued in the ordinary course of action. All site plan, and associated form-based elements, will then be addressed though a site plan submission meeting pursuant to ordinance requirements.

Re-zoning request is subject to the following conditions:

1. BB development to occur only in the areas outside of 100-year flood plain and wetland delineation as per the attached site plan concept. The 100-year flood plain and wetlands areas to be treated as conservation and are not to be developed under the proposed re-zoning request.

2. Proposed parcels to provide 40' egress easement to parcel to the north in case of future development. Easement to satisfy egress guidelines for such development.

3. BB development to include only attached multifamily unit buildings. Such buildings shall not exceed 35' in height and shall have a minimum setback of 35' to R-1C parcels.

ONE (1) HARD COPY OF A STATEMENT INDICATING WHY, IN THE OPINION OF THE APPLICANT, THE REZONING REQUESTED IS CONSISTENT WITH THE MASTER PLAN, AND WHY SUCH A REZONING IS CONSISTENT WITH ADJACENT ZONING DISTRICTS AND USES, AND WILL NOT BE DETRIMENTAL TO THE PROPERTY OF OTHER PERSONS LOCATED IN THE VICINITY

The above referenced properties are located within BB zoning areas to the west of Livernois. Re-zoning to BB should meet Troy Master Plan objectives by providing a transition from non-residential area & main artery roadway and lower density residential land use areas. The City of Troy Zoning Ordinance has additional requirements that must be met for all rezoning approvals. More specifically, Section 16.03(C), Rezoning Procedures, of the ordinance provides the following standards for rezoning requests:

C. Standards for Approval. A rezoning may be approved upon a finding and determination that all of the following are satisfied:

1. The proposed rezoning of the subject site from R1-C, One Family Residential District, to BB Multi Family Residential, is consistent with the Land Use of the surrounding areas. The project enables medium density residential development in one of the "Targeted Locations in Troy for Missing Middle housing." (See Master Plan Chapter 8 Pg. 6) Rezoning the site to BB would enable the project to satisfy the characteristics of Missing Middle Housing as noted in the Master Plan by providing for: 1. Walkable context and sense of community; Transitional density, by creating housing at densities which fall between traditional single family and multiple family; Smaller, well designed units and efficient use of space; Deemphasizing parking.

2. The proposed rezoning will not cause nor increase any non-conformity.

The proposed rezoning to BB One Family Attached will not cause or increase any non-conformity. It will allocate \sim 5.5 acres of wetland conservation to the east of the parcels. Any new development will be in conformity with current City ordinances and Planning Department site plan review and approval as such the development will of necessity continue and transition the current theme of the neighborhood.

3. Public services and facilities affected by a proposed development will be capable of accommodating service and facility loads caused by use of the development.

The subject site is not large in size (1.5 acres) and any proposed development, in compliance with the RT One Family Attached would be easily accommodated by public services and facilities that serve the surrounding commercial and residential developments.

4. The rezoning will not impact public health, safety, or welfare.

The proposed BB designation for this site, and the associated development review process, is intended to ensure that a project does not impact the public health, safety, or welfare. The very designation of the site is intended to provide affordable housing for young families.

5. The rezoning will ensure compatibility with adjacent uses of land.

The rezoning will continue an ongoing theme in the Livernois Road corridor north of Big Beaver road where a number of BB developments have been approved by the city and constructed in recent years in conformity with the Master Plan and its dictates.

ONE (1) CD CONTAINING AN ELECTRONIC VERSION OF THE COMPLETE REZONING APPLICATION

A flash drive with the complete application is provided

To the City of Troy,

William B. Black

3364 Livernois, Troy, MI 48083

I am the property owner to the north of lots Parcel # 2022301008, 2022301009.

I support their efforts to have the property rezoned to residential multiple family.

Sincerely,

W Mon Block

William B. Black

- DATE: December 5, 2019
- TO: Planning Commission
- FROM: R. Brent Savidant, Community Development Director

SUBJECT: <u>PUBLIC HEARING – ZONING ORDINANCE TEXT AMENDMENT (File Number</u> <u>ZOTA 254)</u> – Cluster Square Footage

This item was initiated by Staff based on conversations with the Planning Commission and developers. The One Family Cluster Option incentivizes expanding a variety of housing types in the City via construction of smaller homes that suitable for empty nesters and retirees. Developers have indicated that the maximum 1,500 square foot requirement is too restrictive and is a barrier to providing smaller homes.

The attached report summarizes the issue. The proposed text amendment involves simply raising the minimum square footage from 1,500 square feet to 1,700 square feet.

A Public Hearing is scheduled for this item for the December 10, 2019 Planning Commission meeting.

Attachments:

- 1. Planning Commission Public Hearing Draft.
- 2. Memo prepared by Carlisle/Wortman Associates, Inc., dated October 15, 2019.
- 3. Public input.

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PROPOSED RESOLUTION

<u>PUBLIC HEARING – ZONING ORDINANCE TEXT AMENDMENT (File Number ZOTA 254)</u> – Cluster Square Footage

Resolution # PC-2019-12-

Moved by: Seconded by:

RESOLVED, That the Planning Commission hereby recommends to the City Council that Article 10 of Chapter 39 of the Code of the City of Troy, which proposes to increase the maximum square footage for smaller homes qualifying for a density bonus under the One Family Cluster Option from 1,500 square feet to 1,700 square feet, be amended as printed on the proposed Zoning Ordinance Text Amendment.

Yes: No: Absent:

MOTION CARRIED / DENIED

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CITY OF TROY

AN ORDINANCE TO AMEND CHAPTER 39 OF THE CODE OF THE CITY OF TROY CITY COUNCIL PUBLIC HEARING DRAFT

The City of Troy ordains:

Section 1. Short Title

This Ordinance shall be known and may be cited as an amendment to Chapter 39, Zoning Ordinance, of the Code of the City of Troy.

Section 2. Amendment

Chapter 39 of the Code of the City of Troy is amended as follows:

Revise Section 10.04.H.2 to read as follows:

- 2. Housing Diversity and Options. A bonus above the base yield number of units established in 10.04.C.1 may be provided for a development that provides a diverse variety of housing types or provides a type of housing that is desired, but not currently offered in the city. The following requirements shall be met for the all bonus unit in excess of the base yield number of units:
 - a. Maximum home square footage shall not exceed 1,500 1,700 sq/ft; and
 - b. Master first floor bedroom and bathroom shall be provided; and

Section 3. Savings

All proceedings pending, and all rights and liabilities existing, acquired or incurred, at the time this Ordinance takes effect, are hereby saved. Such proceedings may be consummated under and according to the ordinance in force at the time such proceedings were commenced. This ordinance shall not be construed to alter, affect, or abate any pending prosecution, or prevent prosecution hereafter instituted under any ordinance specifically or impliedly repealed or amended by this ordinance adopting this penal regulation, for offenses committed prior to the effective date of this ordinance; and new prosecutions may be instituted and all prosecutions pending at the effective date of this ordinance, under and in accordance with the provisions of any ordinance in force at the time of the commission of such offenses.

Section 4. Severability Clause

Should any word, phrase, sentence, paragraph or section of this Ordinance be held invalid or unconstitutional, the remaining provision of this ordinance shall remain in full force and effect.

Section 5. Effective Date

This amendment to the Zoning Ordinance shall take effect seven (7) days after publication, which shall be published within 15 days of adoption, as required the Michigan Zoning Enabling Act (Act 110 of 2006).

This Ordinance is enacted by the Council of the City of Troy, Oakland County, Michigan, at a regular meeting of the City Council held at City Hall, 500 W. Big Beaver, Troy, MI, on the _____ day of _____, 2020.

Ethan Baker, Mayor

Aileen Dickson, City Clerk

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117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

<u>MEMORANDUM</u>

TO:	City of Troy Planning Commission Mark Miller, AICP, City Manager R. Brent Savidant, AICP, Planning Director
FROM:	Benjamin R. Carlisle, AICP
DATE:	October 15, 2019
RE:	Transitions and Increasing Square Footage Cap in a Cluster Development

The Planning Commission has recently held discussions regarding two potential zoning amendments to address ongoing topics: 1. Transitions, particularly adjacent to single family residential; and 2). Increasing the maximum square footage cap to quality for a density bonus in the cluster development option.

1. Transitions

The Planning Commission recently discussed transitions and frictions points between intense uses adjacent to single-family uses. At that meeting a number of options were presented for the Planning Commission to consider. The direction of the Planning Commission was to consider zoning amendments to ensure an appropriate intensity, height, and bulk transition between areas of potential friction. Due to the uniqueness of Big Beaver zoning, we recommend that right now we only focus on Neighborhood Nodes.

Based upon the direction from the Planning Commission, we offer the following amendments for consideration. These amendments would only apply to Neighborhood Nodes.

Development height, setback, and greenbelt provisions for any non-single family development in Neighborhood Nodes.

- 1. Height:
 - a. Any building, or portion of a building, on a parcel abutting a one-family residentially zoned parcel shall not exceed 2.5-stories, 30 feet in height.
 - b. Any building, or portion of a building, on a parcel that is not abutting a onefamily residentially zoned parcel shall not exceed 3-stories, 38 feet in height.

- 2. Setback and Greenbelt:
 - a. When a parcel is abutting a one-family residential zoned parcel the building setback from the property line of the one-family residential zoned parcel shall be no less than the height of the proposed building or twenty (20) feet, whichever is greater.
 - b. When a parcel is abutting a one-family residential zoned parcel a minimum 20foot landscaped greenbelt shall be maintained from the property line of the onefamily residential zoned parcel. The greenbelt shall be landscaped and screened in accordance with 13.02.B.
 - c. The Planning Commission may deviate from these setback and greenbelt provisions in the course of its site plan review process; however, the Planning Commission shall not permit a setback or greenbelt that is less than required in the building form or Section 13.02.B. In the review of the deviation, the Planning Commission shall consider the following standards:
 - *i.* The deviation will not adversely impact public health, safety, and welfare.
 - *ii.* The deviation maintains compatibility with adjacent uses.
 - *iii.* The deviation is compatible with the Master Plan and in accordance with the goals and objectives of the Master Plan and any associated subarea and corridor plans.
 - iv. The deviation will not adversely impact essential public facilities and services, such as: streets, pedestrian or bicycle facilities, police and fire protection, drainage systems, refuse disposal, water and sewage facilities, and schools.
 - v. The deviation will be in compliance with all other zoning ordinance standards.
 - vi. The deviation will not adversely impact any on-site or off-site natural features.

2. Housing Diversity and Options

The Planning Commission has questioned why the development community has not taken advantage of the housing diversity and option density bonus for smaller homes. Input from the development community notes that 1,500 sq/ft is too small to consider construction even with the associated density bonus. The Planning Commission has been told that a slight increase to 1,700 sq/ft would greatly assist in utilizing the density bonus. As such, we have proposed revised language to increase the maximum size to receive the density bonus from 1,500 to 1,700 sq/ft.

Revised Language:

Housing Diversity and Options. A bonus above the base yield number of units established in 10.04.C.1 may be provided for a development that provides a diverse variety of housing types or

provides a type of housing that is desired, but not currently offered in the city. The following requirements shall be met for the all bonus unit in excess of the base yield number of units:

- a. Maximum home square footage shall not exceed 1,500 1,700 sq/ft; and
- b. Master first floor bedroom and bathroom shall be provided.

Based upon discussion and direction of the Planning Commission, we can put this in ordinance form and prepare for a public hearing.

I look forward to discussing this further.

Sincerely,

engl. Cali

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP

From:	Tim Loughrin
To:	Brent Savidant
Subject:	Troy Cluster Ordinance Revision Letter
Date:	Friday, November 22, 2019 3:25:50 PM
Attachments:	image001.jpg
	image003.jpg
	RB Troy Cluster Letter.pdf
	Troy Square Footage Comparison.pdf

Hi Brent. Per our conversations, attached is a letter in regard to the upcoming review of the City's cluster provisions relating to home square footage. I'm happy to discuss in further detail. Also attached is some market data that backs up our assumptions.

Thanks, have a great weekend.

Tim Loughrin | Director of Land Acquisition

Robertson Brothers Homes | 6905 Telegraph Road, Suite 200| Bloomfield Hills, MI 48301 Direct: 248.282.1428 | Fax: 248.282.1429 | <u>www.robertsonhomes.com</u>



November 21, 2019

Mr. Brent Savidant City of Troy Community Development Director 500 W Big Beaver Road Troy, MI 48084

Re: Cluster Ordinance Text Amendment

Mr. Savidant:

It is Robertson's understanding that the City will be considering a text amendment to its Zoning Ordinance relating to the maximum square footage permitted for homes that qualify for a density bonus under the One-Family Cluster Development Option. Our understanding is that the maximum livable square footage may increase to 1,700 square feet from 1,500 square feet under Section 10.04.H.2.a. We applaud this direction as we believe there is an underserved market for those that want to stay in the City of Troy and downsize into a new, first-floor master bedroom home.

In considering the request, we would encourage the City to increase the maximum size of detached units to 1,900 square feet, as we believe the market conditions are such that this is a more appropriate size to justify the high costs to acquire and develop land in the City. In addition, our data indicates that the typical existing homeowner in Troy that would be an ideal buyer for this type of housing would be downsizing from a much larger home (2,800+ square feet), and our experience shows that they are looking for slightly more space than what 1,700 square feet provides.

Our organization would be highly interested in introducing a first-floor detached condominium home option similar to developments we are building in surrounding areas, namely Mill Ridge of Northville, and our Brewster Village community to be constructed next year in Rochester Hills. We believe keeping



residents in the City of Troy, while freeing up existing Colonial style housing for new residents to take advantage of the Troy School District, is a worthy endeavor to undertake. Increasing the maximum square footage for these home types is a critical component to doing just that.

Thank you.

Respectfully,

Tim Loughrin | Director of Land Acquisition Robertson Brothers Homes 6905 Telegraph Rd, Suite 200, Bloomfield Hills, MI 48301 Direct Dial: 248.282.1428 | Mobile: 248.752.7402 tloughrin@robertsonhomes.com

Architecture Level	2 Story
Status	Sold
Property Type	Residential

Row Labels	Average of List Price	Average of Est Fin Abv Grd SqFt	Average of Sq Ft \$	Count of MLS Number
100	\$143,267	1653	\$87.07	3
150	\$188,575	1417	\$143.12	4
200	\$238,527	1772	\$136.26	11
250	\$277,264	1868	\$150.48	61
300	\$332,440	2230	\$151.42	68
350	\$380,118	2428	\$158.12	71
400	\$428,192	2641	\$164.51	53
450	\$477,947	2891	\$166.90	56
500	\$529,709	3173	\$168.27	42
550	\$577,474	3309	\$176.29	19
600	\$624,836	3450	\$182.21	19
650	\$684,563	3637	\$190.99	8
700	\$734,400	3848	\$192.16	7
750	\$783,580	4243	\$186.98	5
800	\$849,900	3700	\$229.70	1
850	\$861,333	3874	\$223.85	3
1000	\$1,075,000	5695	\$188.76	1
Grand Total	\$425,718	2616	\$161.75	432

75% (328 of 432) of traditional colonial sales in the last 12 months

Sale Price \$300,000 to \$600,0000 Average sale price \$478,674 Average Square Foot 2875

Architecture Level	1 Story
Status	Sold
Property Type	Residential
Year Built	(All)
Year Built	(All)

Row Labels	Average of List Price	Average of Est Fin Abv Grd SqFt	Average of Sq Ft \$	Count of MLS Number
50	\$89,000	816	\$108.84	2
100	\$137,783	1103	\$130.06	6
150	\$177,668	1106	\$163.77	37
200	\$228,595	1379	\$170.46	77
250	\$279,206	1547	\$187.53	56
300	\$326,949	1822	\$185.02	41
350	\$374,037	1942	\$196.27	30
400	\$430,300	2260	\$193.71	11
450	\$475,775	2372	\$202.32	8
650	\$659,900	3922	\$168.26	1
Grand Total	\$277,476	1571	\$178.72	269

33 % (90 of 269) Ranch homes sold in the last 12 months fell in this range

Sale price \$300,000 to \$600,0000

Average sales price \$401,765

Avergae Square Footage 2099

From:	<u>Tim Loughrin</u>
To:	Brent Savidant
Subject:	Mill Ridge and Brewster Village Projects
Date:	Wednesday, November 27, 2019 3:51:21 PM
Attachments:	image001.jpg
	image003.jpg
	Mill Ridge Raleigh Elevations.pdf
	Raleigh Floor Plan.pdf
	Phase 1 and 2 Site Plan 11-13-19.pdf
	2019.01.24 Illustrative Site Plan.pdf

Hi Brent. Per our conversation, the link below is to our Mill Ridge of Northville community, which we have recently repositioned from duplex homes to a more desirable detached condominium, at 1,834 square feet. These are all first floor master homes and we have received very good feedback from prospective buyers on the design and layout. I believe this would be a very desirable home product in Troy and is one of the justifications we had made for potentially increasing the maximum size to 1,900 square feet for the proposed cluster ordinance revision.

I've attached a pdf of the site plan and elevation drawings, as well as a site plan of a new community named Brewster Village that we will be building in Rochester Hills next Spring, at the corner of Brewster and Walton. This community will use the same Raleigh plan and elevations. We would be happy to give you and any Commission or Council members a tour of the Northville model if you have any interest in that, just let me know.

https://www.robertsonhomes.com/communities/northville/mill-ridge-of-northville

Thanks, and have a Happy Thanksgiving.

Tim Loughrin | Director of Land Acquisition

Robertson Brothers Homes | 6905 Telegraph Road, Suite 200| Bloomfield Hills, MI 48301 Direct: 248.282.1428 | Fax: 248.282.1429 | <u>www.robertsonhomes.com</u>

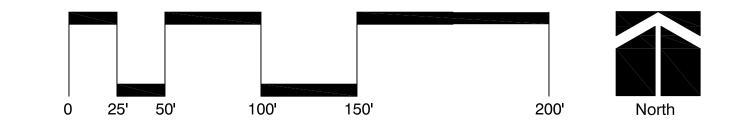




Mill Ridge of Northville

Northville Township, Michigan

November 2019







18161 W. Thirteen Mile Rd, Suite B-4 Southfield, MI 48076 T:: 248.594.3220 F:: 248.594.3260

Site Data:

Gross Site Area: Net Site Area: Proposed Units: 7.705 Ac. 7.168 Ac. 30 4.19 Du./Ac.

Building Setbacks:

- Front Yard:	20' (25' to Garage)
- Building Separation:	20' min.

26'

Road Width:

sheet title: Illustrative Site Plan

project title:

Brewster Village

City of Rochester Hills, MI

prepared for:

Robertson Brothers Homes 6905 Telegraph Rd. - Suite 200 Bloomfield Hills, MI 48301

Phone: 248.644.3460

■ job number:	■ date:
17031	11.13.2018
■ drawn by:	checked by:
■ drawn by: EMJ	■ checked by: WTK

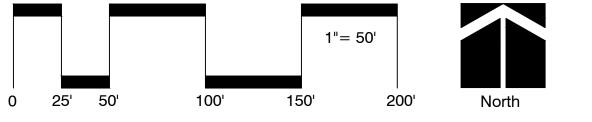


Know what's **below. Call** before you dig.

sheet no.

revisions:

01.24.2019 Per Municipal Review















Barrington Elevation









Cranbury Elevation









Dorchester Elevation

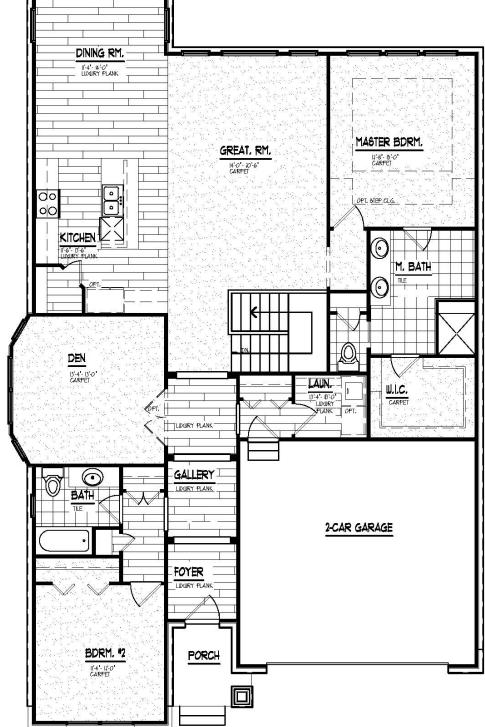












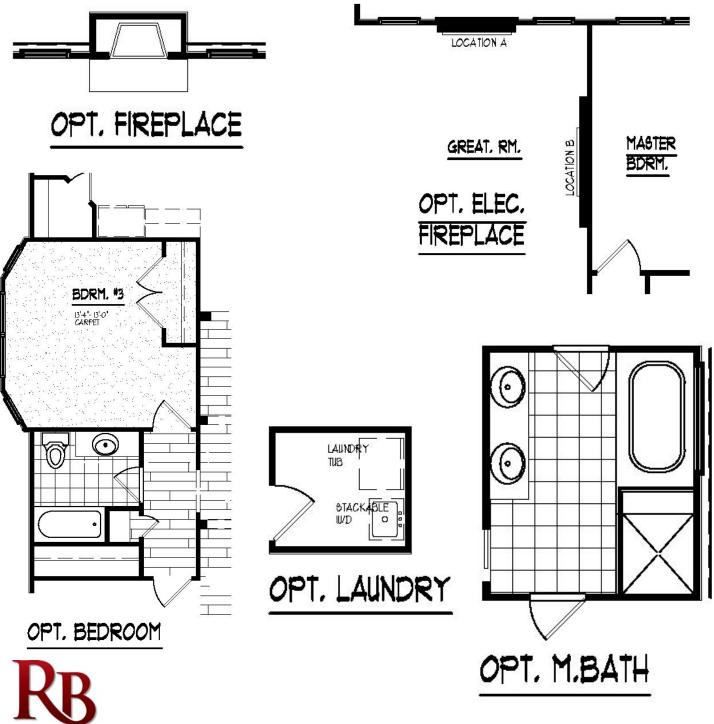






HOMES

Raleigh Main Level Options



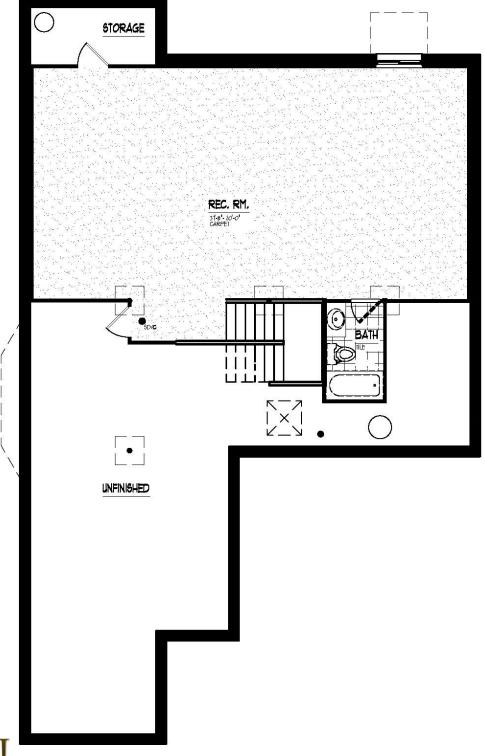




OMES

HC

Raleigh Lower Level Option 1 936 sqft.

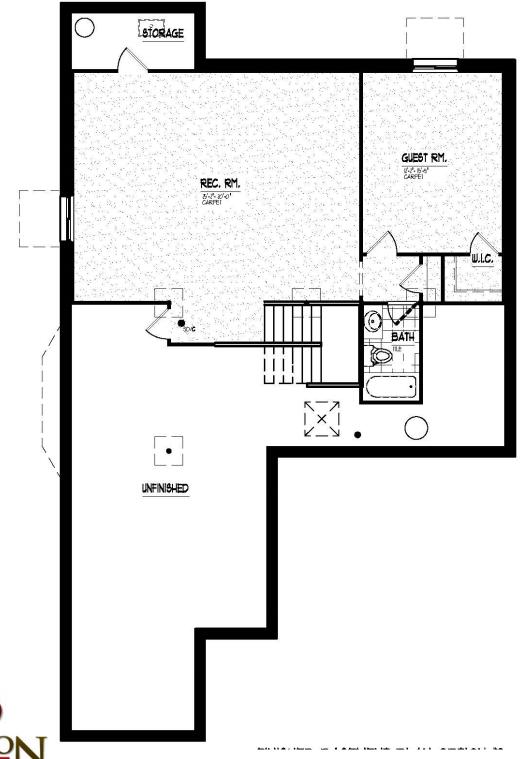






HOMES

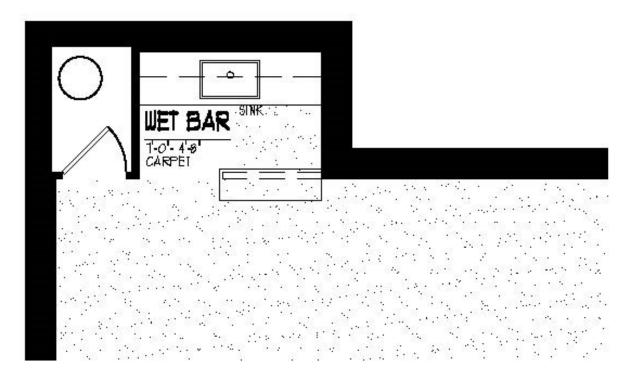
Raleigh Lower Level Option 2 936 sqft.







Raleigh Optional Lower Level Wet Bar







- DATE: December 5, 2019
- TO: Planning Commission
- FROM: R. Brent Savidant, Community Development Director
- SUBJECT: <u>PUBLIC HEARING ZONING ORDINANCE TEXT AMENDMENT (File Number</u> <u>ZOTA 255)</u> – Transitions in NN (Neighborhood Node) Zoning District

This item was initiated by Staff based on conversations with the Planning Commission and feedback from residents. The intent of the amendment is to protect single family residential property values by ensuring smooth transitions between multi-family and commercial development and single family residential neighborhoods.

The attached memo provides further background.

A Public Hearing is scheduled for this item for the December 10, 2019 Planning Commission meeting.

Attachments:

- 1. Planning Commission Public Hearing Draft.
- 2. Memo prepared by Carlisle/Wortman Associates, Inc., dated October 15, 2019.

G:\ZOTAs\ZOTA 255 Transitions in NN\PC Memo 2019 10 12.doc

PROPOSED RESOLUTION

<u>PUBLIC HEARING – ZONING ORDINANCE TEXT AMENDMENT (File Number ZOTA 255)</u> – Transitions in NN (Neighborhood Node) Zoning District

Resolution # PC-2019-12-

Moved by: Seconded by:

RESOLVED, That the Planning Commission hereby recommends to the City Council that Article 10 of Chapter 39 of the Code of the City of Troy, which includes provisions related to height and setback in the NN Neighborhood Node Zoning District, be amended as printed on the proposed Zoning Ordinance Text Amendment.

Yes: No: Absent:

MOTION CARRIED / DENIED

G:\ZOTAs\ZOTA 255 Transitions in NN\Proposed PC Resolution 12 10 2019.doc

CITY OF TROY

AN ORDINANCE TO AMEND CHAPTER 39 OF THE CODE OF THE CITY OF TROY CITY COUNCIL PUBLIC HEARING DRAFT

The City of Troy ordains:

Section 1. Short Title

This Ordinance shall be known and may be cited as an amendment to Chapter 39, Zoning Ordinance, of the Code of the City of Troy.

Section 2. Amendment

Chapter 39 of the Code of the City of Troy is amended as follows:

Revise Section 5.06.E.3 to read as follows:

Height and mass. Building height and mass in the form of building step-backs, recess lines or other techniques shall be graduated so that structures with higher intensity uses are comparable in scale with adjacent structures of lower intensity uses.

- c. Height and Mass. Building height and mass in the form of building step-backs, recess lines or other techniques shall be graduated so that structures with higher intensity uses are comparable in scale with adjacent structures of lower intensity uses.
 - i. <u>Any building, or portion of a building, on a parcel abutting a one-family</u> residentially zoned parcel shall not exceed 2.5-stories, 30 feet in height.
 - ii. <u>Any building, or portion of a building, on a parcel that is not abutting a one-</u><u>family residentially zoned parcel shall not exceed 3-stories, 38 feet in height.</u>
- d. Setback and Greenbelt:
 - i. When a parcel is abutting a one-family residential zoned parcel the building setback from the property line of the one-family residential zoned parcel shall be no less than the height of the proposed building or twenty (20) feet, whichever is greater.
 - ii. When a parcel is abutting a one-family residential zoned parcel a minimum 20-foot landscaped greenbelt shall be maintained from the property line of the one-family residential zoned parcel. The greenbelt shall be landscaped and screened in accordance with 13.02.B.
 - iii. <u>The Planning Commission may deviate from these setback and greenbelt</u> provisions in the course of its site plan review process; however, the Planning Commission shall not permit a setback or greenbelt that is less than required in the building form or Section 13.02.B. In the review of the deviation, the Planning Commission shall consider the following standards:

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- iii. <u>The deviation is compatible with the Master Plan and in accordance with the</u> <u>goals and objectives of the Master Plan and any associated subarea and</u> <u>corridor plans.</u>
- iv. The deviation will not adversely impact essential public facilities and services, such as: streets, pedestrian or bicycle facilities, police and fire protection, drainage systems, refuse disposal, water and sewage facilities, and schools.
- v. <u>The deviation will be in compliance with all other zoning ordinance</u> <u>standards.</u>
- vi. <u>The deviation will not adversely impact any on-site or off-site natural</u> <u>features.</u>

d e. Orientation. Primary building facades shall be placed away from the Buildings shall be oriented in such a way as to minimize the impact on abutting residential use.

e <u>f</u>. Architectural Features. Similarly sized and patterned architectural features such as windows, doors, arcades, pilasters, cornices, wall offsets, building materials, and other building articulations included on the lower-intensity use shall be incorporated in the transitional features.

Section 3. Savings

All proceedings pending, and all rights and liabilities existing, acquired or incurred, at the time this Ordinance takes effect, are hereby saved. Such proceedings may be consummated under and according to the ordinance in force at the time such proceedings were commenced. This ordinance shall not be construed to alter, affect, or abate any pending prosecution, or prevent prosecution hereafter instituted under any ordinance specifically or impliedly repealed or amended by this ordinance adopting this penal regulation, for offenses committed prior to the effective date of this ordinance; and new prosecutions may be instituted and all prosecutions pending at the effective date of this ordinance, under and in accordance with the provisions of any ordinance in force at the time of the commission of such offenses.

Section 4. Severability Clause

Should any word, phrase, sentence, paragraph or section of this Ordinance be held invalid or unconstitutional, the remaining provision of this ordinance shall remain in full force and effect. Section 5. Effective Date

This amendment to the Zoning Ordinance shall take effect seven (7) days after publication, which shall be published within 15 days of adoption, as required the Michigan Zoning Enabling Act (Act 110 of 2006).

This Ordinance is enacted by the Council of the City of Troy, Oakland County, Michigan, at a regular meeting of the City Council held at City Hall, 500 W. Big Beaver, Troy, MI, on the _____ day of _____, 2020.

Ethan Baker, Mayor

Aileen Dickson, City Clerk

G:\ZOTAs\ZOTA 255 Transitions in NN\PC Public Hearing Draft ZOTA 255.doc



117 NORTH FIRST STREET SUITE 70 ANN ARBOR, MI 48104 734.662.2200 734.662.1935 FAX

<u>MEMORANDUM</u>

TO:	City of Troy Planning Commission Mark Miller, AICP, City Manager R. Brent Savidant, AICP, Planning Director
FROM:	Benjamin R. Carlisle, AICP
DATE:	October 15, 2019
RE:	Transitions and Increasing Square Footage Cap in a Cluster Development

The Planning Commission has recently held discussions regarding two potential zoning amendments to address ongoing topics: 1. Transitions, particularly adjacent to single family residential; and 2). Increasing the maximum square footage cap to quality for a density bonus in the cluster development option.

1. Transitions

The Planning Commission recently discussed transitions and frictions points between intense uses adjacent to single-family uses. At that meeting a number of options were presented for the Planning Commission to consider. The direction of the Planning Commission was to consider zoning amendments to ensure an appropriate intensity, height, and bulk transition between areas of potential friction. Due to the uniqueness of Big Beaver zoning, we recommend that right now we only focus on Neighborhood Nodes.

Based upon the direction from the Planning Commission, we offer the following amendments for consideration. These amendments would only apply to Neighborhood Nodes.

Development height, setback, and greenbelt provisions for any non-single family development in Neighborhood Nodes.

- 1. Height:
 - a. Any building, or portion of a building, on a parcel abutting a one-family residentially zoned parcel shall not exceed 2.5-stories, 30 feet in height.
 - b. Any building, or portion of a building, on a parcel that is not abutting a onefamily residentially zoned parcel shall not exceed 3-stories, 38 feet in height.

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2. Housing Diversity and Options

The Planning Commission has questioned why the development community has not taken advantage of the housing diversity and option density bonus for smaller homes. Input from the development community notes that 1,500 sq/ft is too small to consider construction even with the associated density bonus. The Planning Commission has been told that a slight increase to 1,700 sq/ft would greatly assist in utilizing the density bonus. As such, we have proposed revised language to increase the maximum size to receive the density bonus from 1,500 to 1,700 sq/ft.

Revised Language:

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provides a type of housing that is desired, but not currently offered in the city. The following requirements shall be met for the all bonus unit in excess of the base yield number of units:

- a. Maximum home square footage shall not exceed 1,500 1,700 sq/ft; and
- b. Master first floor bedroom and bathroom shall be provided.

Based upon discussion and direction of the Planning Commission, we can put this in ordinance form and prepare for a public hearing.

I look forward to discussing this further.

Sincerely,

engl. Cali

CARLISLE/WORTMAN ASSOC., INC. Benjamin R. Carlisle, LEED AP, AICP

CITY OF TROY MICHIGAN PUBLIC NOTICE CITY PLANNING COMMISSION

In accordance with the provisions of the Michigan State Law, Notice is hereby given that the Planning Commission of the City of Troy will hold Public Meetings in the City Hall, 500 West Big Beaver Road, Troy, Michigan, (248) 524-3364, on the following dates:

2020 PLANNING COMMISSION REGULAR MEETING DATES

January 14	July 14
January 28	July 28
February 11	August 11
February 25	August 25
March 10	September 8
March 24	September 22
April 14	October 13
April 28	October 27
May 12 May 26	November 10
June 9 June 23	December 8

All meetings are held in City Hall and are open to the public. The Agenda and City website will reflect any changes in meeting times and/or rooms.

Regular Planning Commission meetings begin at 7:00 p.m. and are held in the Council Board Room. Meetings are subject to be held in the Council Chamber based on anticipated audience capacity.

This notice is hereby posted as required by Section 4 of the Open Meetings Act (MCLA 15.261 et seq.)

R. Brent Savidant, AICP Community Development Director

Posted: _____, 2019

NOTICE: People with disabilities needing accommodations for effective participation in this meeting should contact the City Clerk by e-mail at <u>clerk@troymi.gov</u> or by calling (248) 524-3316 at least two working days in advance of the meeting. An attempt will be made to make reasonable accommodations.